

PLANS OF PROPOSED P.P.C.C. BRIDGE OVER ON

LENGTH 12 352 OUT TO OUT OF ABUTMENT PRECAST BACKWALL PANELS

SUPERSTRUCTURE ONE SIMPLY SUPPORTED SPAN OF PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS WITH ASPHALT OVERLAY

SUBSTRUCTURE TWO PRECAST CONCRETE ABUTMENTS WITH STEEL H-PILES

ROADWAY WIDTH 8 400 OUT TO OUT OF GIRDERS

LOCATION IN R.M. OF

SHEET LEGEND

1. COVER SHEET
2. GENERAL ELEVATION
3. BORING LOGS
4. SITE AND EROSION CONTROL DETAILS
5. ASSEMBLY DETAILS
6. ASSEMBLY DETAILS
7. STEEL PILE CAP DETAILS
8. BEARING AND ERECTION DETAILS
9. RAILING LAYOUT AND DETAILS
10. RAILING DETAILS
11. RAILPOST DETAILS
- P1. PRECAST PANEL DETAILS
- P2. PRECAST PANEL DETAILS
- G1. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G2. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G3. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G4. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G5. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS

DESIGN DATA

SPECIFICATIONS

AASHTO LRFD Bridge Design Specifications, First Edition, 1994 plus 1996/97 Interims

VEHICULAR LIVE LOADING

1. Modified AASHTO HSS-25 Truck
2. AASHTO LRFD "HL-93" Loading

STRUCTURAL CONCRETE

CSA A23.1, Exposure Class C-1 Air content category 1

1. PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS - $f_c = 45 \text{ MPa}$ at 28 days
 $f_{ci} = 35 \text{ MPa}$ at time of de-stressing
2. PRECAST PANELS - $f_c = 35 \text{ MPa}$

REINFORCING STEEL

1. PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS - CAN/CSA-G30.18-M92 Grade 400W black (i.e no epoxy coating)
2. PRECAST PANELS - CAN/CSA-G30.18-M92 Grade 400W black (i.e no epoxy coating)

STRUCTURAL STEEL

1. All Structural Steel shall conform to CAN/CSA G40.21-M92 Grade 300W
2. HSS Tubing for Bridge Rail shall conform to CAN/CSA- G40.21-M92 Grade 350W

PRESTRESSING STRAND

20-13 # low relaxation strands, $f_{pu} = 1860 \text{ MPa}$

PILE LOADING

	END PILE BENTS	INTERMEDIATE PILE BENTS
MAXIMUM FACTORED LOAD	582 kN	531 kN
FACTORED BEARING RESISTANCE		

HYDRAULIC DESIGN DATA

DESIGN DISCHARGE

$Q = \text{-----} \text{ m}^3/\text{sec}$

SURVEY CONTROL

HORIZONTAL DATUM: NAD83CSRS
 VERTICAL DATUM: CGVD28
 ELLIPSOID: GRS 1980
 GEOID (HT2.0): -----
 UTM: ZONE ----
 SCALE FACTOR: -----

SITE CONTROL POINT DATA

CONTROL POINT *-----	NORTHING: -----	-----
	EASTING: -----	-----
	ELEVATION: -----	-----
	DATE: -----	-----
CONTROL POINT *-----	NORTHING: -----	-----
	EASTING: -----	-----
	ELEVATION: -----	-----
	DATE: -----	-----
CONTROL POINT *-----	NORTHING: -----	-----
	EASTING: -----	-----
	ELEVATION: -----	-----
	DATE: -----	-----



TP. -
PLACE LOCATION
MAP HERE

RGE. -
LOCATION MAP
Not to Scale

MANITOBA INFRASTRUCTURE

WATER MANAGEMENT AND STRUCTURES

ENVIRONMENTAL APPROVALS

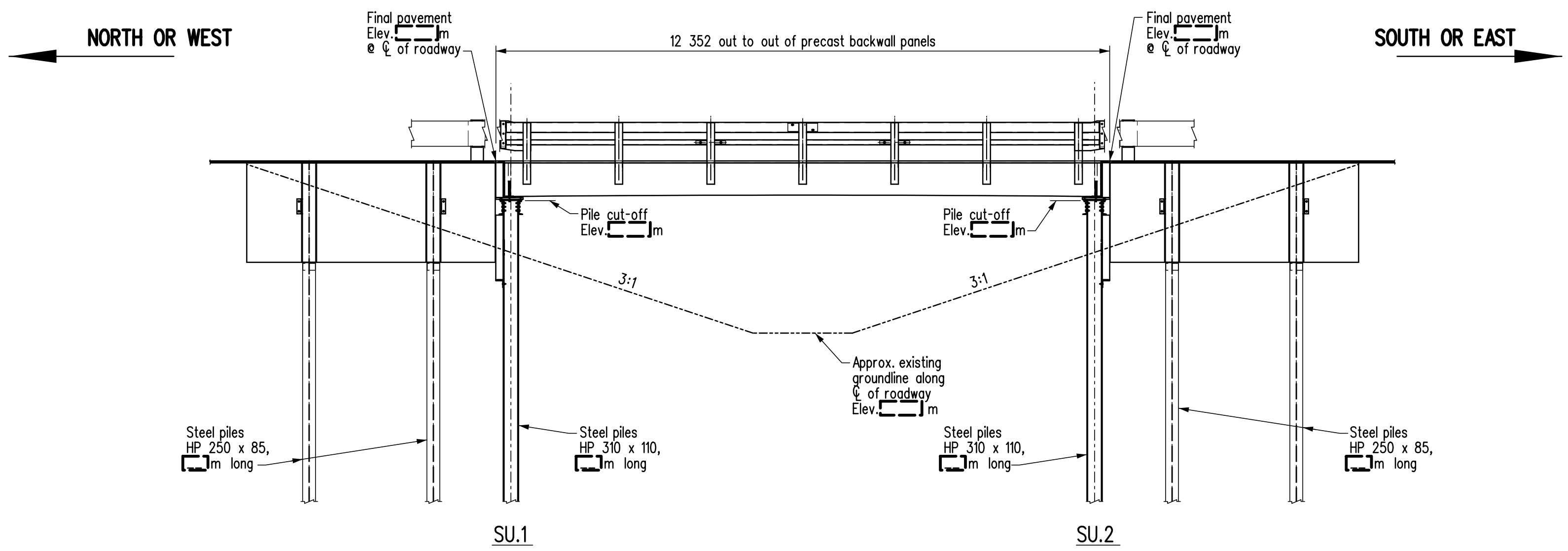
- MANITOBA ENVIRONMENT ACT LICENCE
DATE : _____
FILE # : _____
- FISHERIES AND OCEANS CANADA - AUTHORIZATION OR REVIEW
DATE : _____
FILE # : _____
- TRANSPORT CANADA - NAVIGATION ACT
DATE : _____
FILE # : _____
- MANITOBA INFRASTRUCTURE ENVIRONMENTAL APPROVAL
DATE : _____
FILE # : _____
- ENVIRONMENTAL REVIEW COMPLETED
DATE : _____
COMPLETED BY : _____

ALL DIMENSIONS ARE IN MILLIMETRES (mm) AND ALL ELEVATIONS AND STATIONS ARE IN METRES (m) UNLESS SHOWN OTHERWISE.

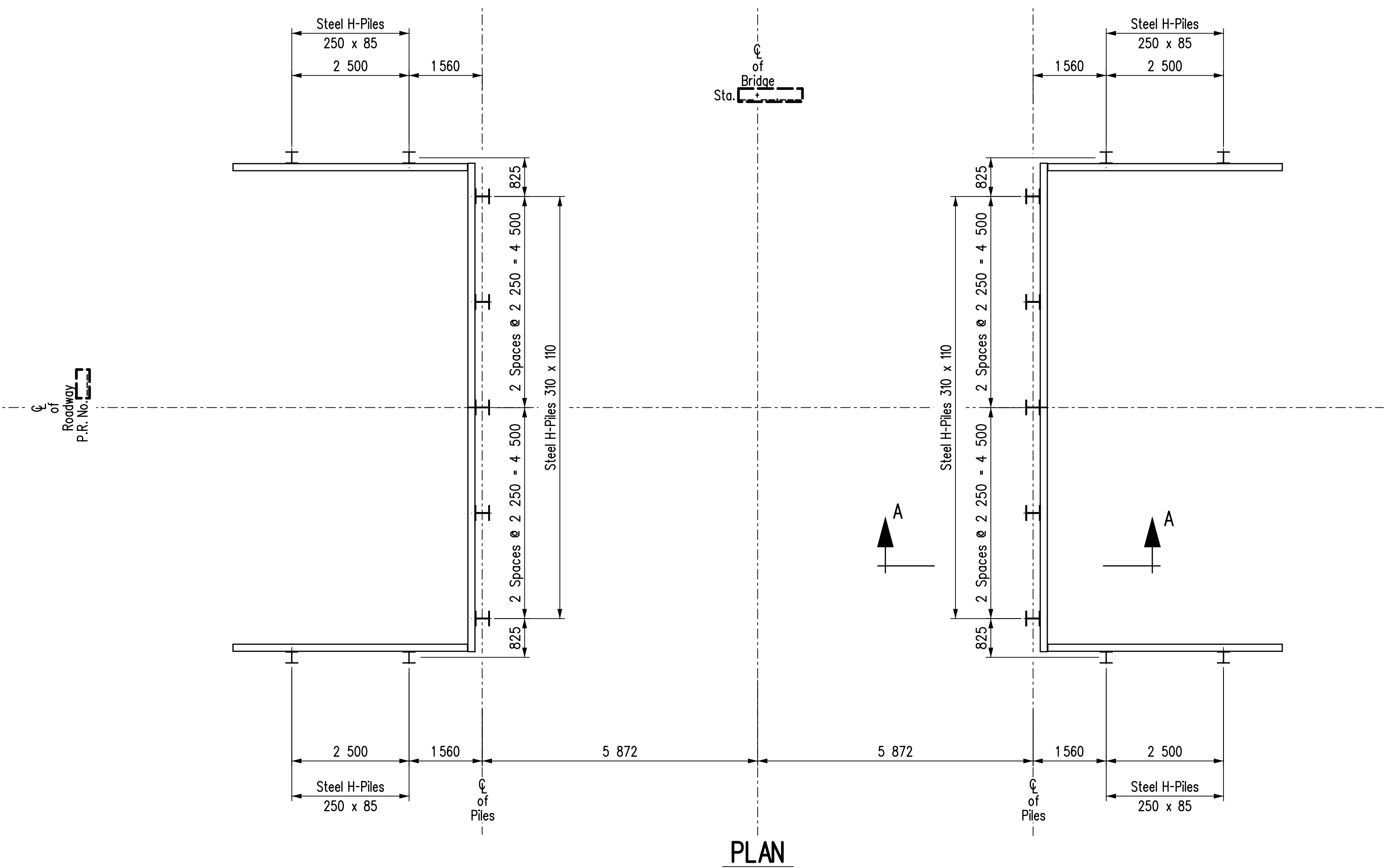
RELEASED FOR CONSTRUCTION BY : _____
 EXECUTIVE DIRECTOR OF STRUCTURES
 DATE _____

DRAWN BY: _____	DATE: _____	SHEET No. 1
CHECKED BY: _____	DATE: _____	SITE No. _____

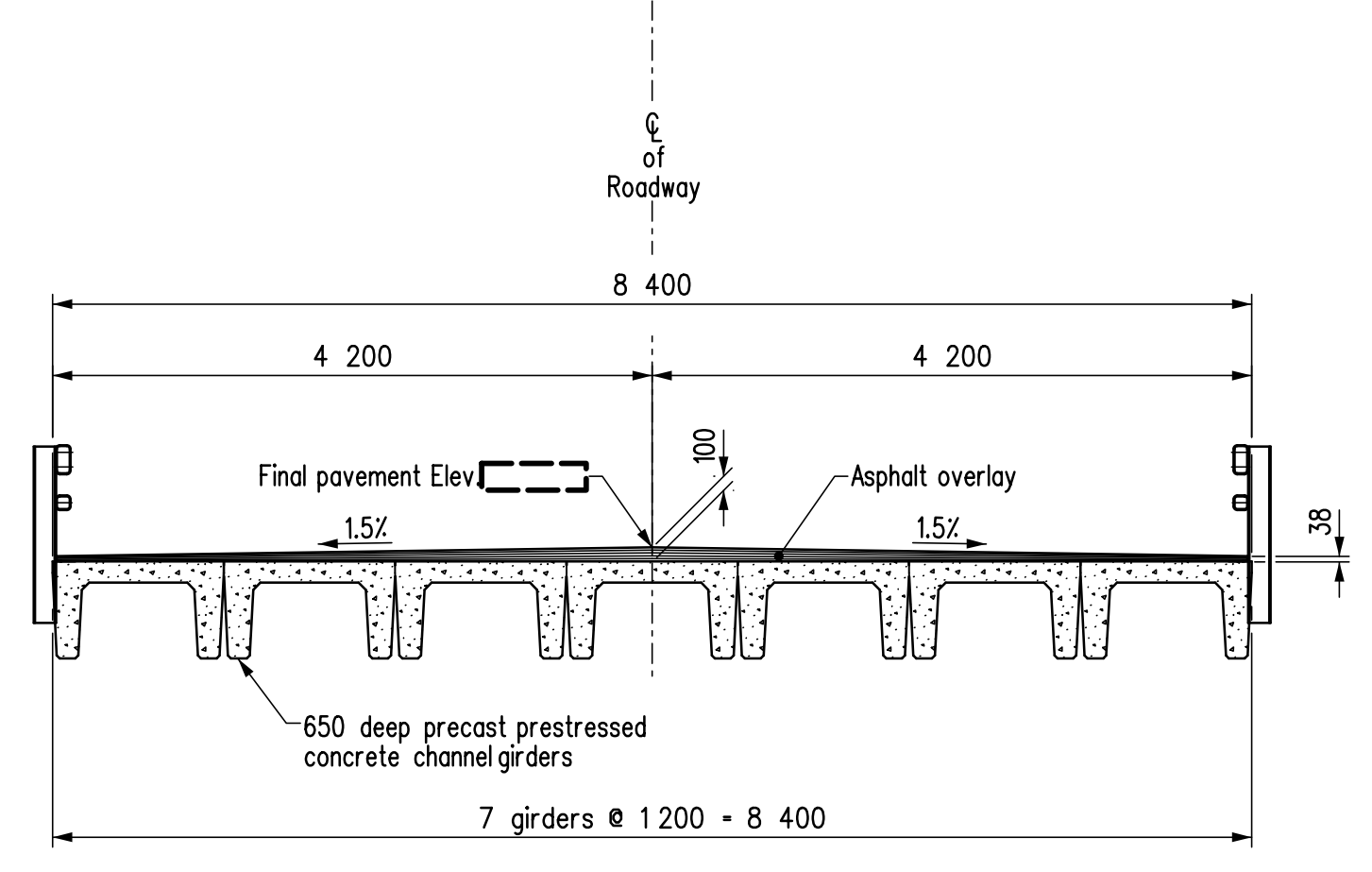




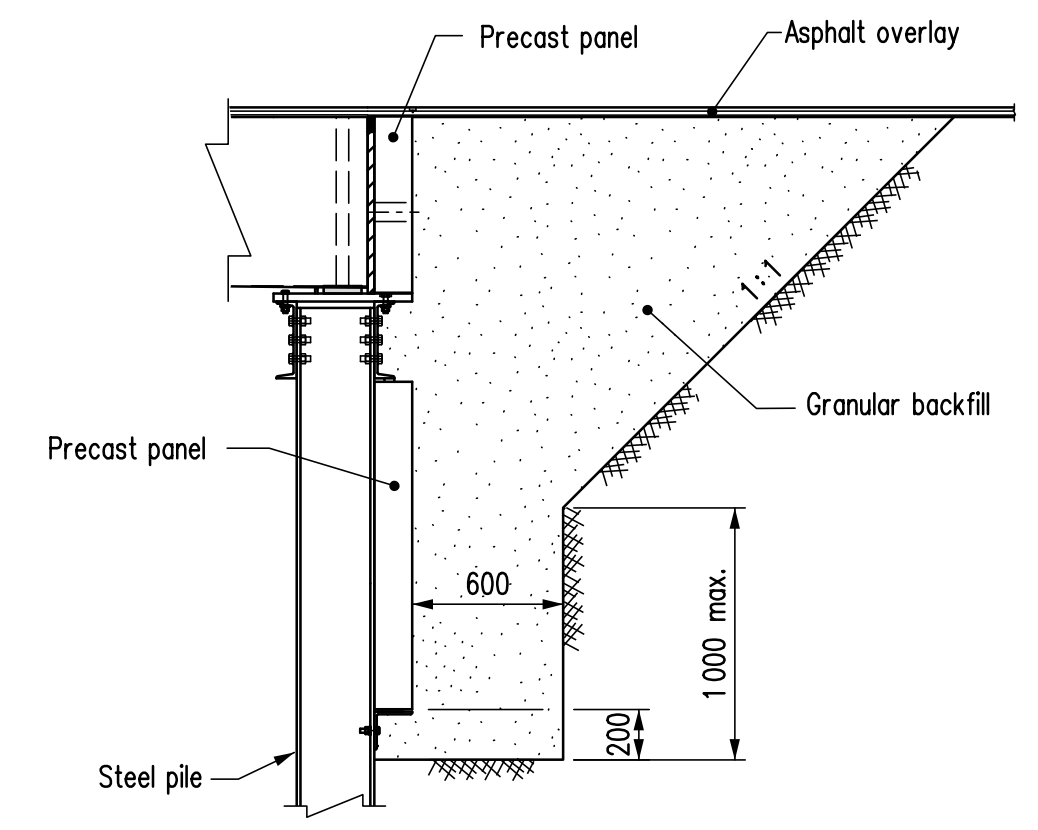
ELEVATION



PLAN



CROSS SECTION
Scale 1:50

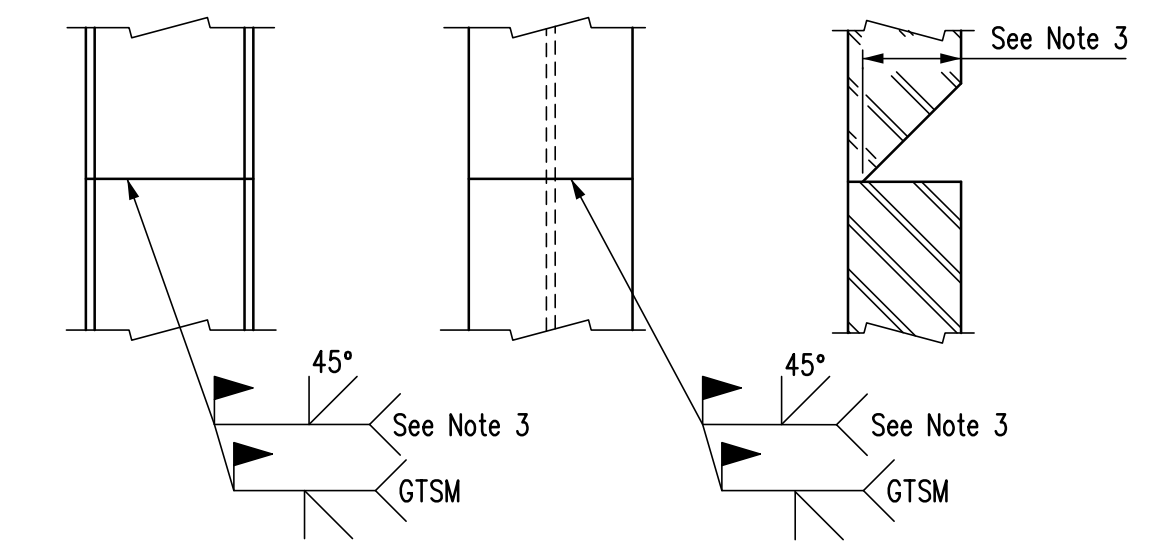


SECTION A-A
Typical at Su.1
Scale 1:30

- NOTES :**
re: Backfill Behind Abutment Ballast Walls
- Backfill behind ballast wall and wingwall panels shall be Type 1 - Granular backfill supplied and placed in accordance with Bridge Specification 1001 (I). The granular backfill shall be placed and compacted in lifts not exceeding 150 mm.
 - Compaction equipment used within 2 m of ballast walls and wingwalls shall be limited to light vibratory equipment with a mass not exceeding 120 kg unless otherwise approved.
 - Steel pile tip to be PRUYN "Hard-Bite" or equivalent.

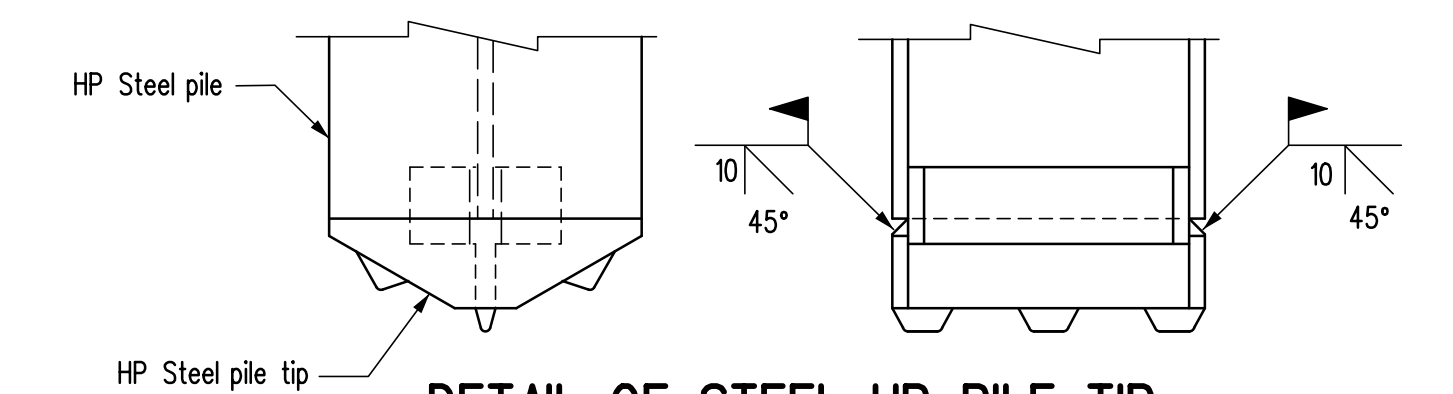
BILL OF PILES			Site No.	
LOCATION	DESCRIPTION	No. OF PILES	LENGTH	TOTAL LENGTH (m)
SU.1 & SU.2	Steel piles - HP310 x 110 (abutments)	10		0
SU.1 & SU.2	Steel piles - HP250 x 85 (wing walls)	8		0
TOTAL LENGTH OF PILES (m) =				0

BILL OF PILE TIPS		
LOCATION	DESCRIPTION	No. OF PILES
SU.1 & SU.2	Hard-Bite Point HP-77750-B for HP310 x 110 (Abutments)	10



DETAIL OF STEEL HP PILE SPLICE

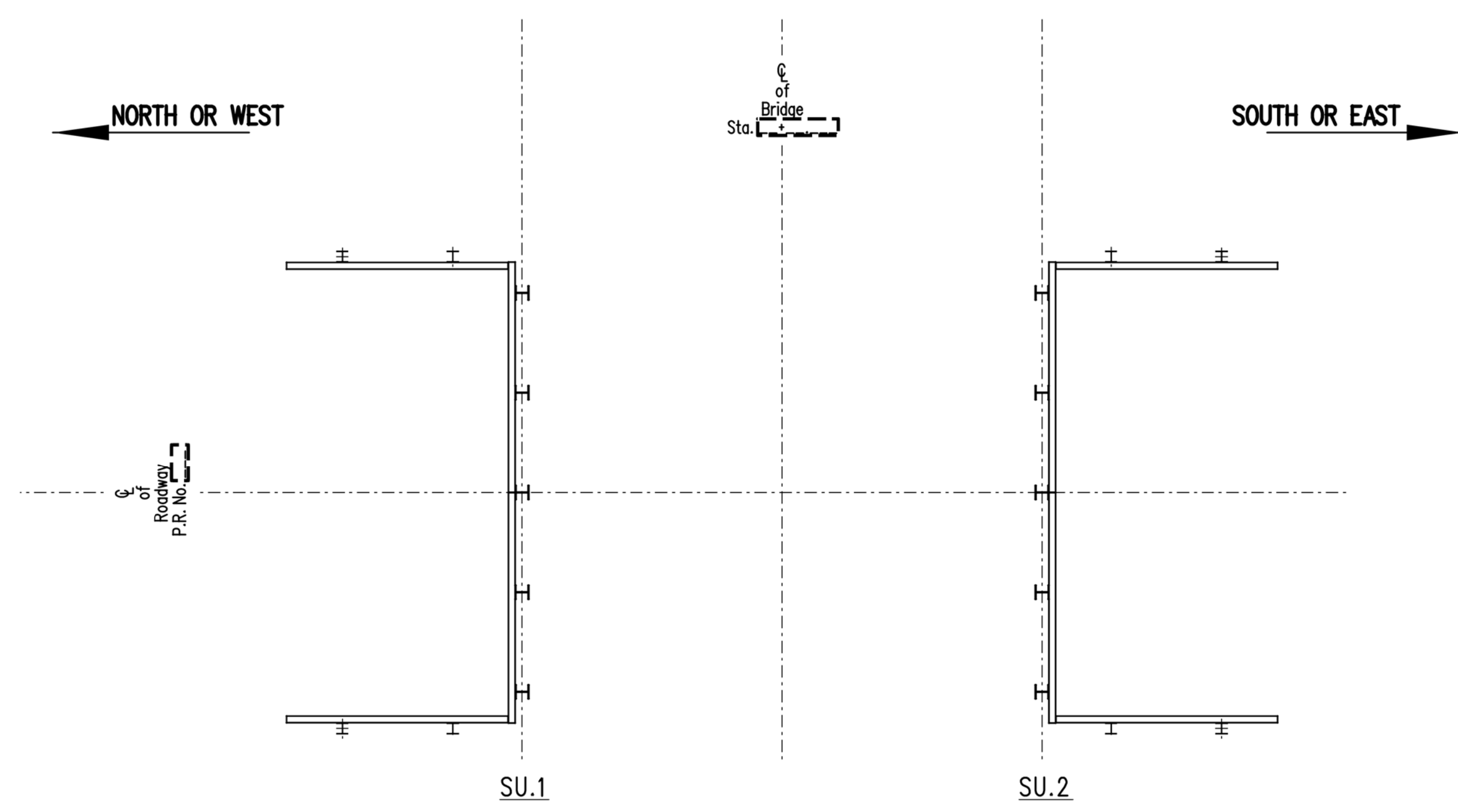
- NOTES:**
Not To Scale
- re: Welding
- Low hydrogen *E70 series electrodes shall be used.
 - The minimum root pass shall be 6 mm.
 - Preparation for welding requires 13 mm bevel for HP 250 piles and 14 mm bevel for HP 310 piles.
 - Weld both flanges and web as shown. The inside beveling and welds to be completed first.
 - Before undertaking the back welds, the weld preparation shall be carried out with a carbon Arc-Air gouger.
- *E48018 equivalent metric electrode



DETAIL OF STEEL HP PILE TIP

- NOTES :**
Not to Scale
- Edges of HP Steel pile tip to be ground on 45° bevel for 10 mm.
 - Low hydrogen *E70 series electrodes shall be used.
 - The minimum root pass shall be 6 mm.
- *E48018 equivalent metric electrode

REVISIONS		GENERAL ELEVATION	
DATE	BY	DESIGN	RELEASED FOR CONSTRUCTION BY:
		DESIGN SEAL	EXECUTIVE DIRECTOR OF STRUCTURES DATE
		RECORD SEAL	SCALE: 1:75 SHEET No. 2
<p style="text-align: center;">PLACE ENGINEERS ELECTRONIC SEAL HERE</p>		<p style="text-align: center;">Manitoba Infrastructure Water Management and Structures</p>	
		<p>BY: B.A.N.</p> <p>CHECKED: _____</p> <p>BY: _____</p> <p>CHECKED: _____</p>	
		<p>or as shown</p> <p>SITE No. _____</p>	



PLAN
Showing Bore Hole Locations

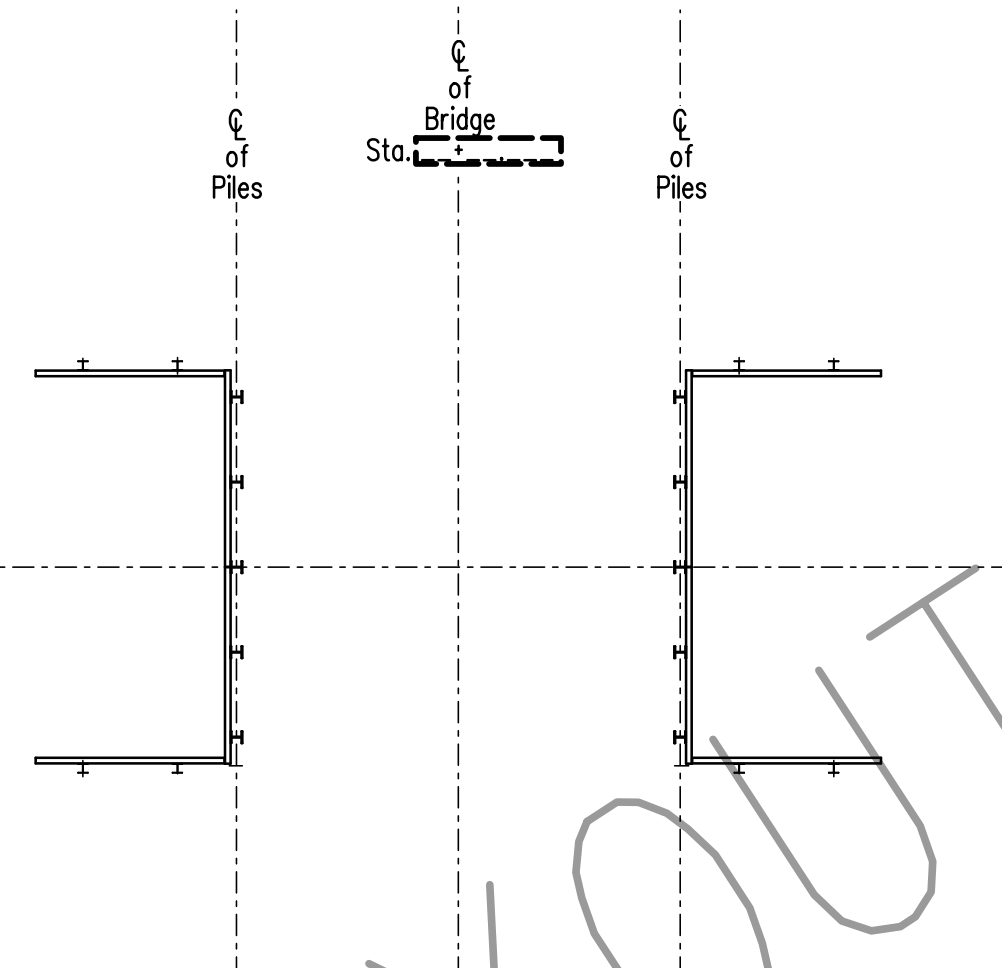
NOTES - re: Boring Logs

- The Department provides log boring information shown on the Plans. This information may not be representative of the soil conditions throughout the site. Contractors may peruse all available soil information in the Water Management and Structures Branch located at 6th floor, 215 Garry Street, Winnipeg.
- The following abbreviations apply to bore hole information:
 Qu - Laboratory unconfined compressive strength in kPa
 SPT (N) - Number of blows per 300 mm - Standard Penetration Test
 USC - Unified Soil Classification
 M.C. - Moisture Content
- All stations, elevations, offsets and depths as shown are in meters.
 All dimensions are in millimeters.
- All bore hole locations shown in plan view are approximate.
- Elevations on boring logs are at a vertical scale of 1:100.

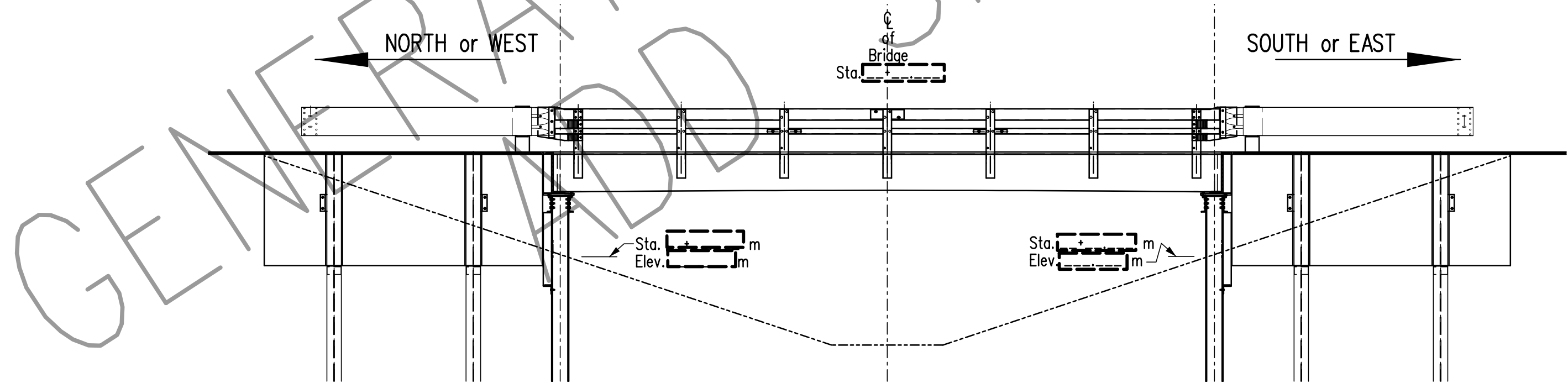
REVISIONS		BORING LOGS	
DATE	BY	DESCRIPTION	
		DESIGN SEAL	RECORD SEAL
PLACE ENGINEERS ELECTRONIC SEAL HERE		 Infrastructure Water Management and Structures	
		RELEASED FOR CONSTRUCTION BY: _____	
		EXECUTIVE DIRECTOR OF STRUCTURES DATE _____	
		SCALE: 1:100 SHEET No. 3	
		or as shown SITE No. 221	

NORTH OR WEST

SOUTH OR EAST



PLAN



NORTH or WEST

SOUTH or EAST

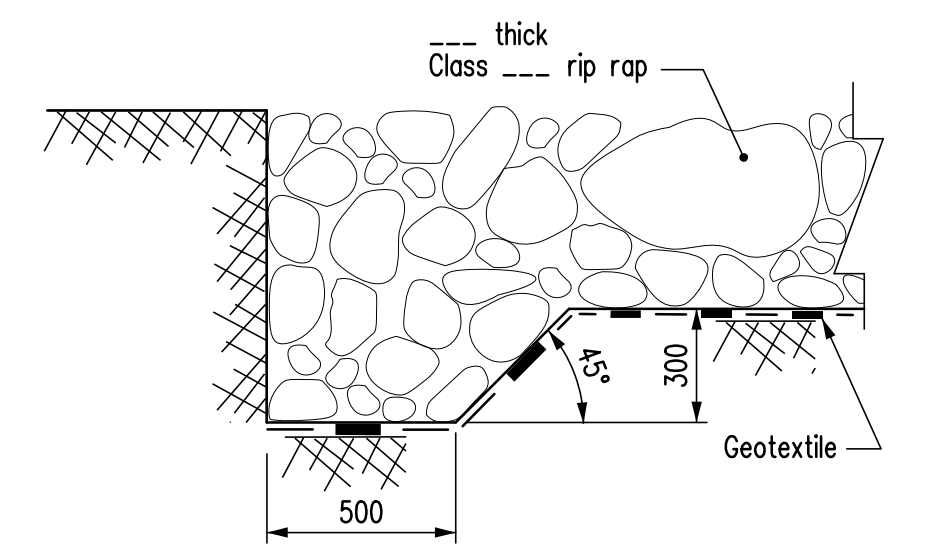
SU.1

SU.2

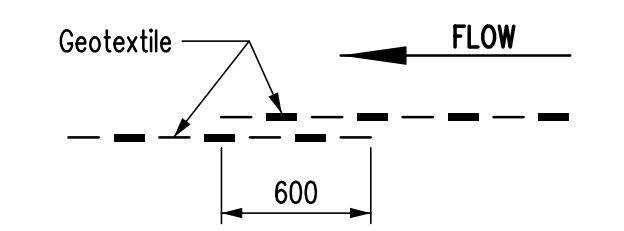
ELEVATION
Scale: 1:75

UTILITY DISCLAIMER:
 LOCATIONS OF UTILITIES AS SHOWN ARE BASED ON READILY AVAILABLE INFORMATION. NO GUARANTEE IS GIVEN THAT ALL UTILITIES ARE SHOWN OR THAT THE GIVEN LOCATIONS ARE EXACT. CONTRACTOR SHALL CONFIRM THE EXISTENCE AND LOCATION OF UTILITIES BY OBTAINING FROM THE INDIVIDUAL UTILITIES BEFORE PROCEEDING WITH CONSTRUCTION.

GENERATED EXAMPLE SHEET FROM ONLY YOUR SITE PLAN
 LAYOUT YOUR DIMENSIONS



EDGE TREATMENT



OVERLAPPING DETAILS

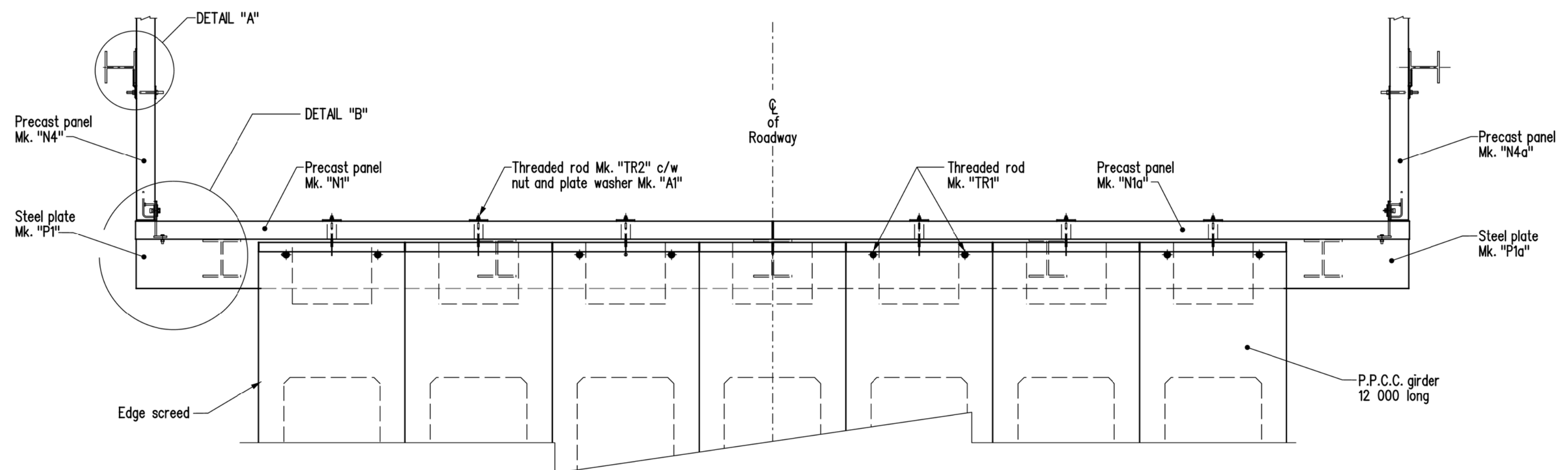
RIP RAP DETAILS

Not To Scale

- NOTES:**
- All geotextile shall be Non-Woven Geotextile, Class 1 (Heavy Duty) from the Manitoba Infrastructure's Approved Product List.
 - Geotextile shall be placed under all rip rap, overlapping 600mm in direction of flow.

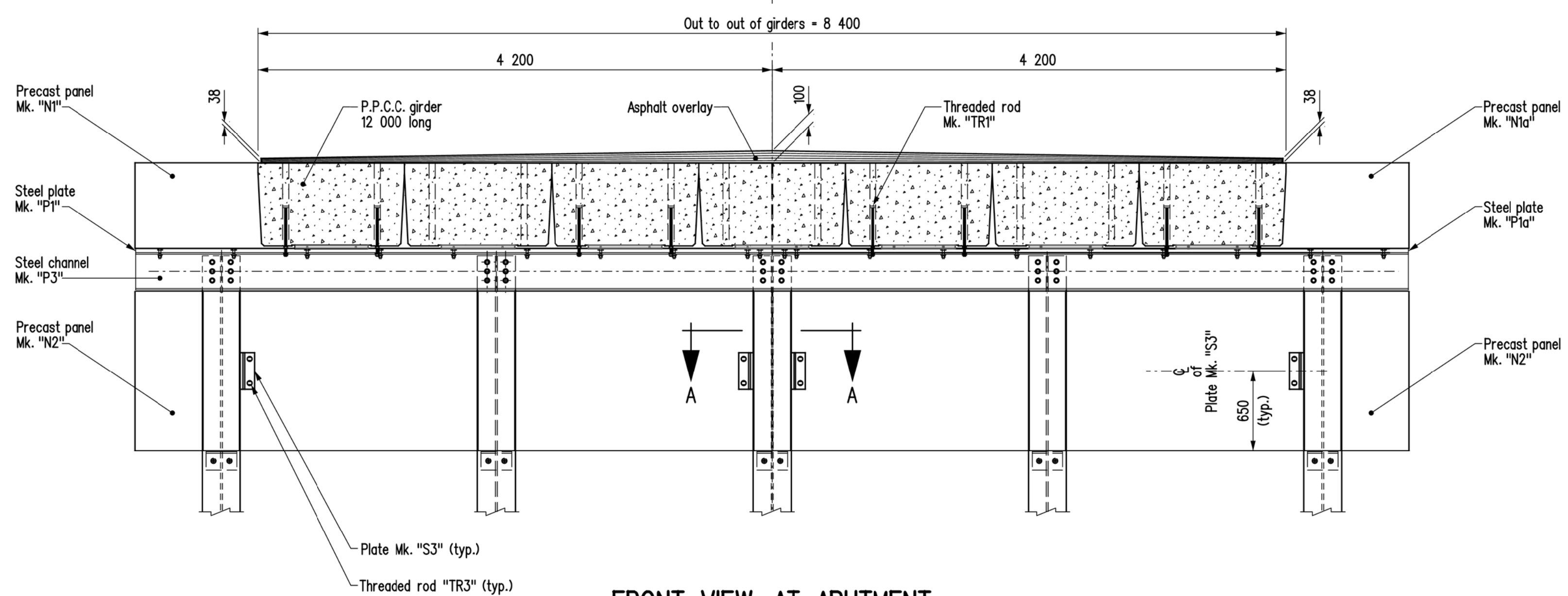
NOTE:
 Existing pile bents to be removed by Bridge Contractor.

REVISIONS		SITE AND EROSION CONTROL DETAILS	
DATE	DESCRIPTION		
		<p>Manitoba Infrastructure Water Management and Structures</p>	
DESIGN SEAL	RECORD SEAL	<p>BY: _____ B.A.N.</p> <p>CHECKED: _____</p>	<p>RELEASED FOR CONSTRUCTION BY: _____</p> <p>EXECUTIVE DIRECTOR OF STRUCTURES DATE</p>
<p style="text-align: center; font-weight: bold; font-size: 1.2em;">PLACE ENGINEERS ELECTRONIC SEAL HERE</p>		<p>BY: _____ K.P.</p> <p>CHECKED: _____</p>	<p>SCALE: 1:200 SHEET No. 4</p> <p>or as shown SITE No. _____</p>



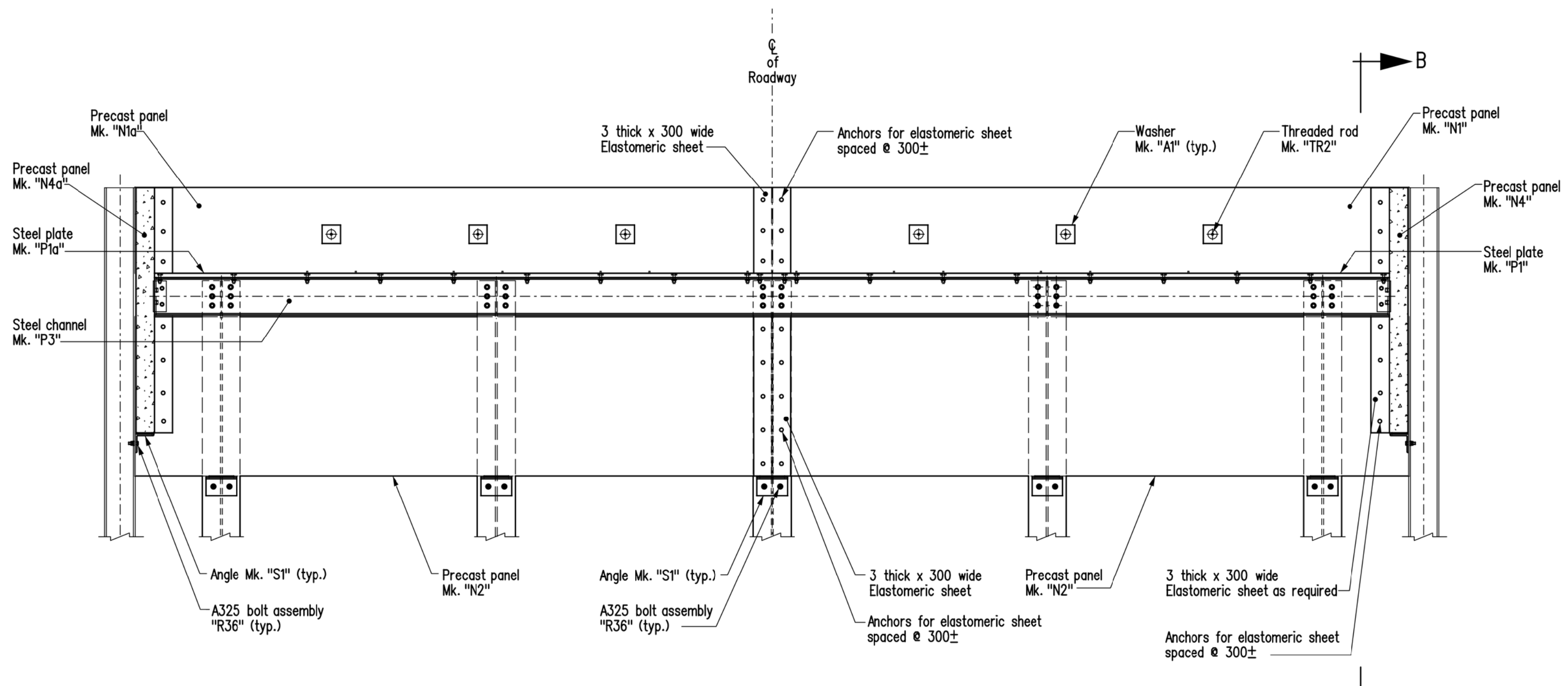
PART PLAN

Showing abutment SU.1
Asphalt and bridge railing not shown for clarity



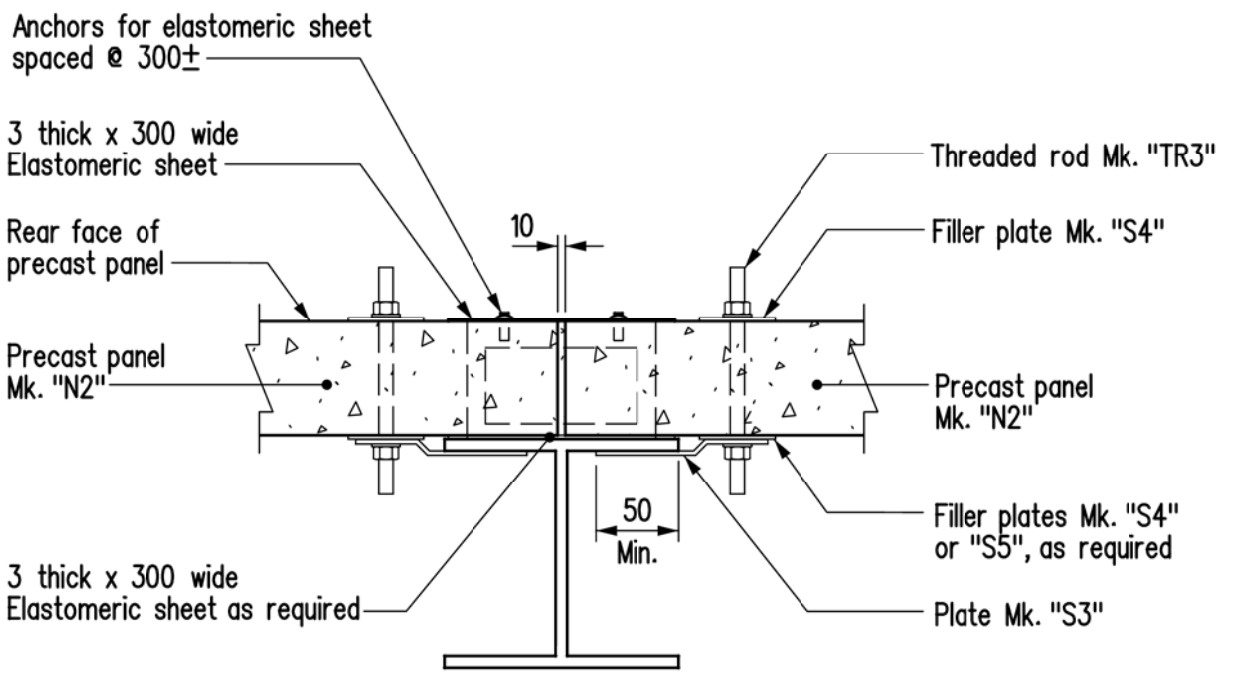
FRONT VIEW AT ABUTMENT

Bridge railing not shown for clarity



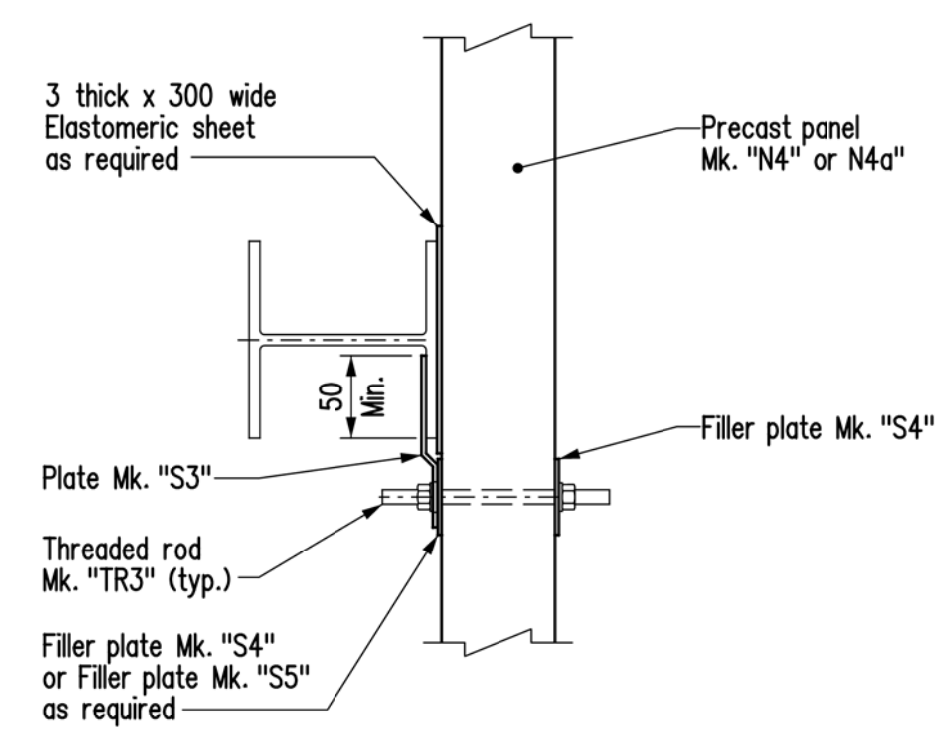
REAR VIEW AT ABUTMENT

Bridge railing not shown for clarity



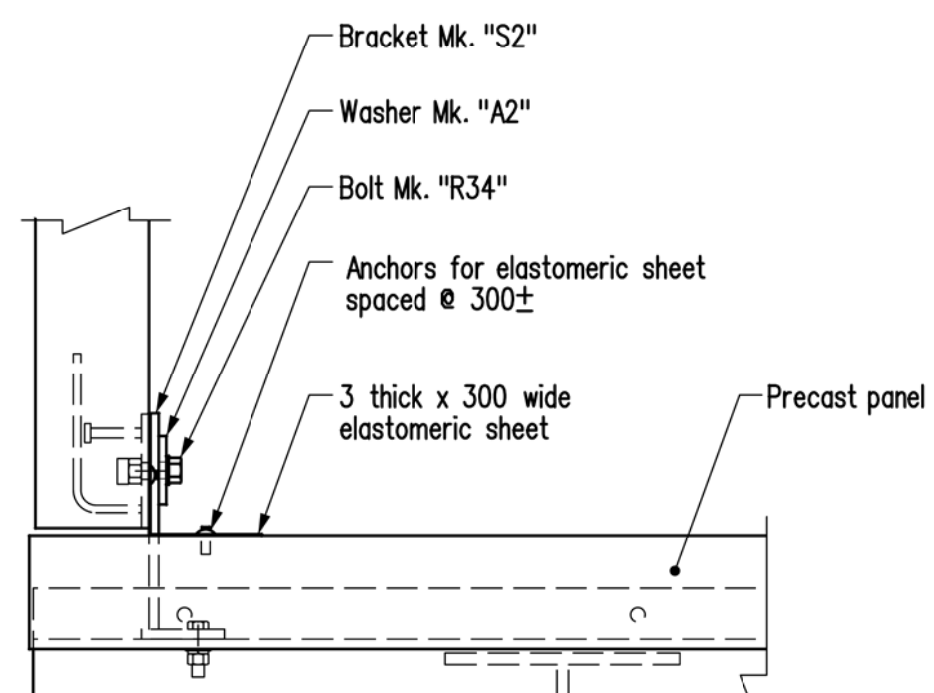
SECTION A-A

Scale 1:10



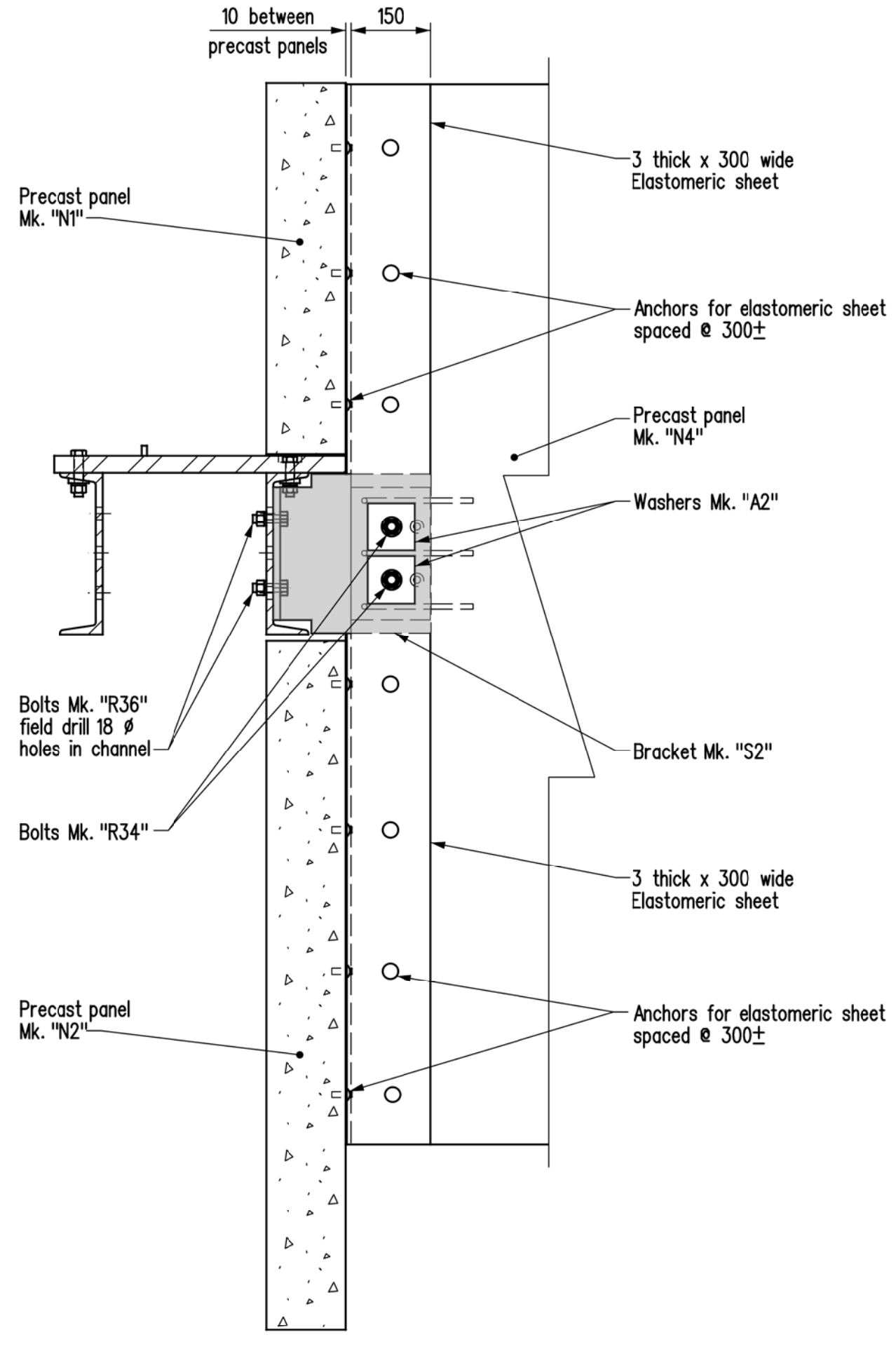
DETAIL "A"

Scale 1:10



DETAIL "B"

Scale 1:10



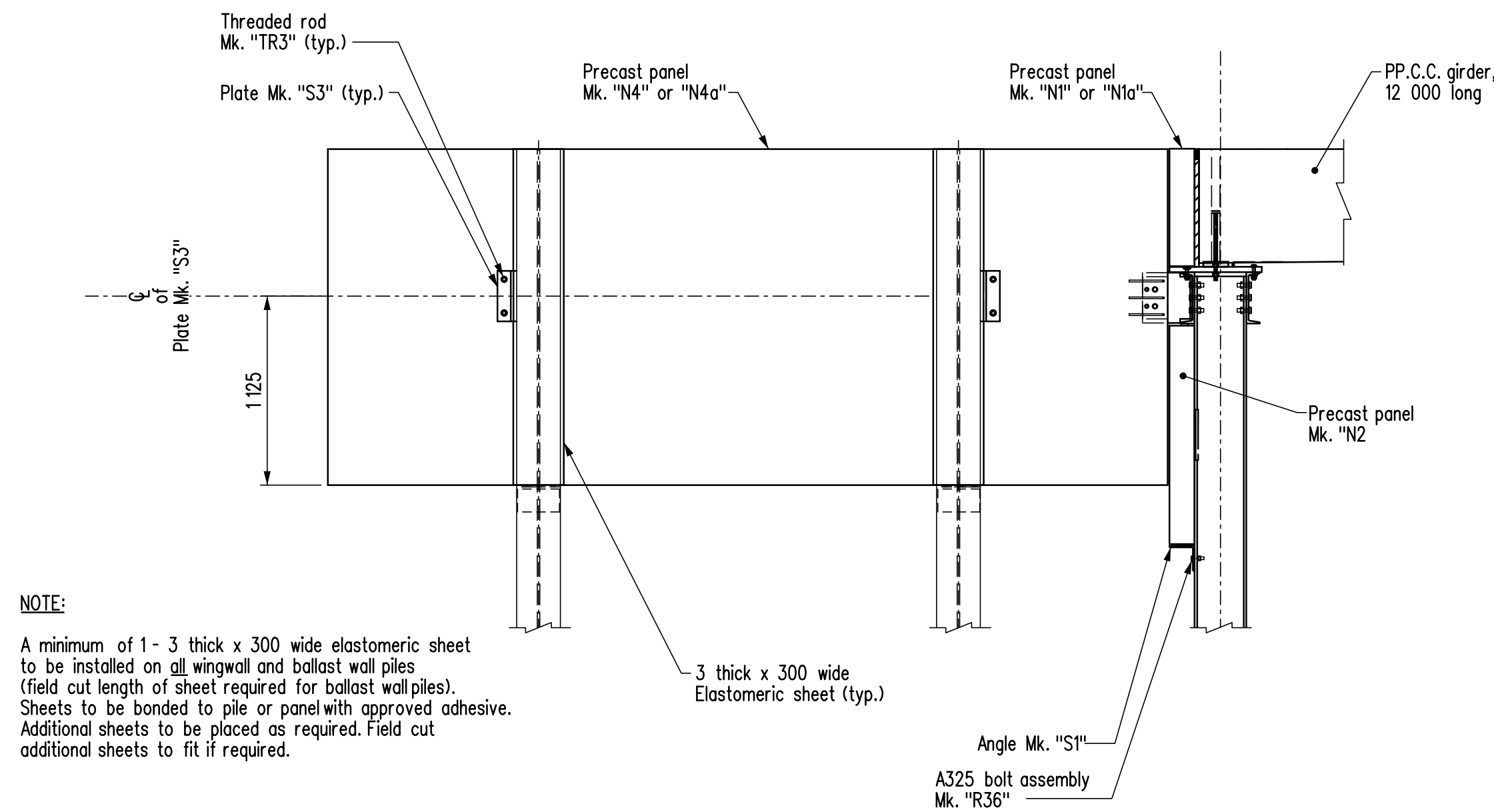
SECTION B-B

Scale 1:10

NOTES:

- For Section "B-B" and DETAIL "B" see Sheet No.
- For "BILL OF MISCELLANEOUS METAL" see Sheet No.
- The Contractor shall field drill 22 # holes in the precast panels for threaded rods Mk. "TR3". Should rebar be encountered, abandon hole, patch and drill in new location. Rebar locations are marked on the panels by the Panel Fabricator.
- Back faces of the upper and lower ballast walls shall be aligned in the same vertical plane.
- The Contractor shall ensure that the upper ballast walls are placed with the edge 5mm from \hat{Q} of roadway.

REVISIONS				ASSEMBLY DETAILS			
DATE	BY	DESCRIPTION		Manitoba Infrastructure Water Management and Structures		RELEASED FOR CONSTRUCTION	
DESIGN SEAL		RECORD SEAL				BY: _____	
PLACE ENGINEERS ELECTRONIC SEAL HERE				DESIGN BY: <u> </u> B.A.N. CHECKED: <u> </u> DETAILS BY: <u> </u> K.P. CHECKED: <u> </u>		SCALE: 1 : 30 SHEET No. <u> </u> 6 or as shown SITE No. <u> </u>	

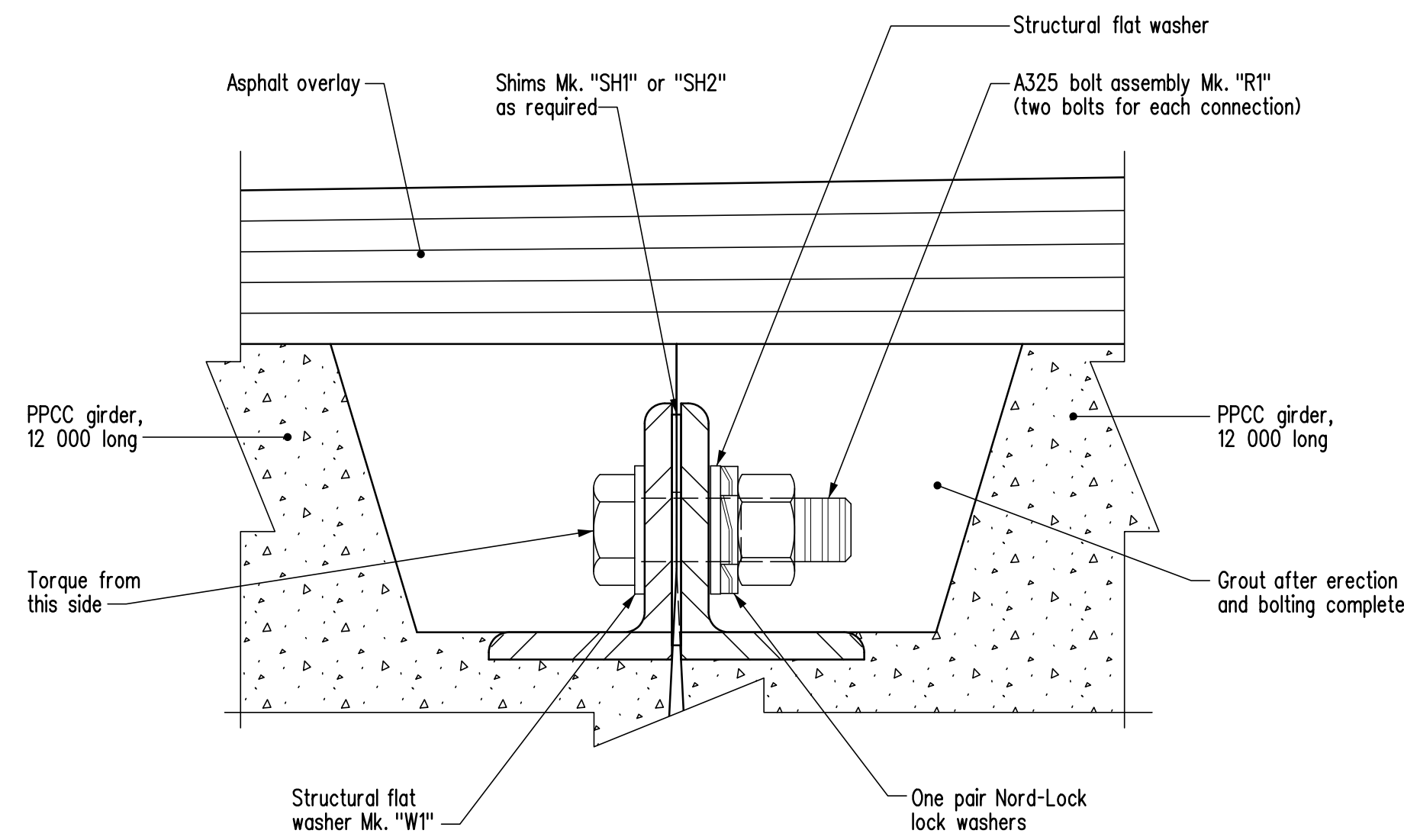


NOTE:

A minimum of 1 - 3 thick x 300 wide elastomeric sheet to be installed on all wingwall and ballast wall piles (field cut length of sheet required for ballast wall piles). Sheets to be bonded to pile or panel with approved adhesive. Additional sheets to be placed as required. Field cut additional sheets to fit if required.

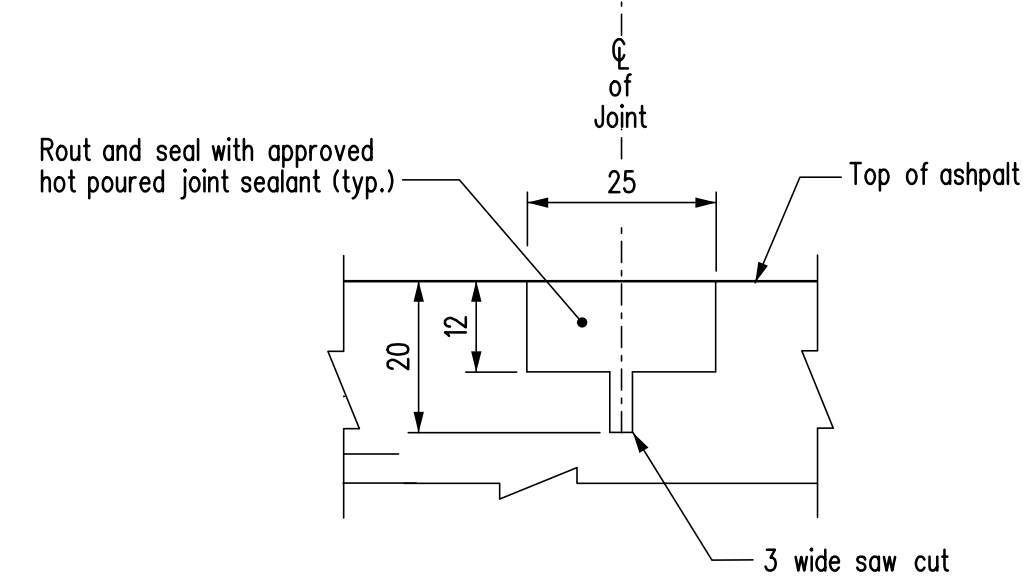
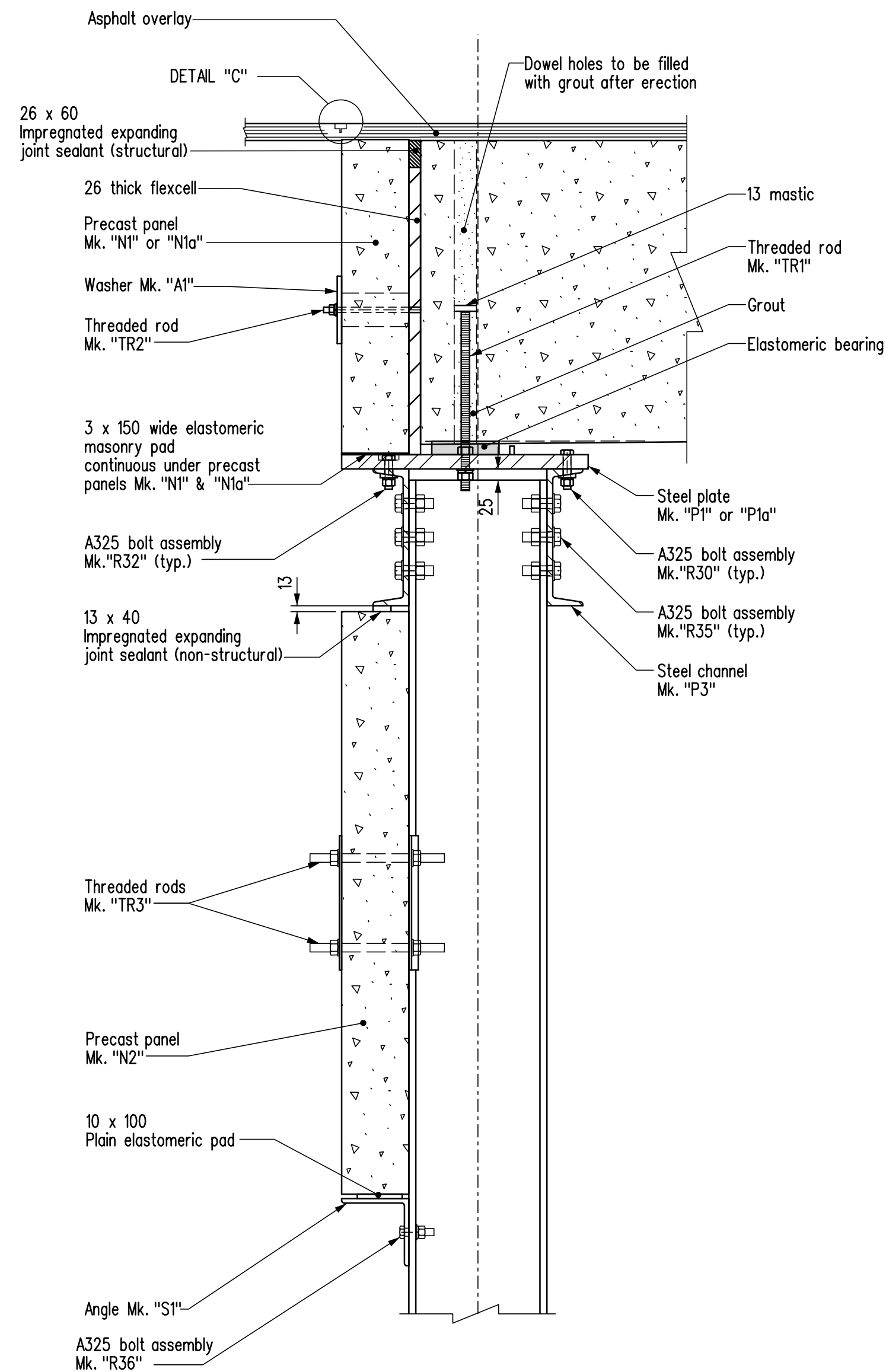
PART SIDE ELEVATION

Bridge railing not shown for clarity



DETAIL OF LATERAL CONNECTION ANGLE

Scale 1:2



DETAIL "C"

Scale 1:1

SECTION AT ABUTMENT

Scale 1:10

NOTES:

1. RE-BOLTING

a) GIRDER LATERAL CONNECTION

- Bolts Mk. "R1" - c/w one F436 hardened washer, one structural plate washer Mk. "W1", one pair Nord-Lock washers and one Grade DH heavy hex. nut.

b) STEEL CAP

- Bolts Mk. "R30" - One F436 hardened washer, one hardened bevel washer and one Grade DH heavy hex. nut.
- Bolts Mk. "R32" - One hardened bevel washer and one Grade DH heavy hex. nut.
- Bolts Mk. "R35" - Two F436 hardened washers and one Grade DH heavy hex. nut.

c) PRECAST PANELS

- Bolts Mk. "R36" - Two F436 hardened washers and one Grade DH heavy hex. nut.
- Bolts Mk. "R34" - One F436 hardened washer and one structural plate washer Mk. "A2", no nuts.
- Threaded rod Mk. "TR2" - One standard flat washer, one structural lock washer, structural plate washer Mk. "A1" and one stainless steel hex. nut.
- Threaded rod Mk. "TR3" - two Filler plates Mk. "S4", one structural lock washer, two standard flat washers and two hex. nuts, Filler plate Mk. "S5" if required.

d) GIRDER TO STEEL CAP

- Threaded rod Mk. "TR1" - one standard flat washer and structural lock washer and two hex. nuts.

e)

High strength bolts shall be tightened by the turn-of-nut method as per Bridge Specifications. Ensure nuts are lubricated prior to bolting.

f)

Fill counter bored holes with mastic filler after tightening bolts.

2.

When grouting dowel holes in girders, ensure that there is no grout between bottom of girder and bearing plate.

3.

Apply galvalloy to all field welds & areas where galvanizing has been damaged.

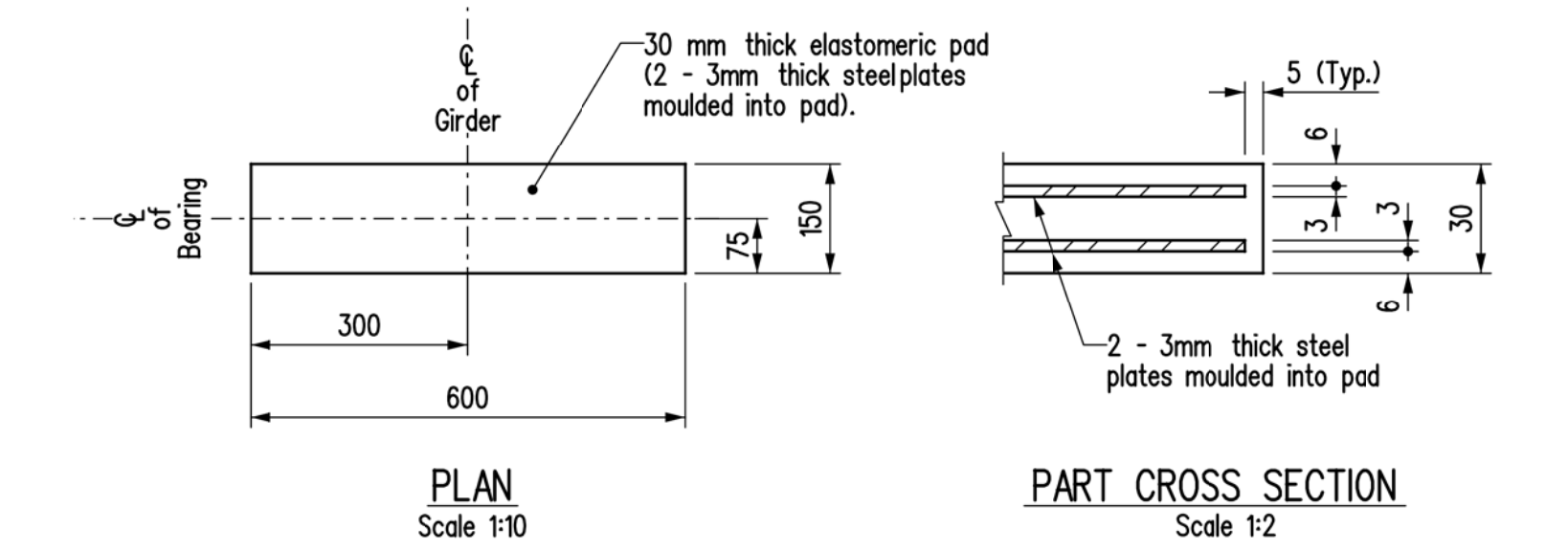
4.

Impregnated expanding joint sealant shall be installed as per manufacturer's recommendations.

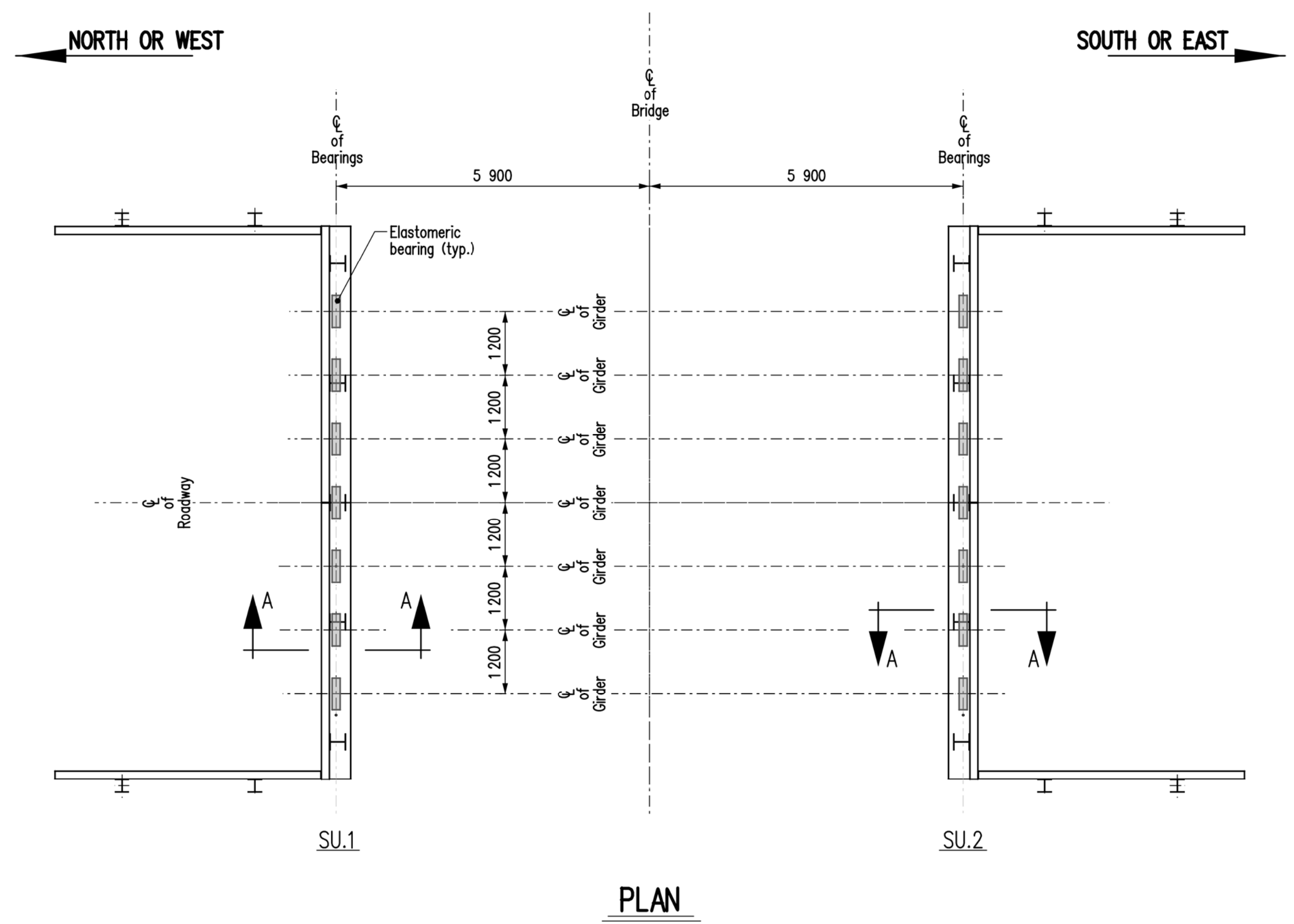
REVISIONS		ASSEMBLY DETAILS		
DATE	BY	DESCRIPTION		RELEASED FOR CONSTRUCTION BY:
		DESIGN SEAL	RECORD SEAL	
PLACE ENGINEERS ELECTRONIC SEAL HERE				EXECUTIVE DIRECTOR OF STRUCTURES DATE
DESIGN	BY: B.A.N.	SCALE: 1:30 SHEET No. 7		
CHECKED:				
DETAILS	BY: K.P.	or as shown SITE No. 000		
CHECKED:				

No.	LOCATION	DESCRIPTION	REMARKS
14	SU.1 - SU.2	Elastomeric bearings	As detailed

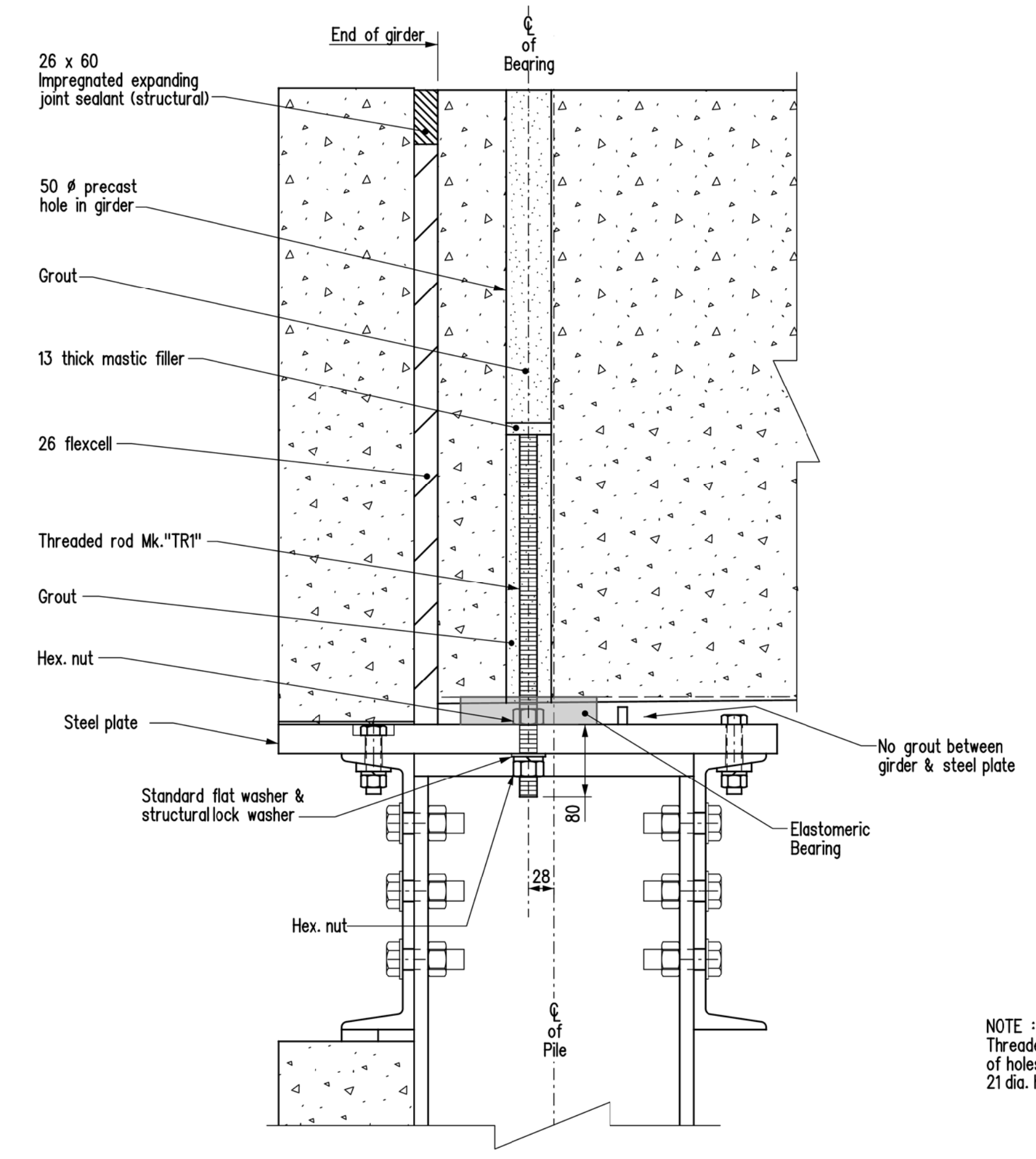
NOTE:
 1. Elastomer shall be natural rubber. Elastomer shall be AASHTO low temperature Grade 5 with a minimum shear modulus $G \geq 0.9 \text{ MPa}$ and a 60 durometer Shore A hardness.
 2. Internal steel reinforcing plates for laminate bearings shall be rolled mild steel with a minimum yield strength of 300 Mpa.



ELASTOMERIC BEARINGS



PLAN



SECTION "A-A"

Threaded rods at SU.1 & SU.2. See sheet No. 6 for layout. Scale 1:5

NOTE :
 Threaded rods Mk. "TR1", set exactly on centreline of holes in precast girders and grouted. Field drill 21 dia. hole in steel plate to match girders.

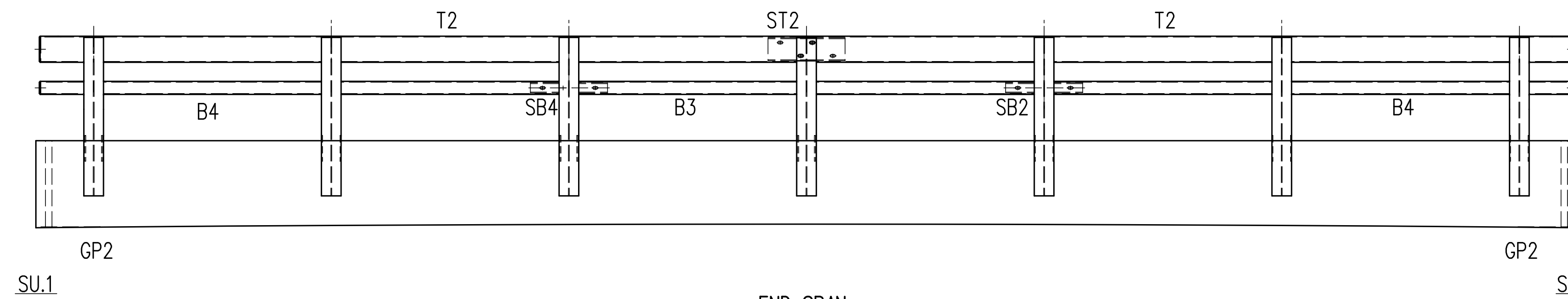
NOTES:

Re: Girder Erection Operations Behind Abutment Ballast Walls

- Surcharge loading on the backfill resulting from girder erection operations shall be minimized near the precast concrete ballast walls and wingwalls.
- Where possible, girder erection equipment shall be positioned such that there are no surcharge loads behind the back face of the precast panels within a distance equal to the depth of backfill to the bottom of the panels at the time of girder erection.
- Should the Contractor propose to encroach on this zone, the following requirements must be satisfied:
 - Submit a girder erection procedure for approval outlining type, configuration, weights and locations of equipment including expected tipping forces on crane outriggers, etc.
 - Perform all precautionary measures outlined by the Department as a result of that submission.
 - All surcharge loads encroaching in this zone must be distributed over an area not less than 2.0 m².

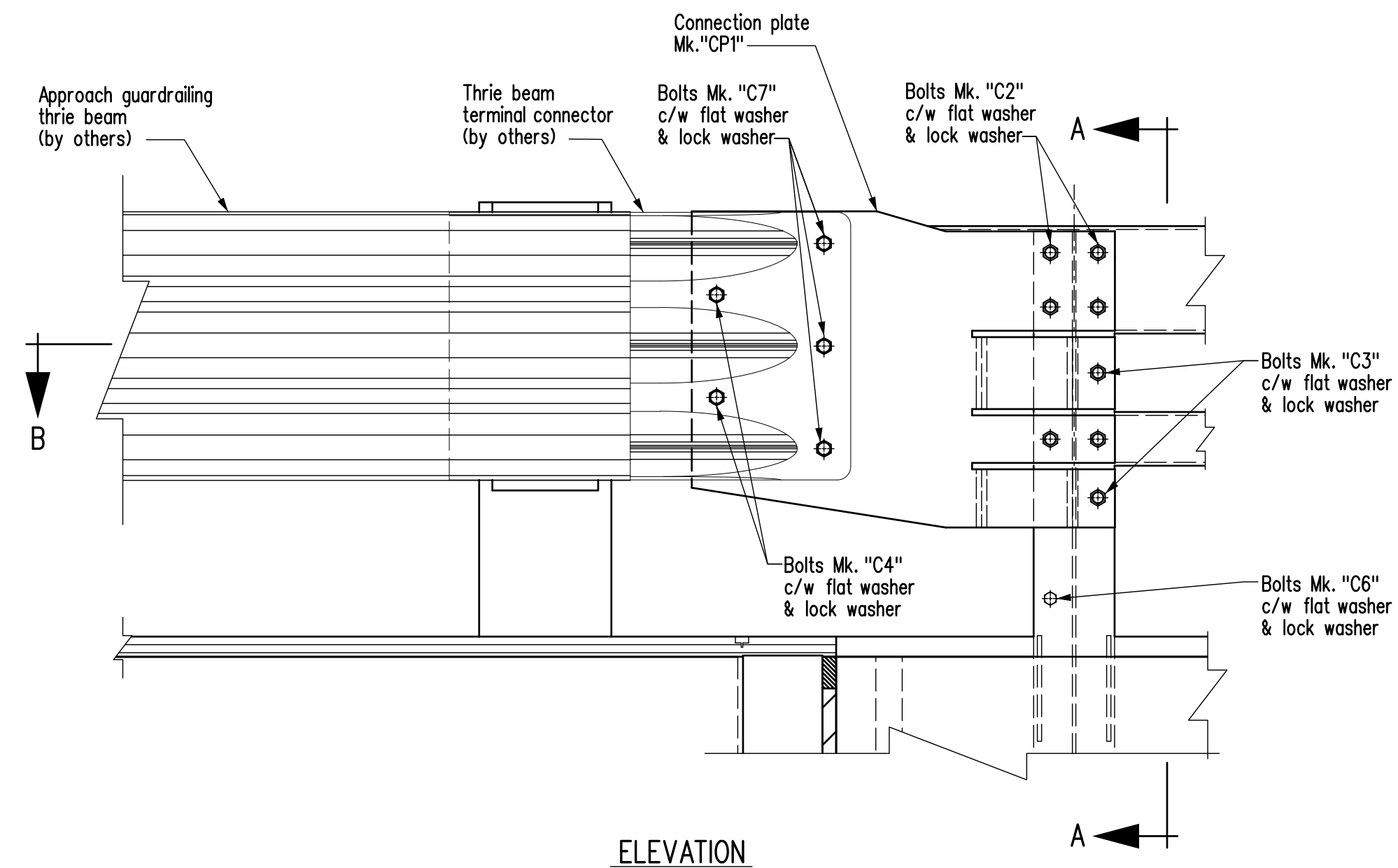
REVISIONS		BEARING AND ERECTION DETAILS	
DATE	BY		
		RELEASED FOR CONSTRUCTION BY: _____ DATE: _____	
		DESIGN BY: <u>B.A.N.</u>	EXECUTIVE DIRECTOR OF STRUCTURES DATE: _____
		CHECKED: _____	SCALE: 1:75 SHEET No. <u>9</u>
		DETAILS BY: <u>K.P.</u>	or as shown SITE No. <u>5555</u>
		CHECKED: _____	

PLACE ENGINEERS ELECTRONIC SEAL HERE

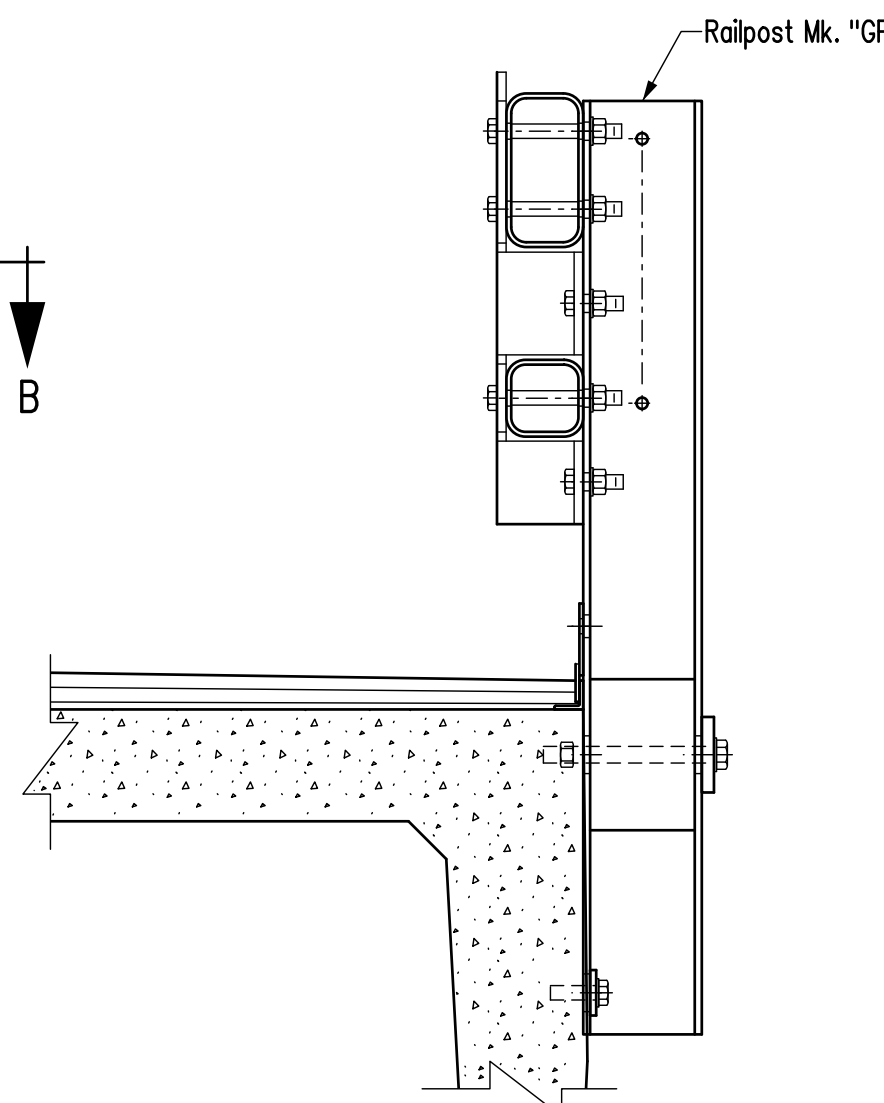


RAILS			SLEEVES		RAILPOSTS	
T2	B3	B4	ST2	SB2	GP1	GP2
4	2	4	2	4	10	4

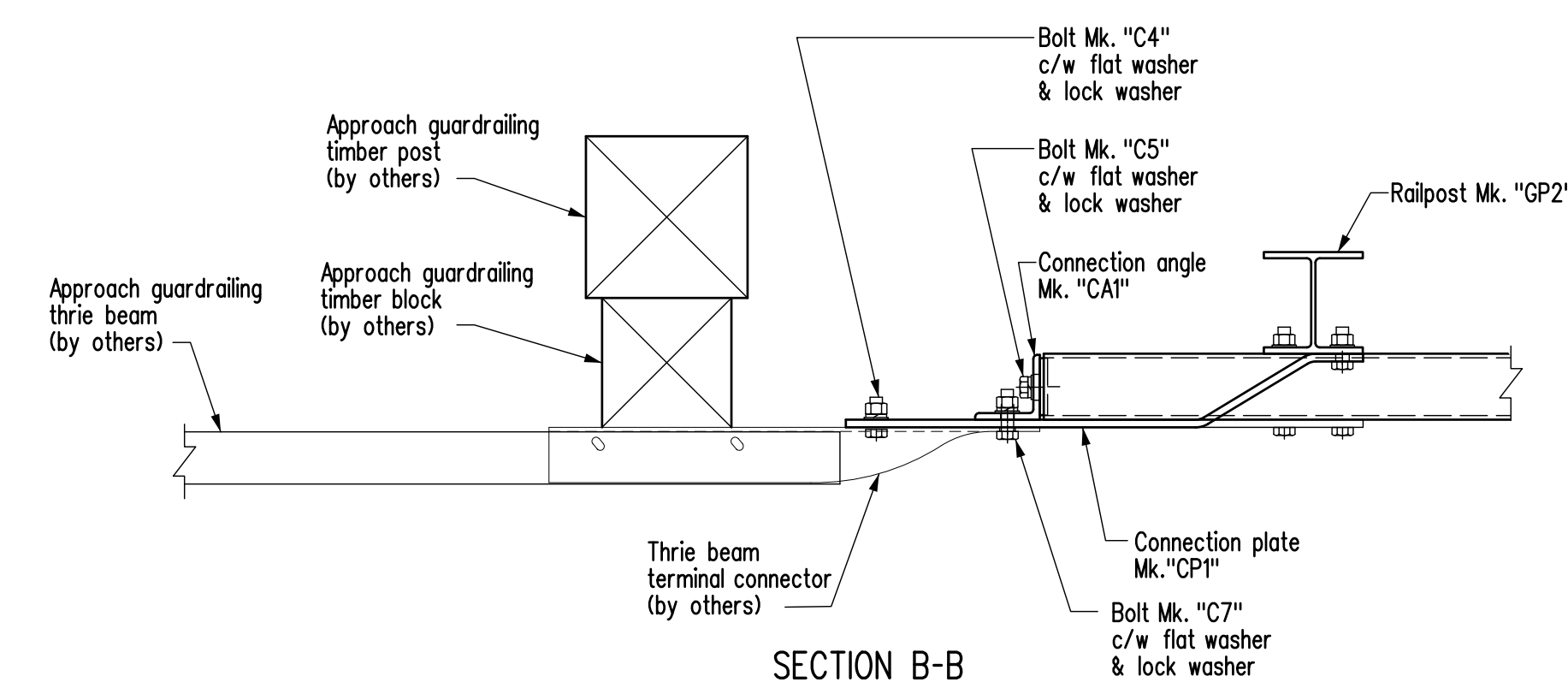
RAILING LAYOUT
Not to Scale



ELEVATION

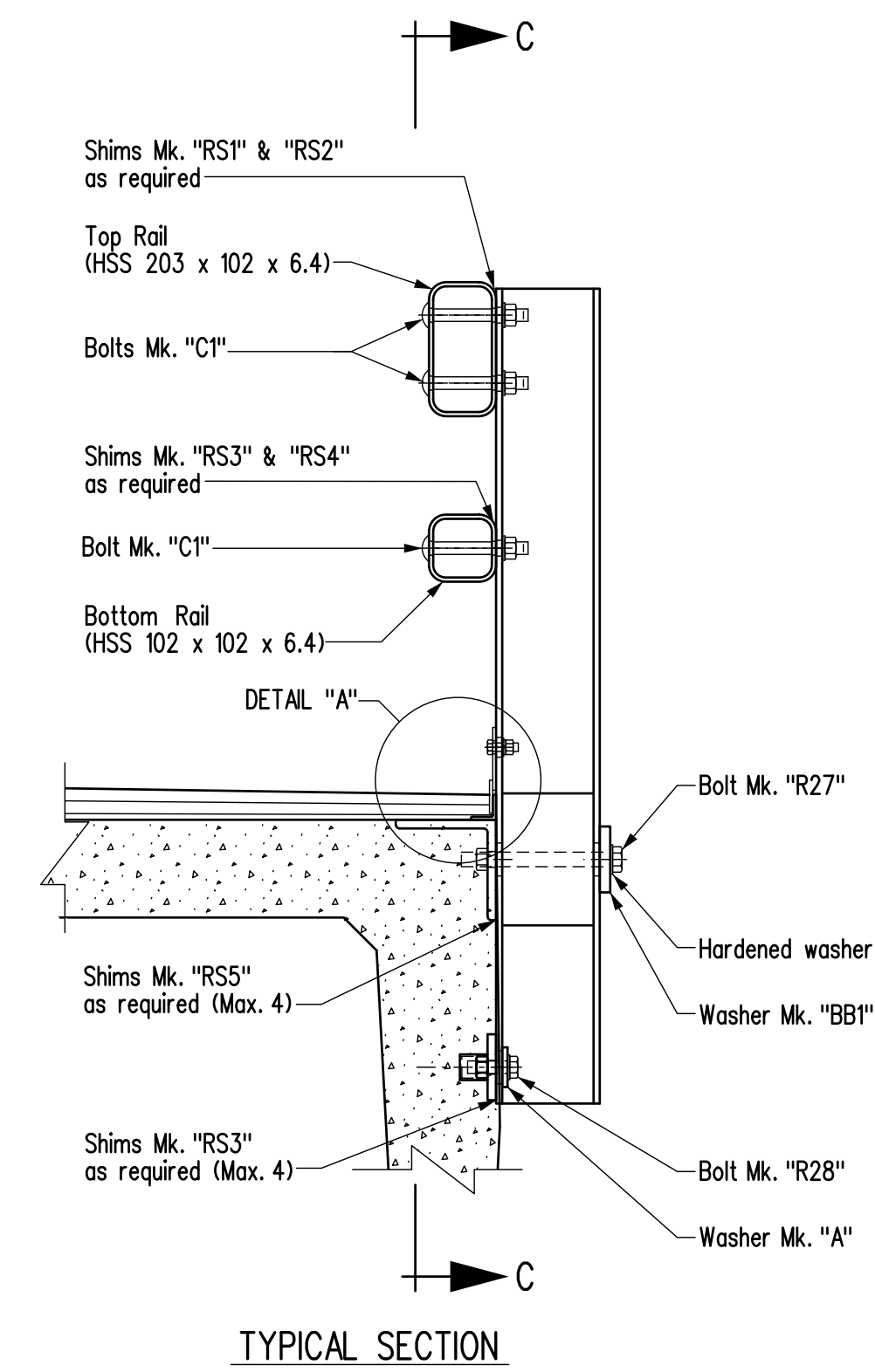


SECTION A-A

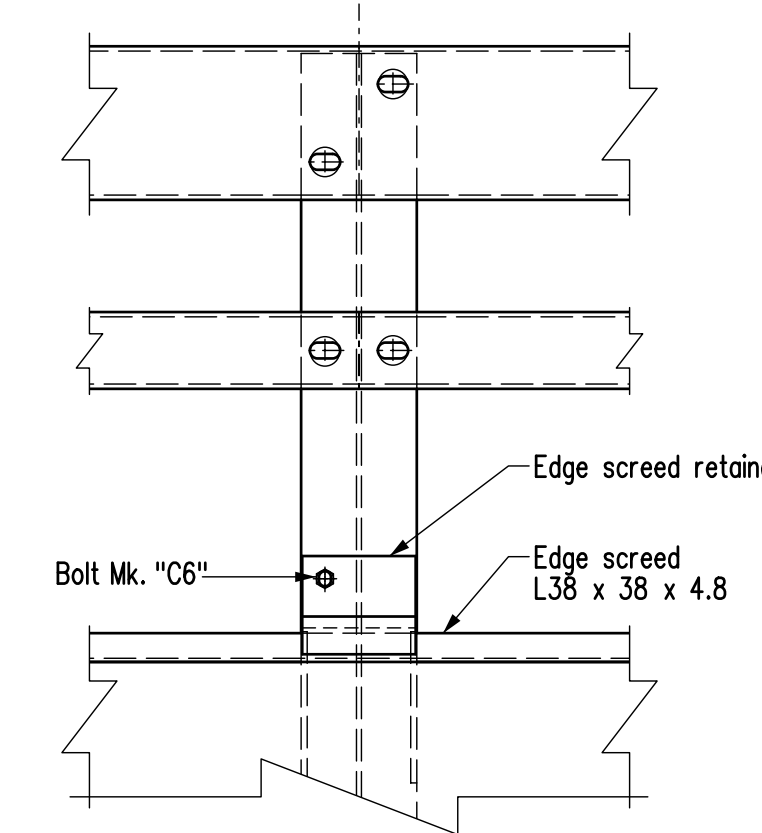


SECTION B-B

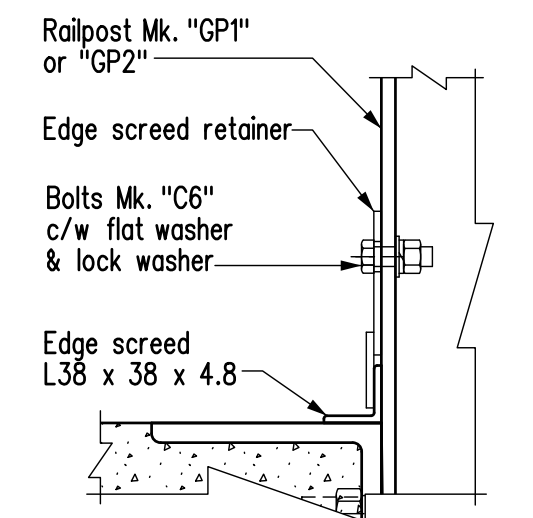
APPROACH RAIL CONNECTION DETAILS



TYPICAL SECTION



ELEVATION C-C
Showing edge screed installation detail



DETAIL "A"
Showing edge screed installation detail
Scale 1:5

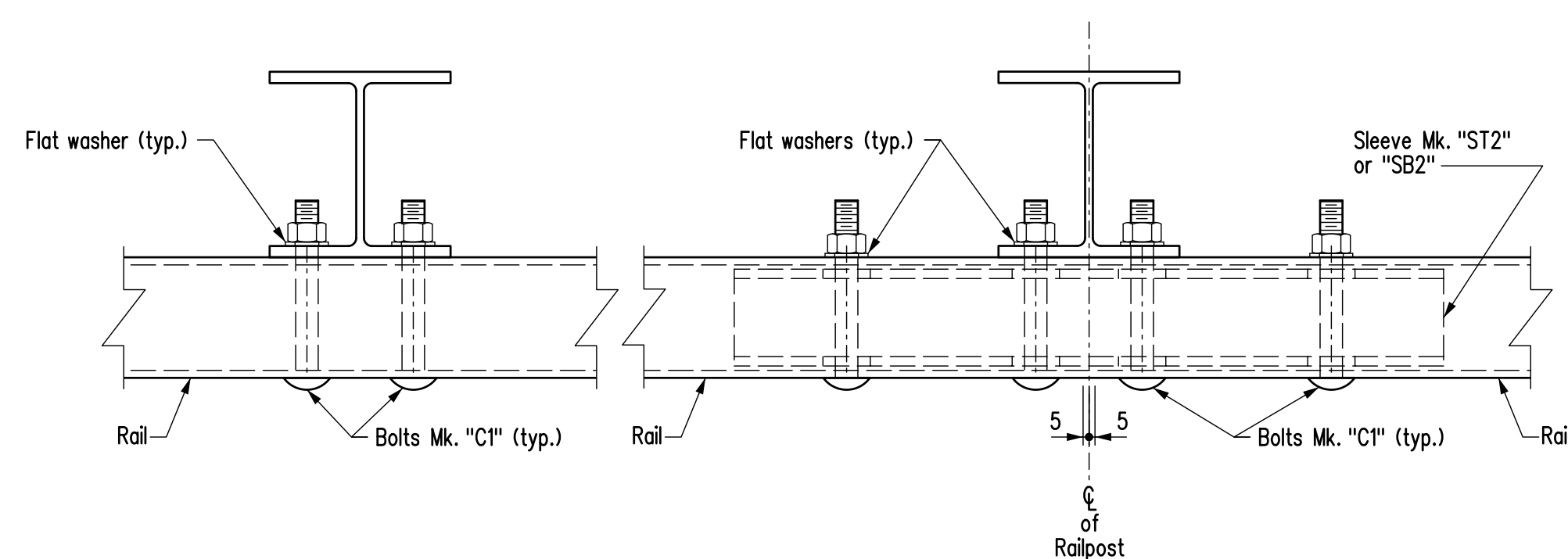
RAILPOST ERECTION DETAILS

NOTES:

- All railposts shall be Mk. "GP1" unless noted otherwise.
- This sheet to be read in conjunction with Sheets & .

NOTES:

- High strength bolts Mk. "R27" & "R28" shall be tightened by turn-of-nut method as per Specification 1061. These bolts to be supplied by the Girder Fabricator. For quantities see Bill of Miscellaneous Metal on Girder sheet.
- High strength bolted connection may be shimmed to a maximum of 12 mm with shims Mk. "RS3" & "RS4".



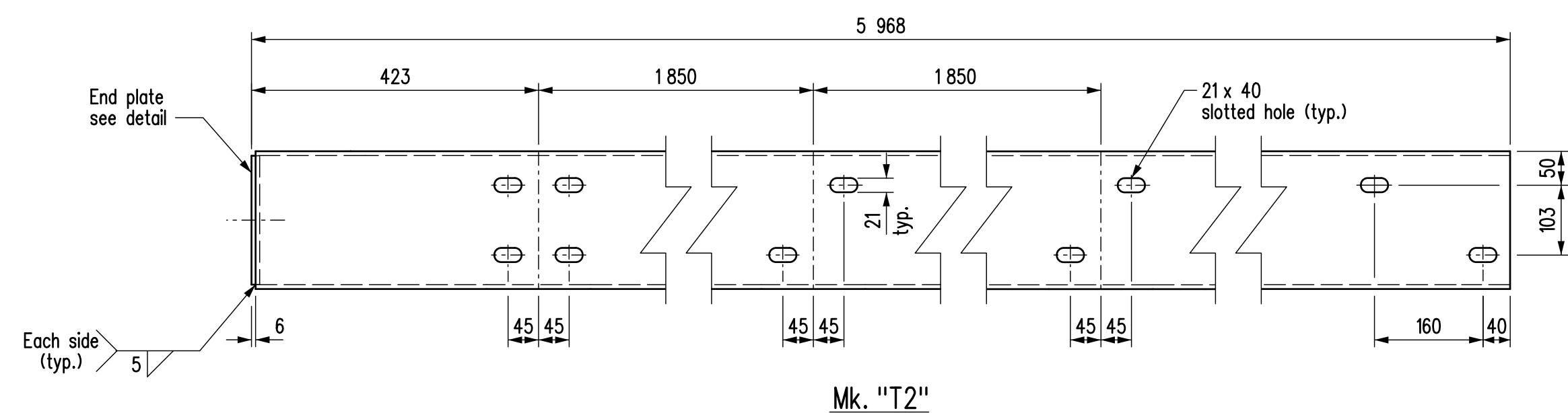
TYPICAL OF CONTINUOUS RAILS

RAIL END CONNECTION

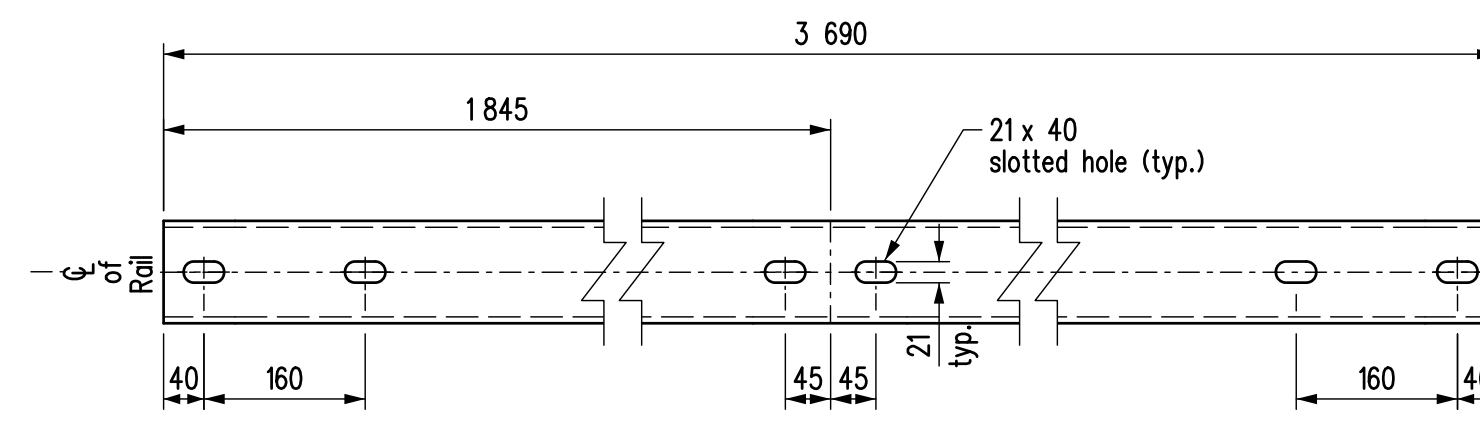
RAILING ERECTION DETAILS

Scale 1:5

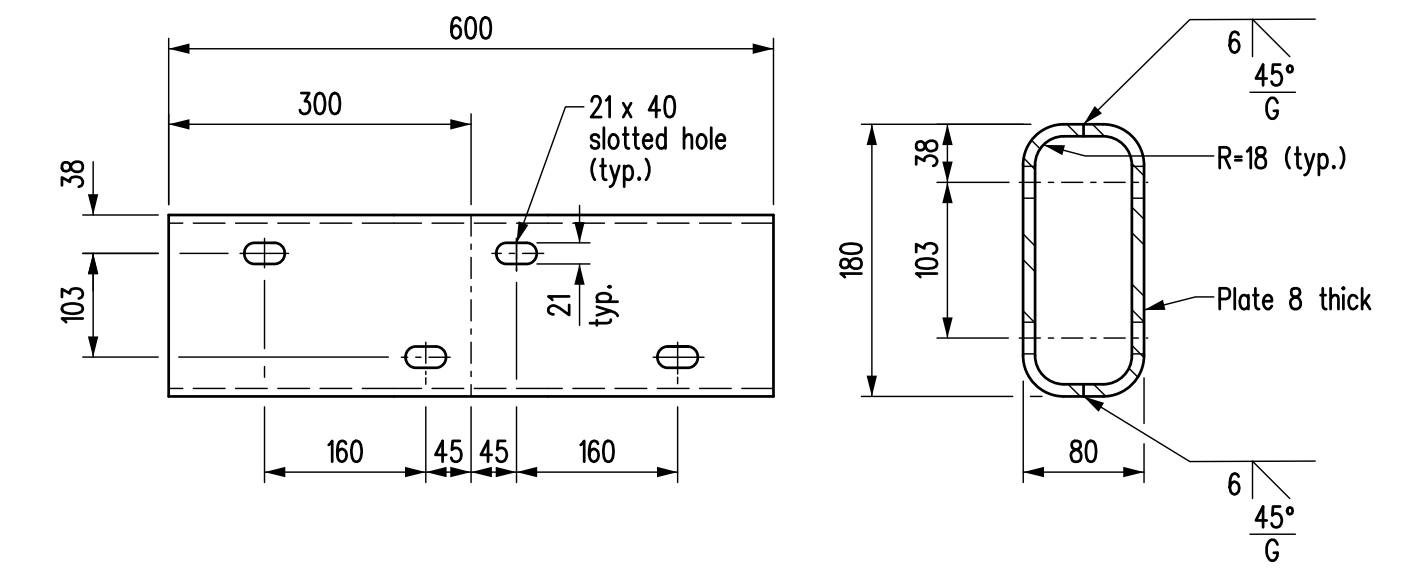
REVISIONS		RAILING LAYOUT AND DETAILS	
DATE	BY	DESCRIPTION	
		DESIGN SEAL	RECORD SEAL
		Infrastructure Water Management and Structures	
		BY: _____ CHECKED: _____	RELEASED FOR CONSTRUCTION BY: _____ EXECUTIVE DIRECTOR OF STRUCTURES DATE: _____
		BY: _____ CHECKED: _____	SCALE: 1:10 SHEET No. _____ or as shown SITE No. _____



Mk. "T2"

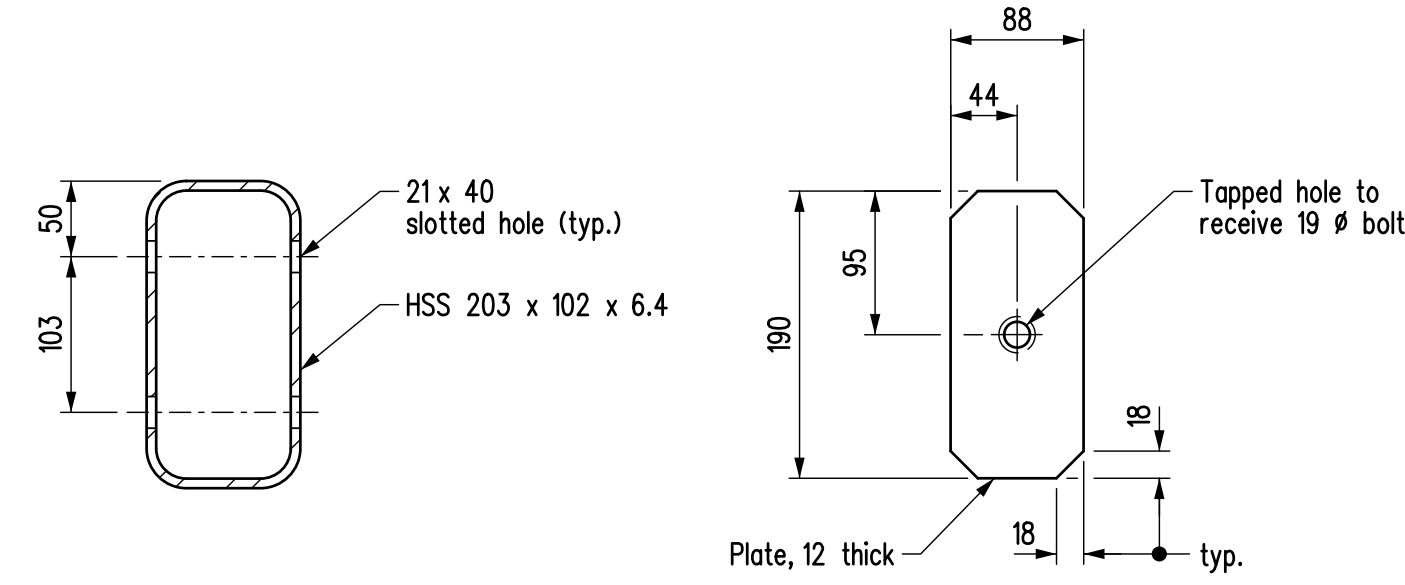


Mk. "B3"



Mk. "ST2"

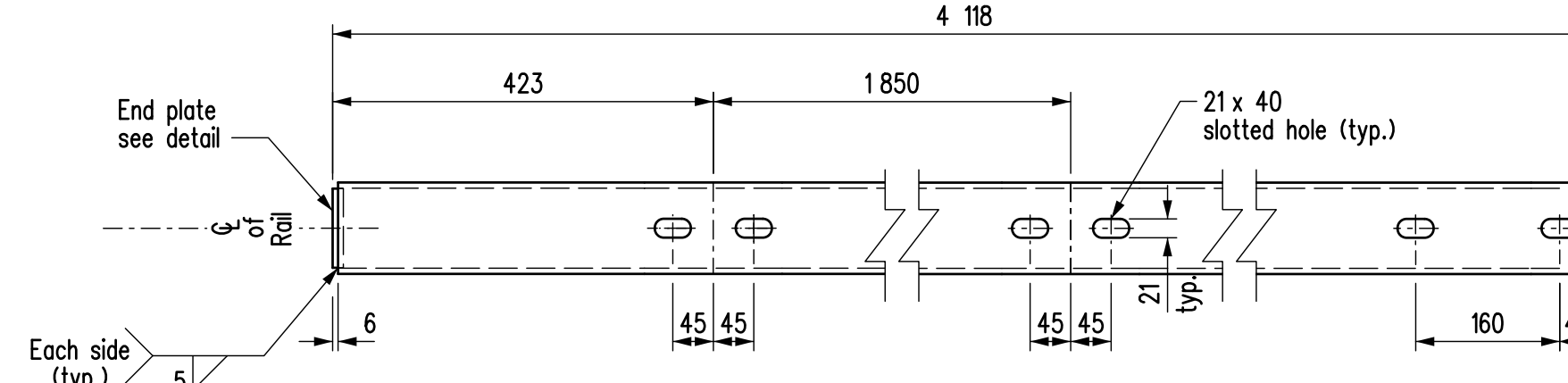
TYPICAL CROSS SECTION
Scale 1:5



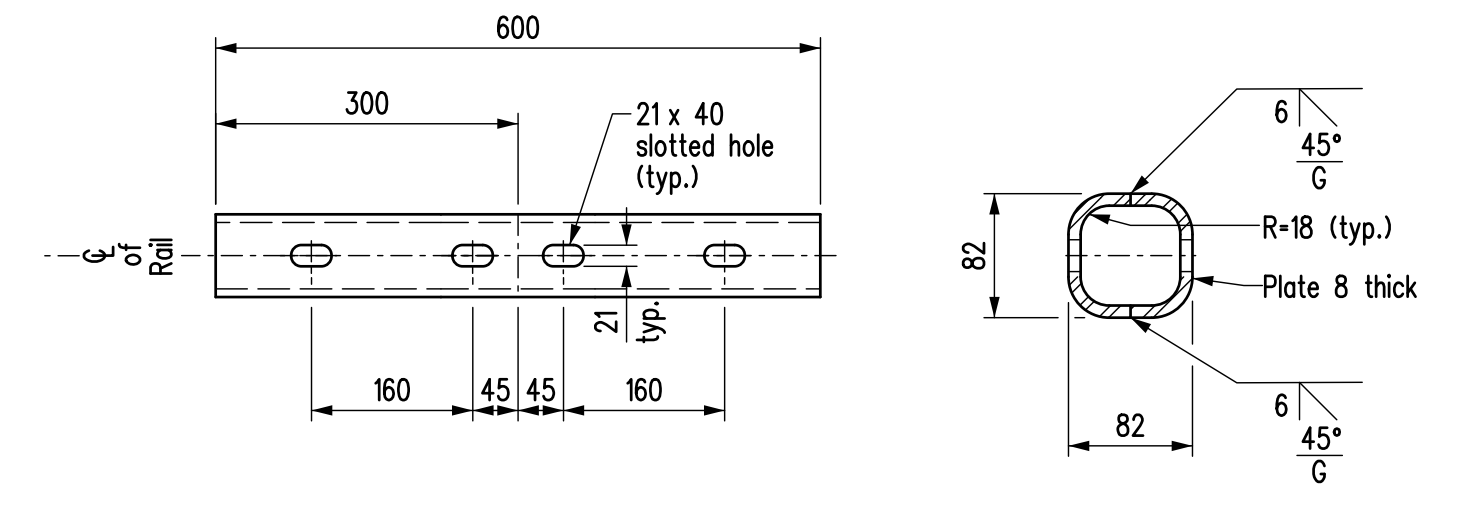
TYPICAL CROSS SECTION
Typical for rail Mk. "T2"
Scale 1:5

END PLATE
For rail Mk. "T2"
Scale 1:5

DETAILS OF TOP RAILS



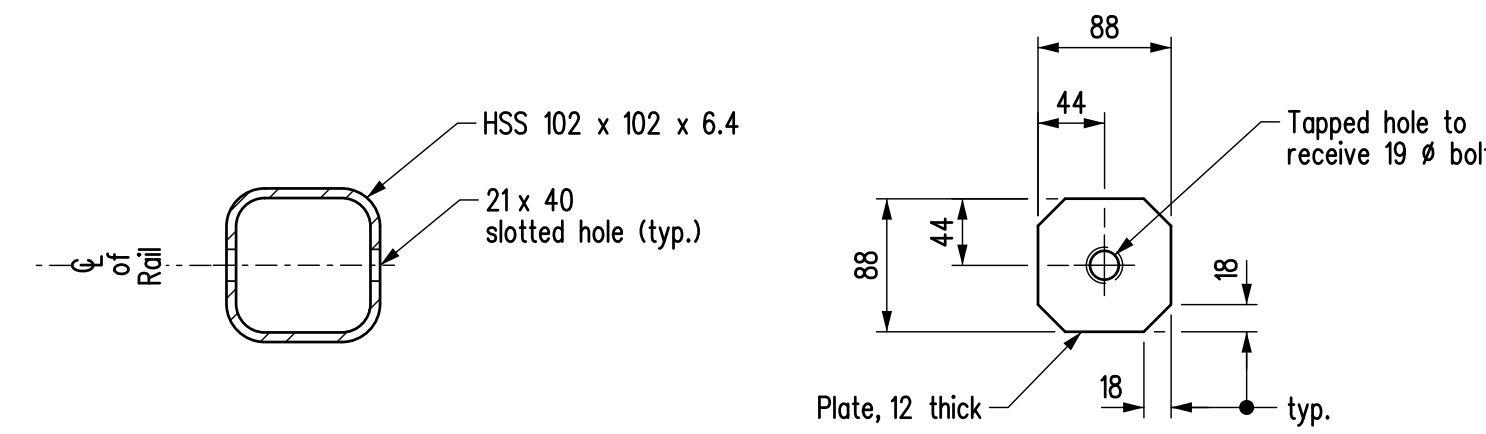
Mk. "B4"



Mk. "SB2"

TYPICAL CROSS SECTION
Scale 1:5

DETAILS OF SLEEVES



TYPICAL CROSS SECTION
Typical for rails Mk. "B3" & "B4"
Scale 1:5

END PLATE
For rail Mk. "B4"
Scale 1:5

DETAILS OF BOTTOM RAILS

NOTES:

1. It is imperative that all rail and sleeve holes in each pair of holes be opposite to each other.
2. The length of slotted holes shall not be less than shown.
3. The width and height of the sleeves shall not exceed the dimensions shown.
4. All dimensions are in millimeters (mm).

REVISIONS		RAILING DETAILS		
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:	

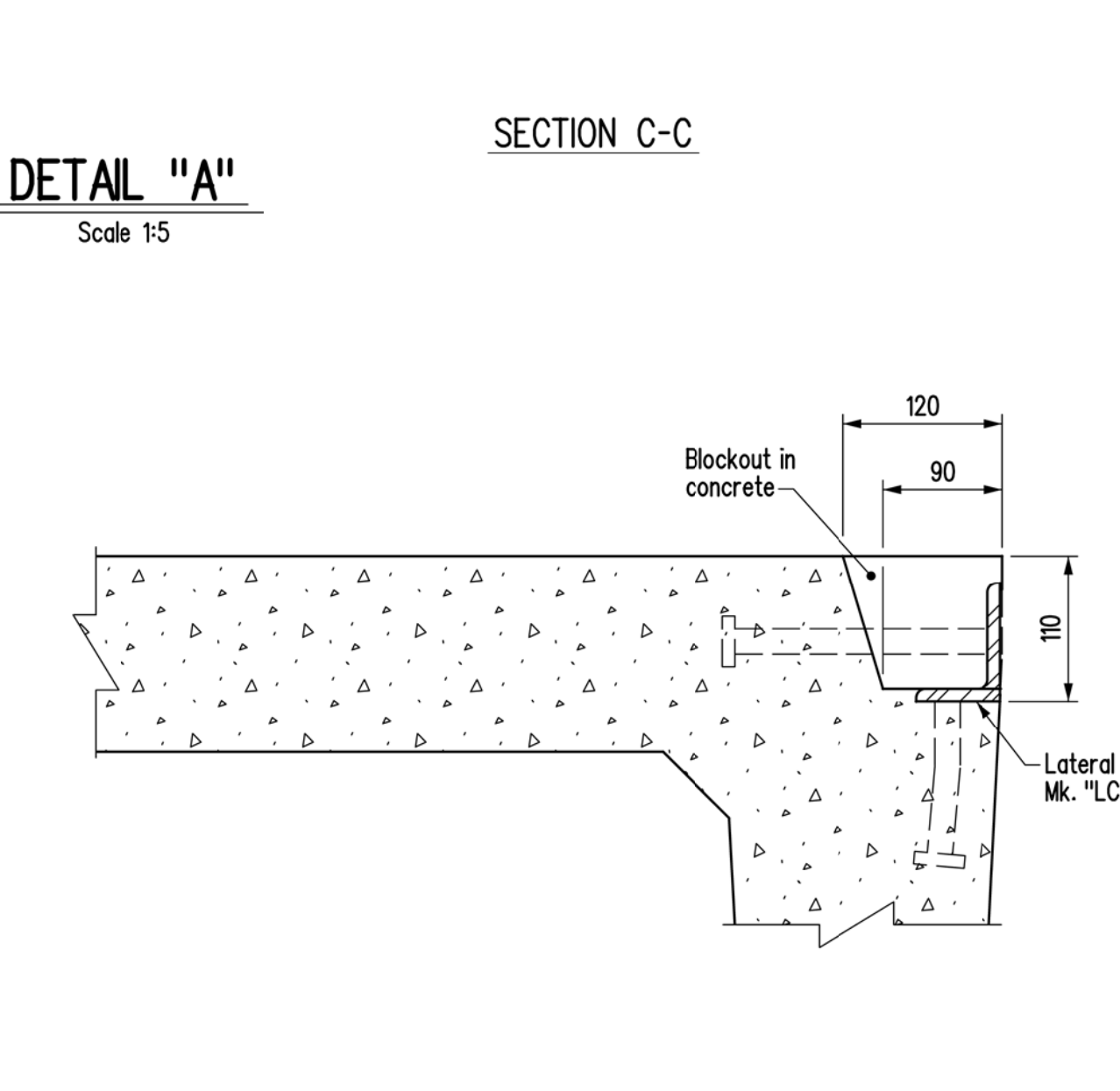
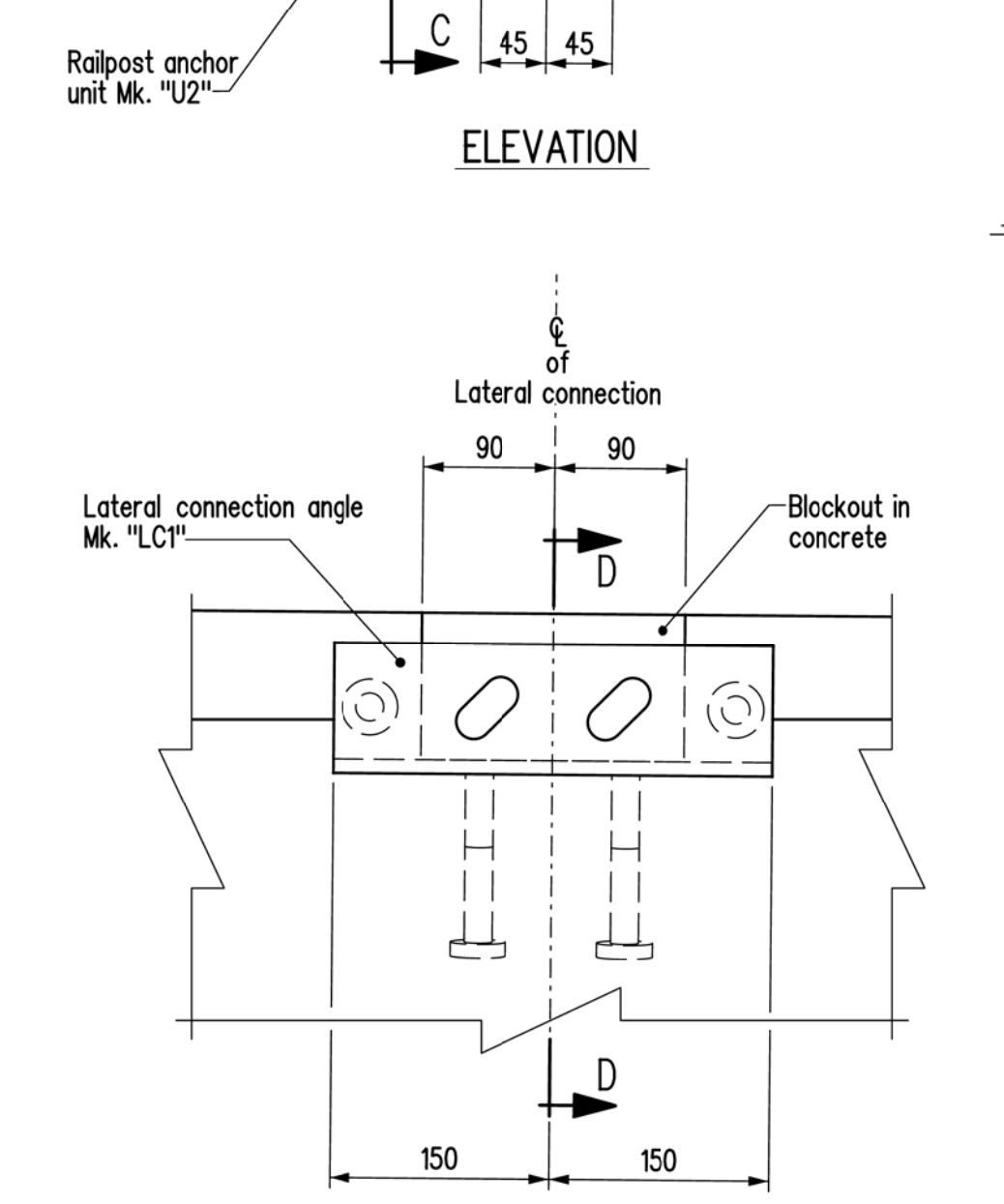
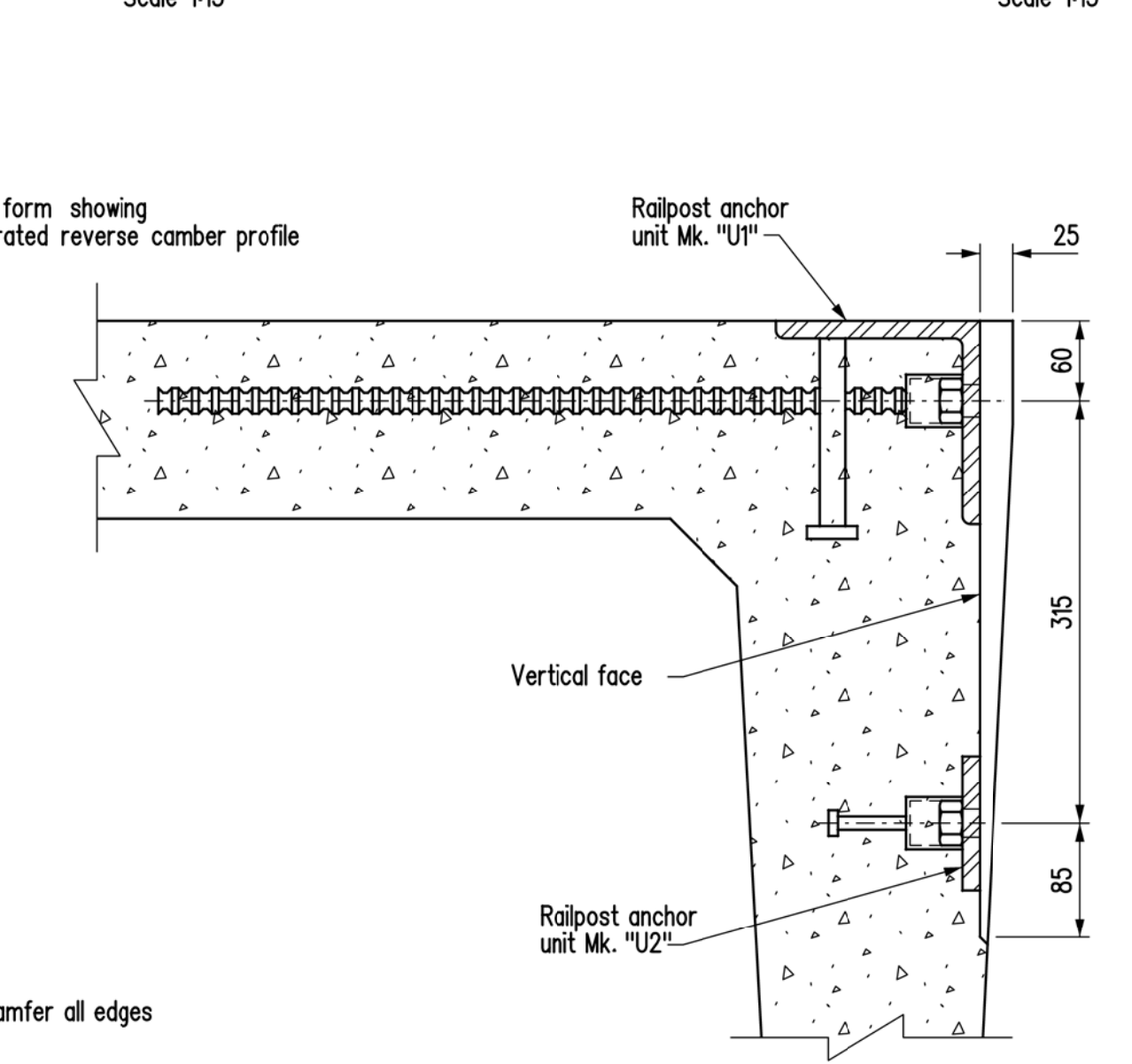
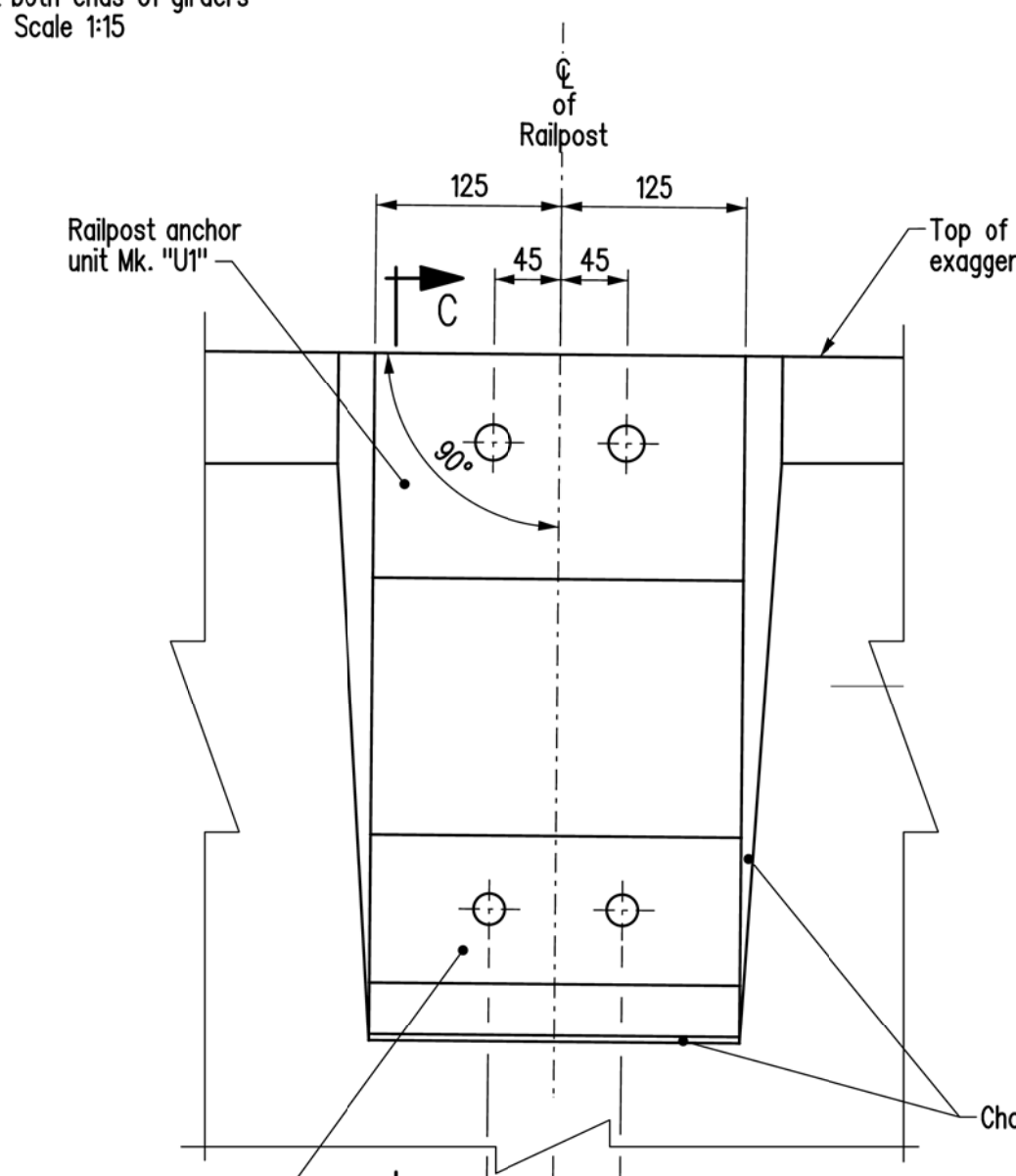
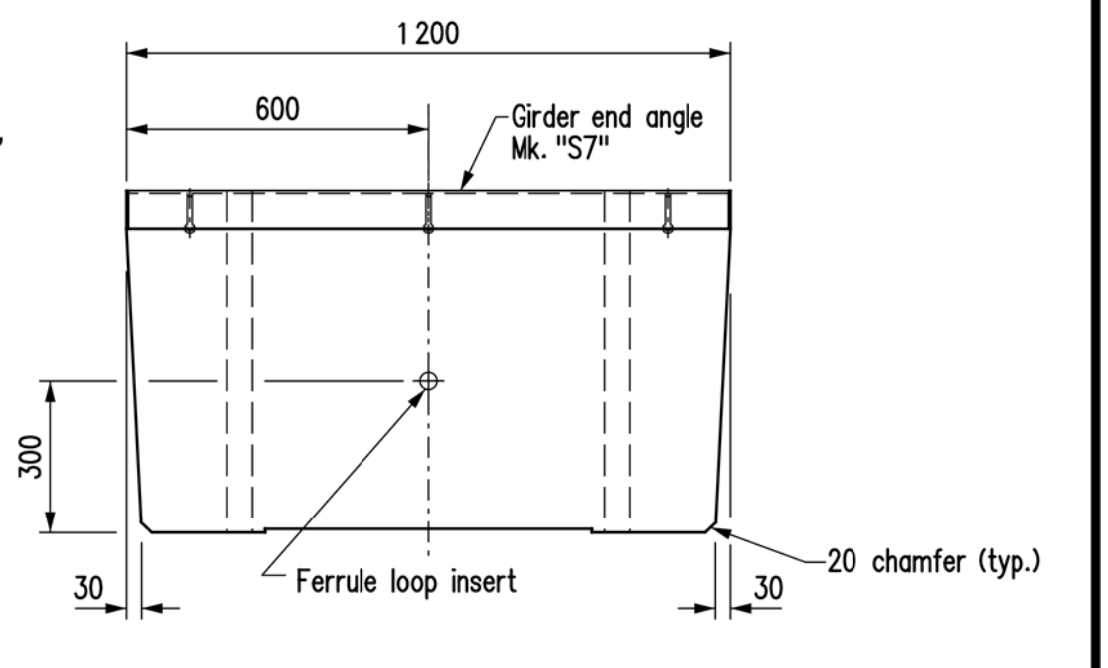
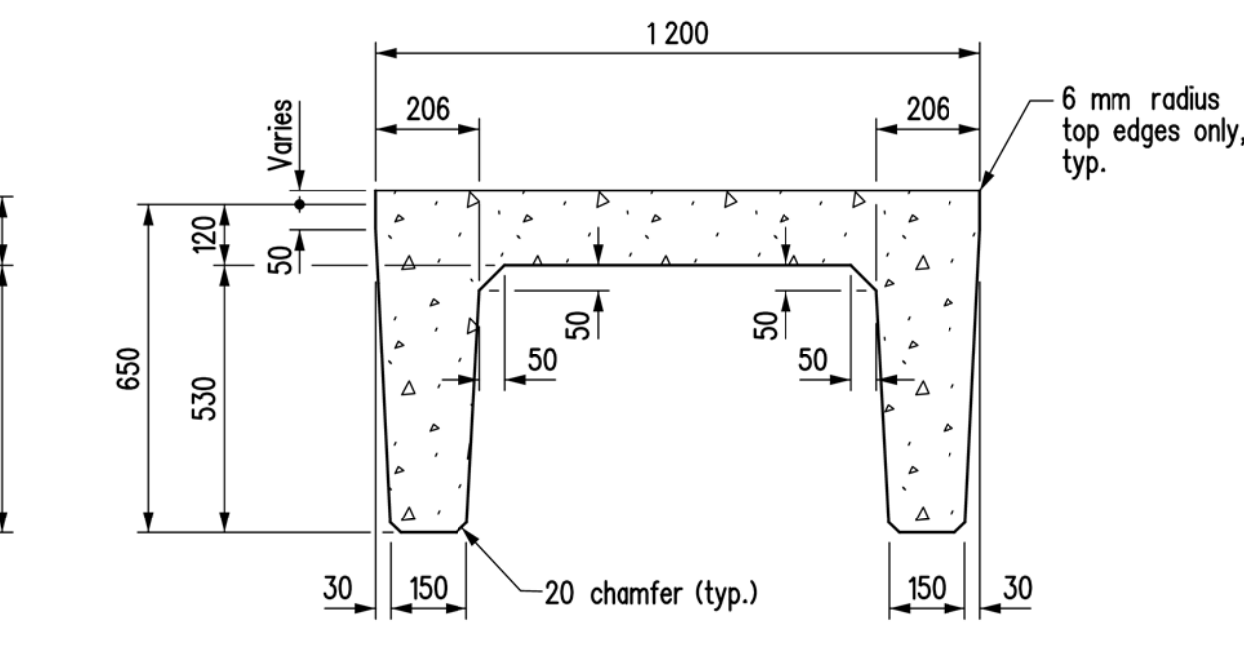
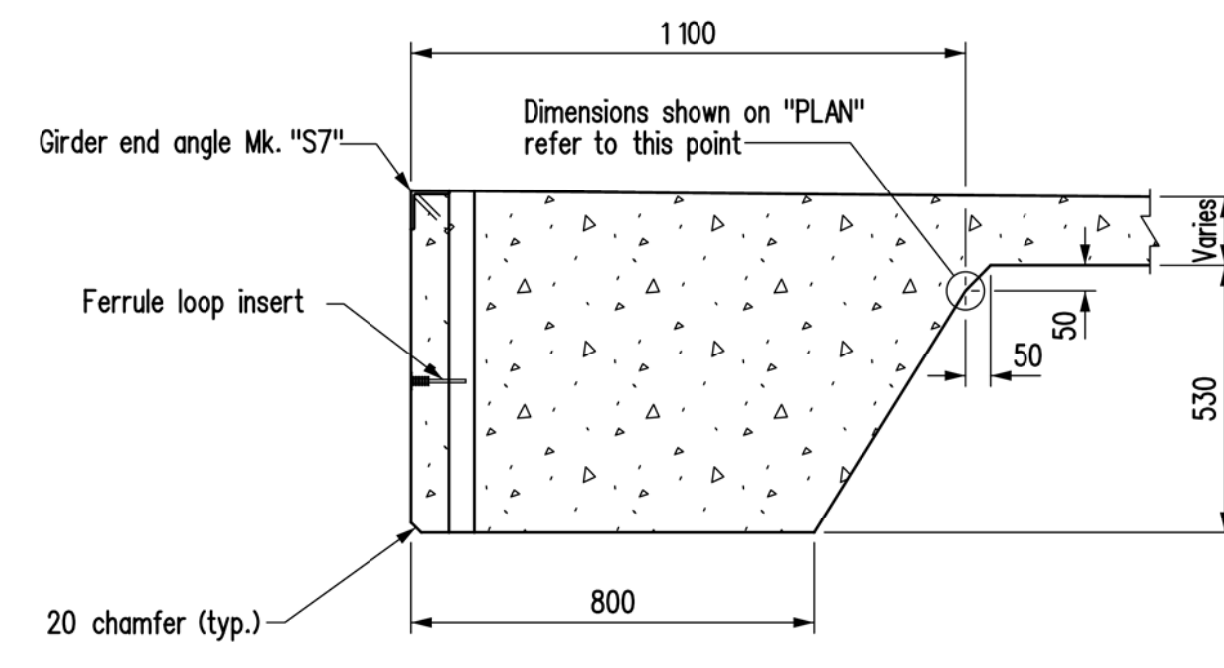
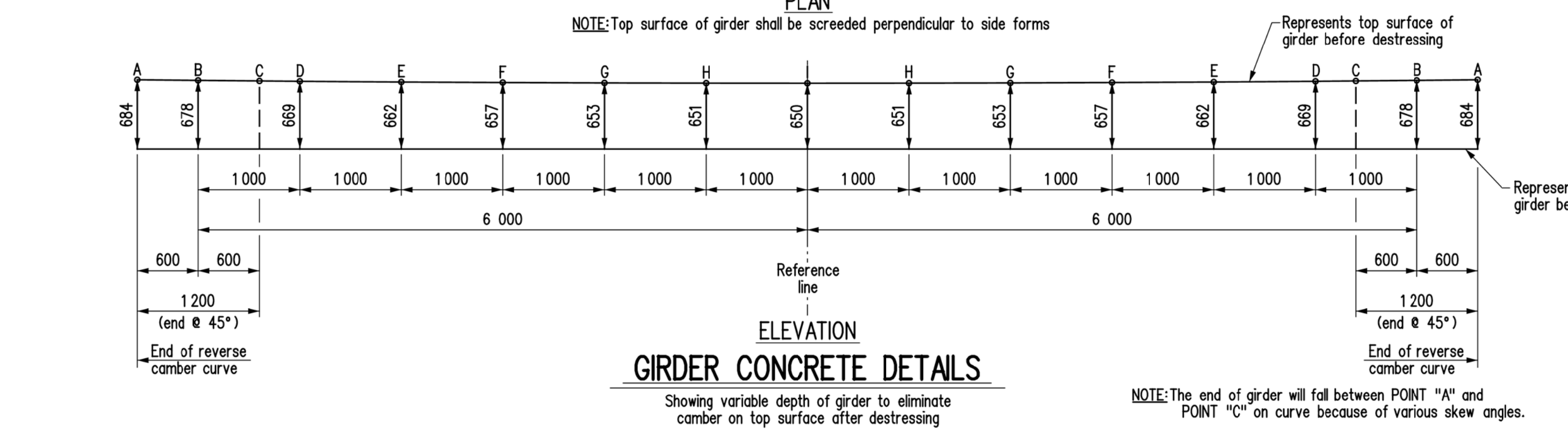
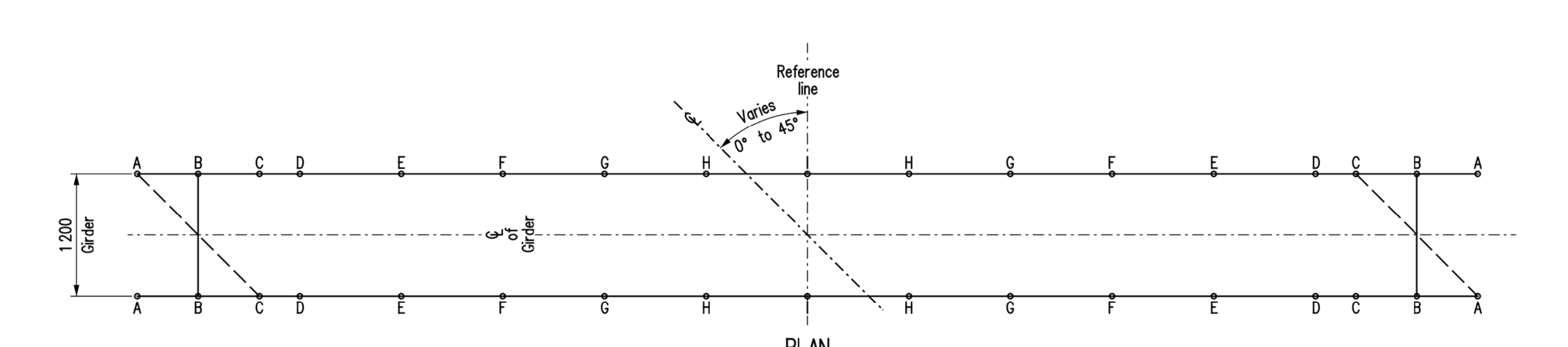
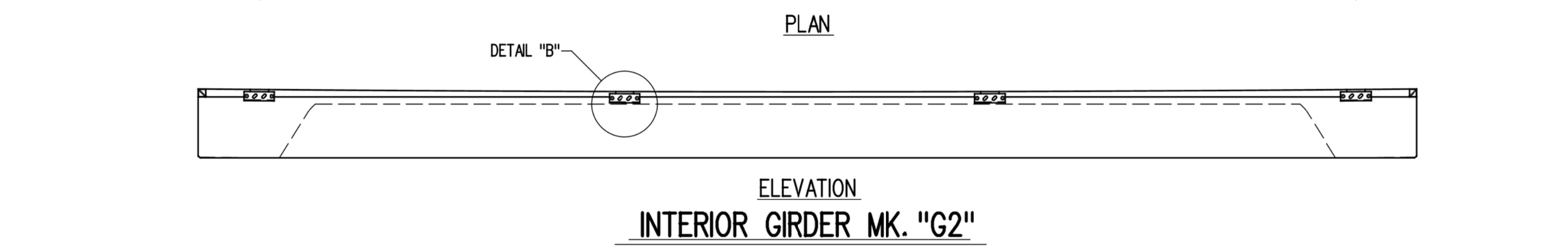
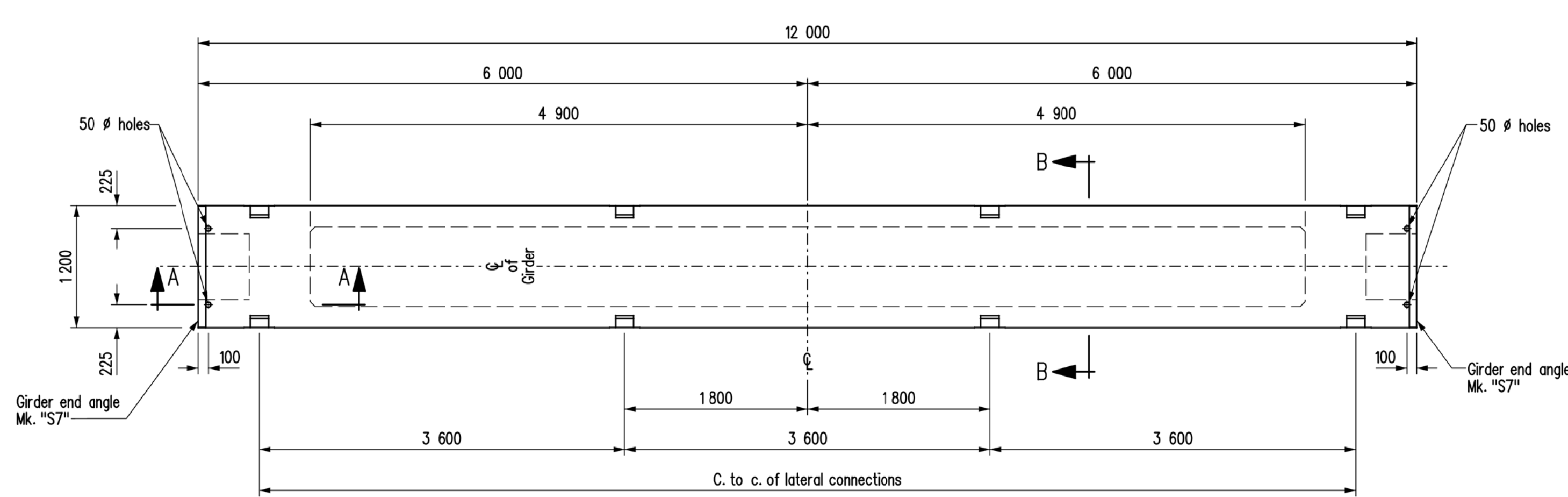
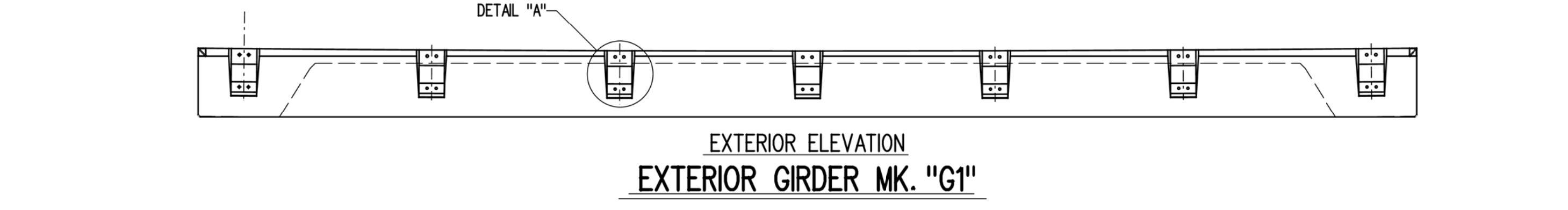
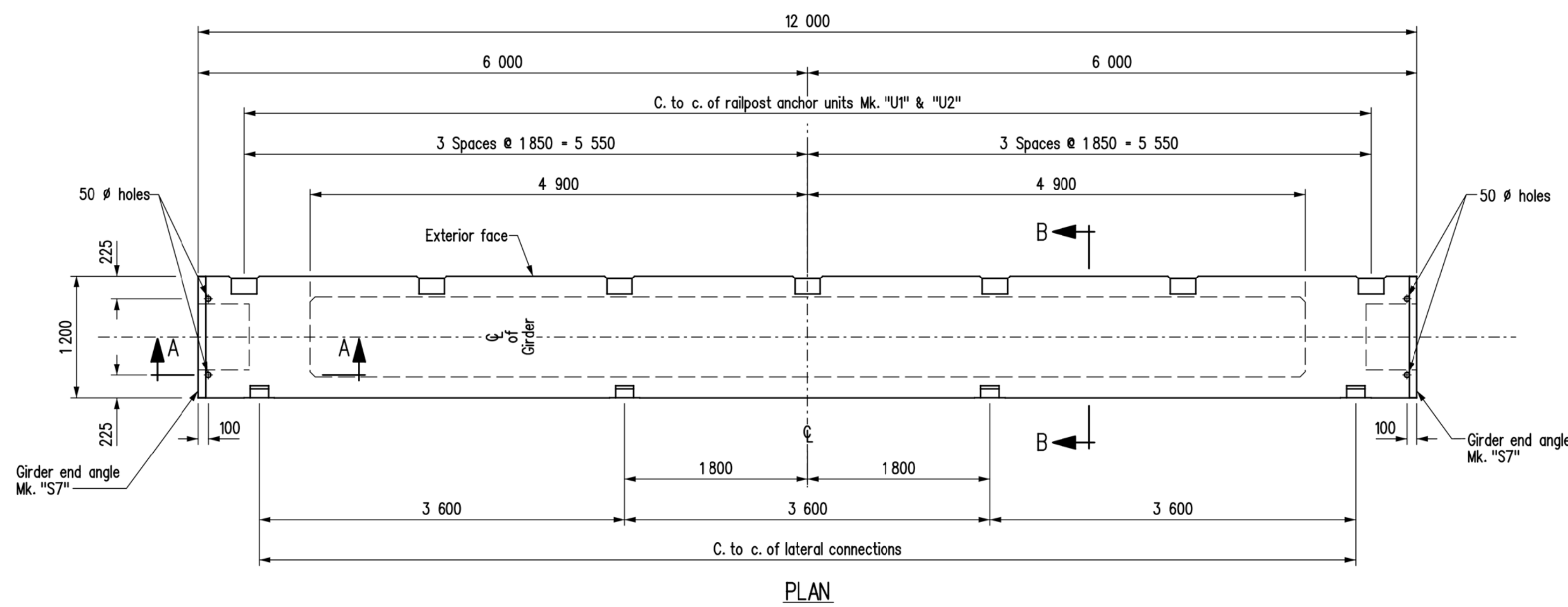


Infrastructure
Water Management and Structures

EXECUTIVE DIRECTOR OF STRUCTURES DATE

SCALE: 1:7.5 SHEET No. _____

or as shown SITE No. _____



- NOTES:
- Design in accordance with AASHTO LRFD Bridge Design Specifications, First Edition, 1994 plus 1996/1997 interims.
 - Design Vehicular Live Load: Modified AASHTO HSS-25 AASHTO LRFD "HL-93"
 - Design distribution factor = 0.5 lanes/girder.
 - Concrete strength: f_c transfer, f_{ci} - 35 MPa
 f_c 28 days, f_c - 45 MPa
 - Prestressing steel: 13 mm ϕ low relaxation strands
Minimum ultimate strength, f_{pu} = 1860 MPa
Jacking force/strand, f_{pj} = 128.5 kN/strand
 - Girder dimensioning tolerances: Length 3 mm \pm
Cross section 2 mm \pm
 - Approximate mass per girder = 12 000 kg

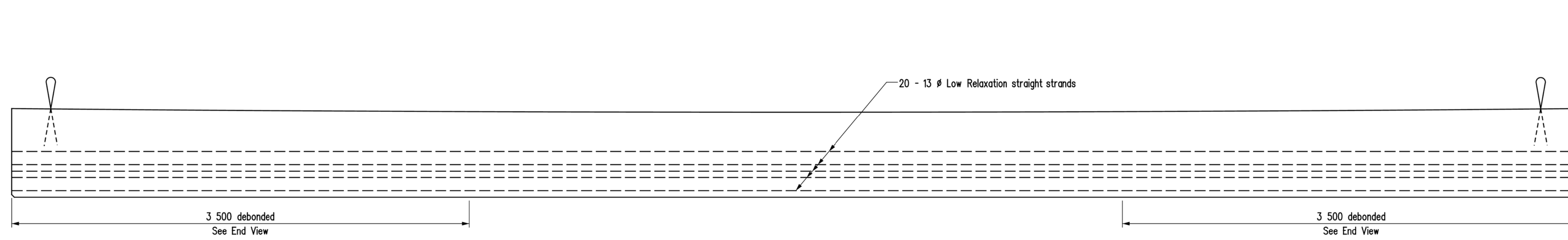
REVISIONS		PRECAST PRESTRESSED CHANNEL GIRDER DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:

DESIGN	BY: B.A.N.	EXECUTIVE DIRECTOR OF STRUCTURES	DATE
	CHECKED: _____		
DETAILS	BY: K.P.	SCALE: 1:40	SHEET No. 11
	CHECKED: _____		

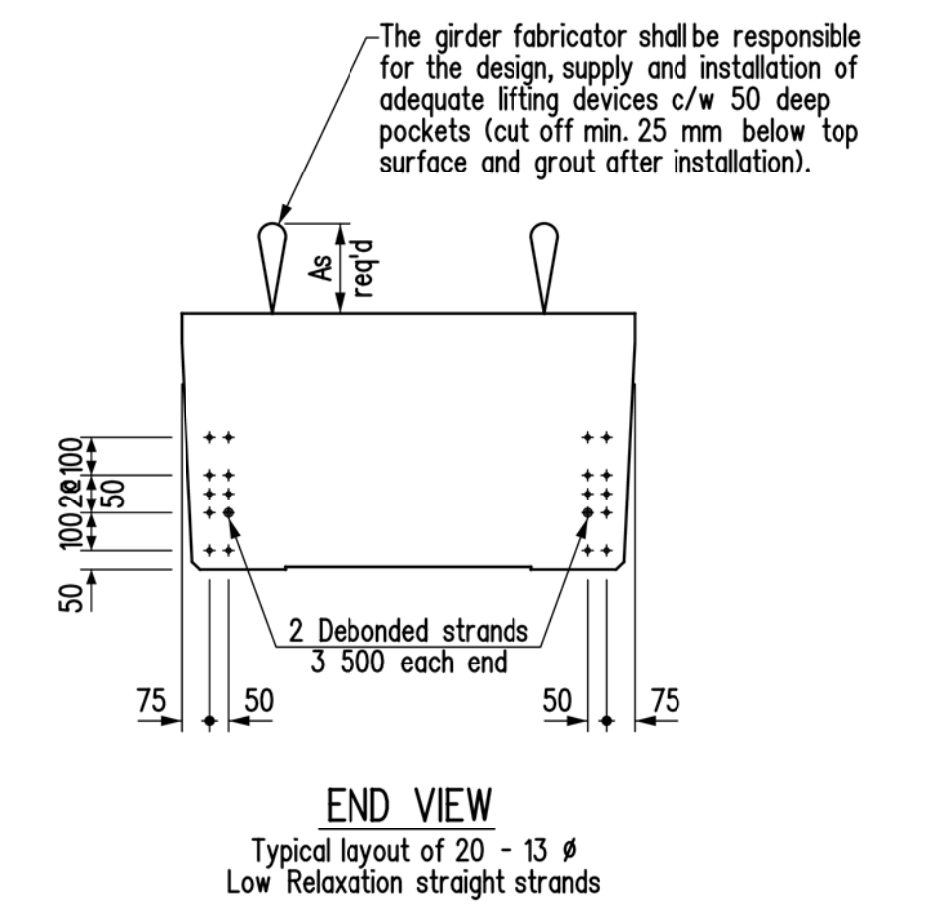
PLACE ENGINEERS ELECTRONIC SEAL HERE



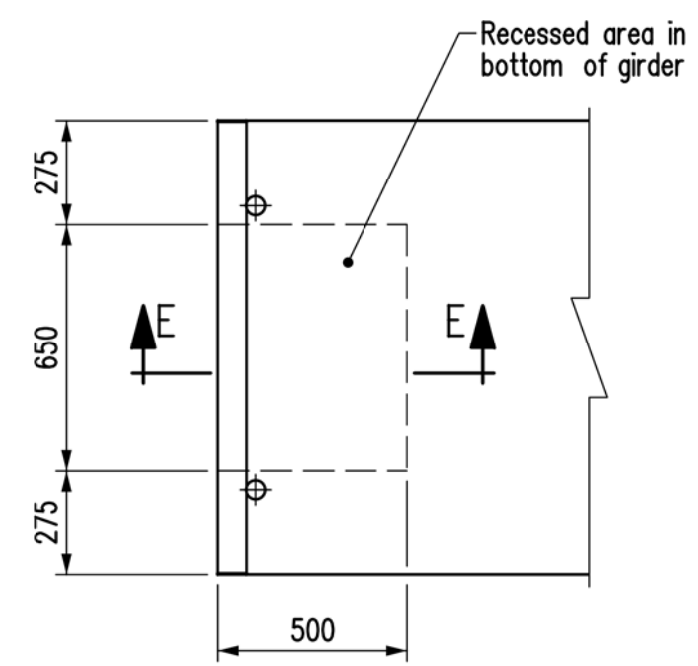
NOTE: The end of girder will fall between POINT "A" and POINT "C" on curve because of various skew angles.



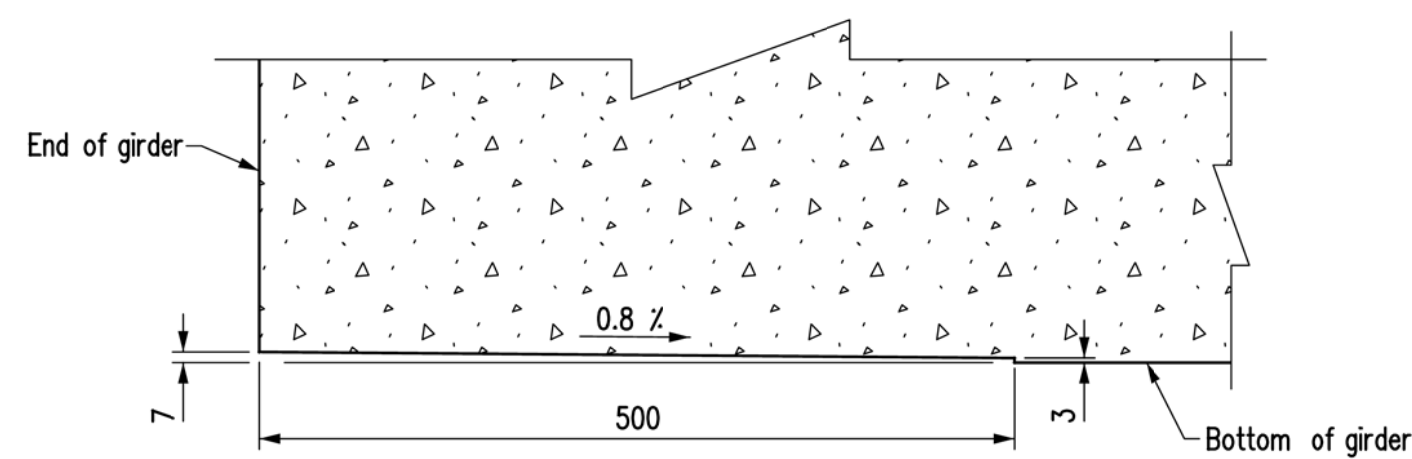
ELEVATION
GIRDER STRAND LAYOUT



END VIEW
Typical layout of 20 - 13 #
Low Relaxation straight strands



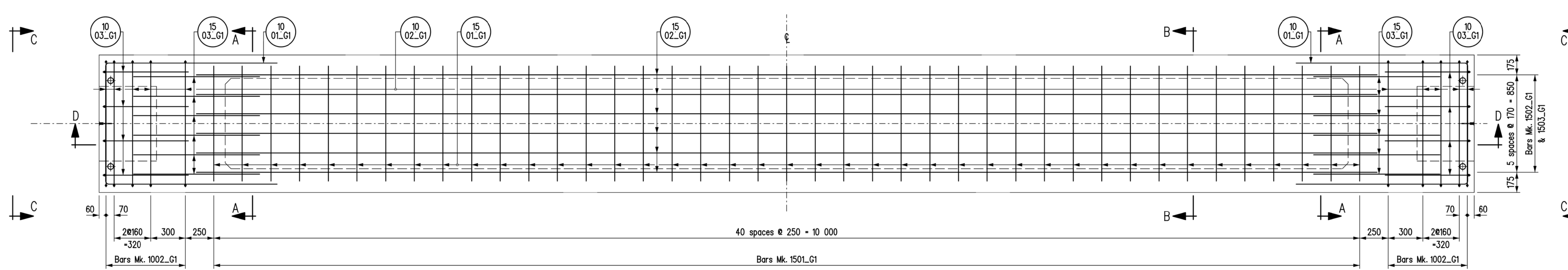
PART PLAN
Typical at both ends of girders



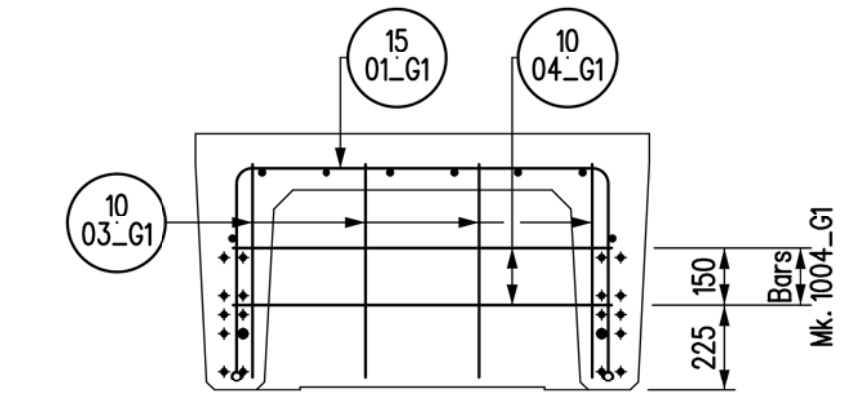
SECTION E-E
Scale 1:5

BEARING RECESS DETAILS

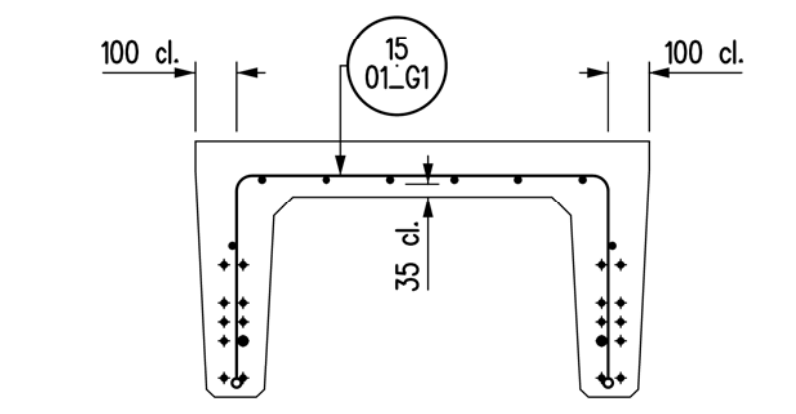
REVISIONS		PRECAST PRESTRESSED CHANNEL GIRDER DETAILS		
DATE	DESCRIPTION			
DESIGN SEAL	RECORD SEAL		RELEASED FOR CONSTRUCTION BY:	
PLACE ENGINEERS ELECTRONIC SEAL HERE			BY: <u>B.A.N.</u> CHECKED: _____ BY: <u>K.P.</u> CHECKED: _____	EXECUTIVE DIRECTOR OF STRUCTURES DATE
			SCALE: <u>1:20</u>	SHEET No. <u>12</u>
			or as shown	SITE No. _____



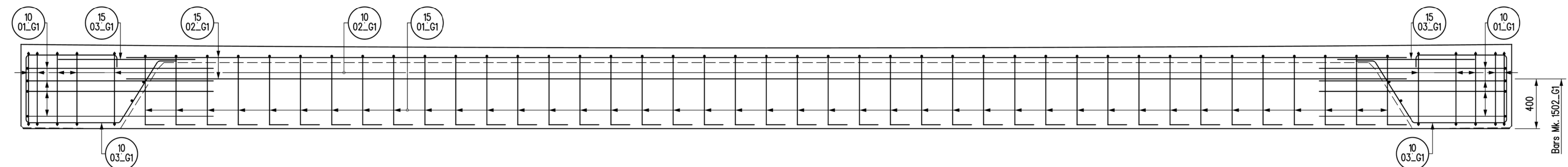
PLAN OF GIRDER



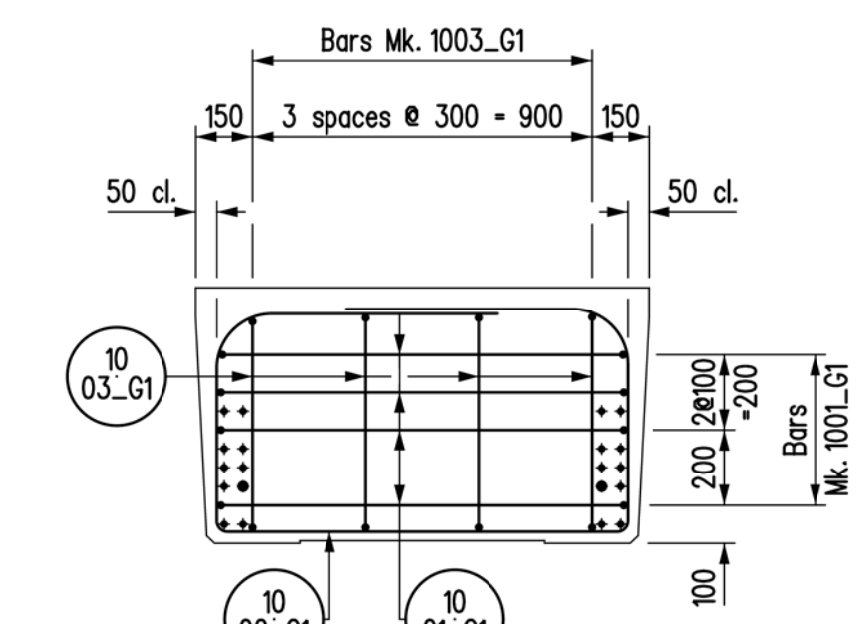
SECTION A-A



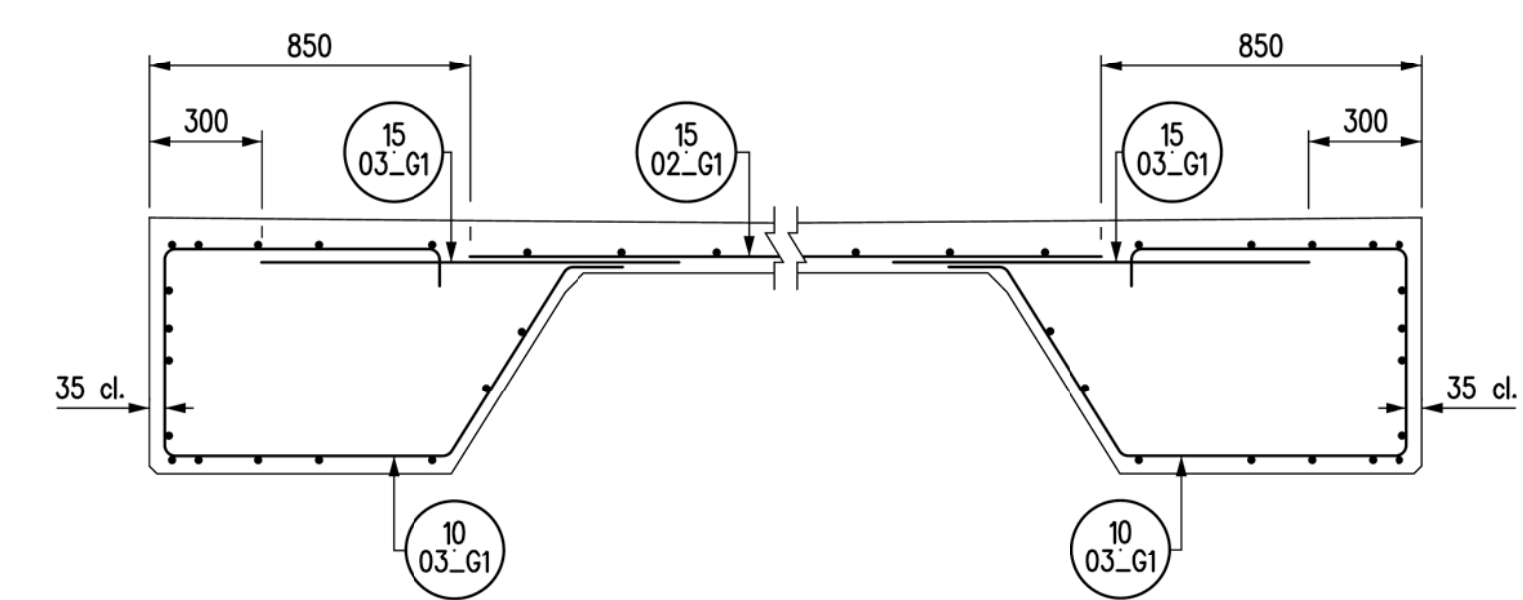
SECTION B-B



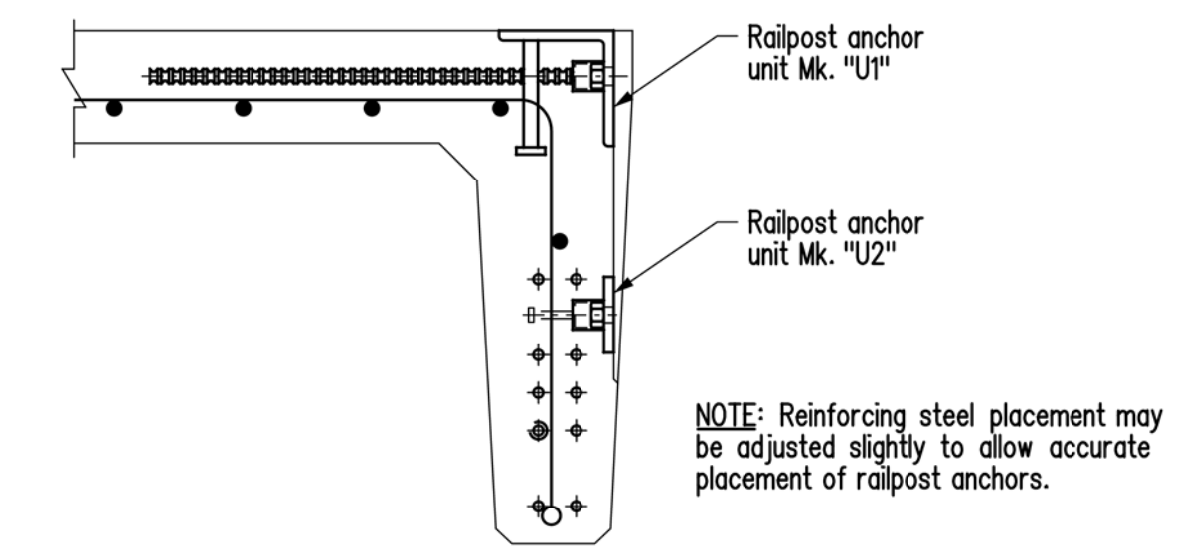
ELEVATION OF GIRDER



END VIEW C-C



PART SECTION D-D



DETAIL AT RAILPOST ANCHOR

Scale 1:10

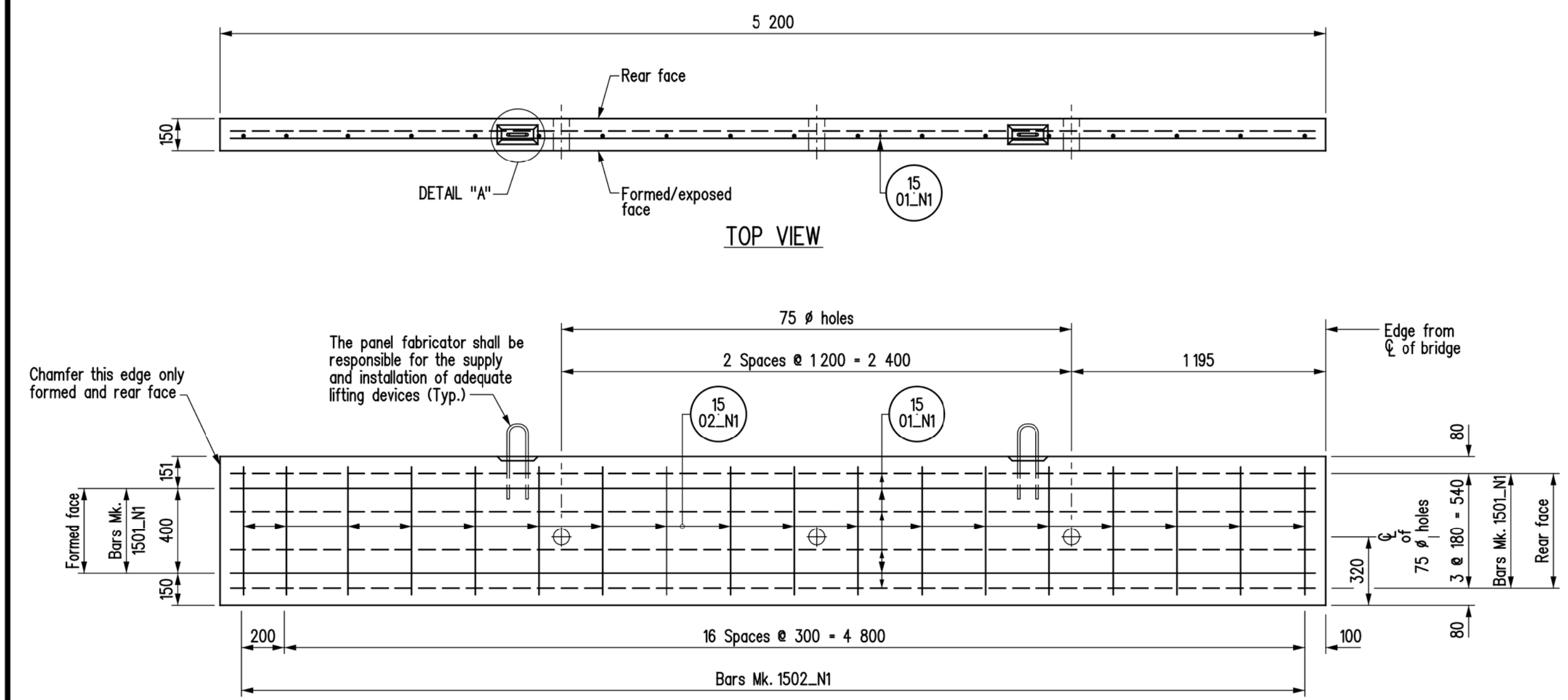
- NOTES:**
- 1. Concrete cover shall be 25 mm unless noted otherwise.
 - 2. Reinforcing details are typical for all 12 m girders unless noted otherwise.
 - 3. Bar Mark labels with suffix _G1 are Exterior girders and suffix _G2 are Interior girders. See Bill of Reinforcing Sheet No. G

<table border="1"> <thead> <tr> <th colspan="2">REVISIONS</th> </tr> </thead> <tbody> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> </tbody> </table>		REVISIONS																		<p>PRECAST PRESTRESSED CHANNEL GIRDER DETAILS</p>	
REVISIONS																					
<table border="1"> <thead> <tr> <th>DATE</th> <th>BY</th> <th colspan="2">DESCRIPTION</th> </tr> <tr> <td> </td> <td> </td> <td>DESIGN SEAL</td> <td>RECORD SEAL</td> </tr> </thead> <tbody> <tr> <td colspan="4" style="text-align: center;">PLACE ENGINEERS ELECTRONIC SEAL HERE</td> </tr> </tbody> </table>	DATE	BY	DESCRIPTION				DESIGN SEAL	RECORD SEAL	PLACE ENGINEERS ELECTRONIC SEAL HERE				<p>Manitoba Infrastructure Water Management and Structures</p>	<p>RELEASED FOR CONSTRUCTION BY:</p>							
DATE	BY	DESCRIPTION																			
		DESIGN SEAL	RECORD SEAL																		
PLACE ENGINEERS ELECTRONIC SEAL HERE																					
<p>DESIGN BY: <u>B.A.N.</u></p> <p>CHECKED: _____</p>	<p>DETAILS BY: <u>K.P.</u></p> <p>CHECKED: _____</p>	<p>EXECUTIVE DIRECTOR OF STRUCTURES _____ DATE _____</p> <p>SCALE: Scale 1: 20</p> <p>or as shown</p>	<p>SHEET No. <u>G4</u></p> <p>SITE No. _____</p>																		

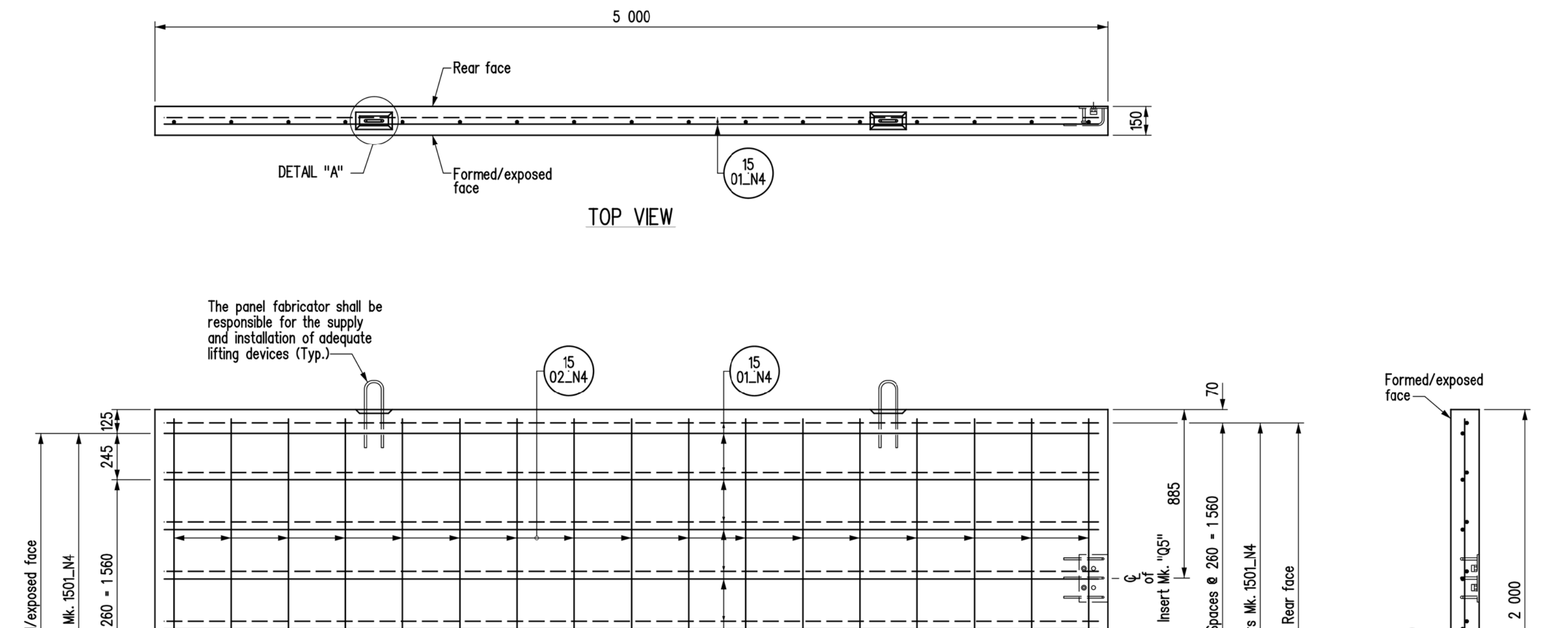
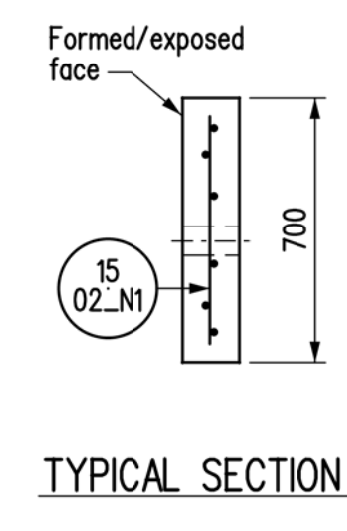
BILL OF REINFORCING STEEL - 12 M GIRDERS								SITE No.
MARK	TYPE	PIN DIAMETER	LENGTH	GIRDER TYPE	No. of GIRDERS	No. of BARS PER GIRDER	TOTAL No. of BARS PER GIRDER TYPE	BENDING DIAGRAM
1001_G1	BENT	45	4 080	G1	2	8	16	
1002_G1	BENT	45	3 660	G1	2	10	20	
1003_G1	BENT	45	2 950	G1	2	8	16	
1004_G1	STR		1 000	G1	2	4	8	
1501_G1	BENT	65	2 440	G1	2	41	82	
1502_G1	STR		10 300	G1	2	8	16	
1503_G1	STR		1 100	G1	2	12	24	
1001_G2	BENT	45	4 080	G2	5	8	40	
1002_G2	BENT	45	3 660	G2	5	10	50	
1003_G2	BENT	45	2 950	G2	5	8	40	
1004_G2	STR		1 000	G2	5	4	20	

BILL OF REINFORCING STEEL - 12 M GIRDERS								SITE No.
MARK	TYPE	PIN DIAMETER	LENGTH	GIRDER TYPE	No. of GIRDERS	No. of BARS PER GIRDER	TOTAL No. of BARS PER GIRDER TYPE	BENDING DIAGRAM
1501_G2	BENT	65	2 440	G2	5	41	205	
1502_G2	STR		10 300	G2	5	8	50	
1503_G2	STR		1 100	G2	5	12	60	
Total volume of structural concrete per exterior girder							4.94 m³	
Total volume of structural concrete per interior girder							4.93 m³	
NOTES:								
1. All dimensions given in bending diagram are out to out, except radii and extensions on 90°, 135° & 180° hooks. Extensions on 90°, 135° & 180° hooks are the "A" or "G" dimensions for standard 90°, 135° & 180° hooks referenced from the RSIC "Manual of Standard Practice". Radii are inside dimensions. All reinforcing steel bends and hooks shall conform to Clause 6.6.2 of C.S.A. A23.1-04, unless noted otherwise in the BILL OF REINFORCING STEEL.								
2. All reinforcing steel shall be deformed steel, unless noted otherwise in the BILL OF REINFORCING STEEL.								
3. All reinforcing steel shall conform to CSA G30.18-M92 "Billet Steel Bars for Concrete Reinforcement" Grade 400W, unless noted otherwise in the BILL OF REINFORCING STEEL.								
4. Like bars shall be bundled, securely tied and identified as to Mark and Site No. by appropriate means. All other items to be identified in a similar fashion.								
5. All bars shall be bent in accordance with the following detail:								

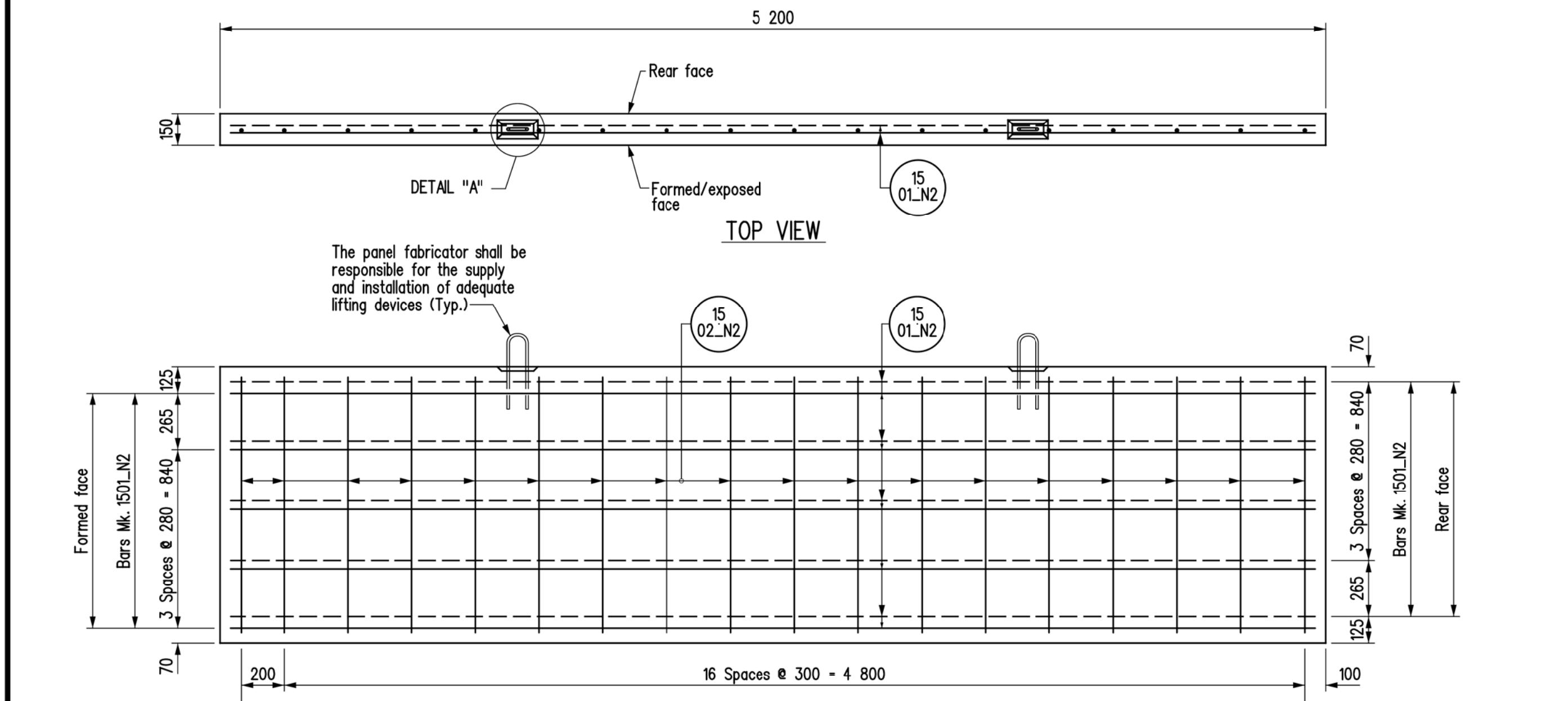
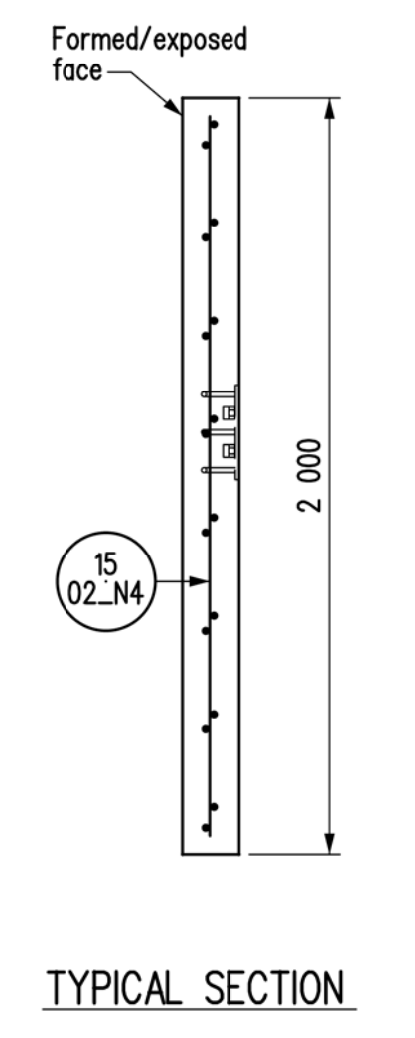
REVISIONS		PRECAST PRESTRESSED CHANNEL GIRDER DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:
DESIGN SEAL	RECORD SEAL		
PLACE ENGINEERS ELECTRONIC SEAL HERE			
		DESIGN	BY: <u>B.A.N.</u>
		CHECKED:	
		DETAILS	BY: <u>K.P.</u>
CHECKED:		EXECUTIVE DIRECTOR OF STRUCTURES	DATE
		SCALE:	SHEET No. <u>15</u>
			SITE No. <u> </u>



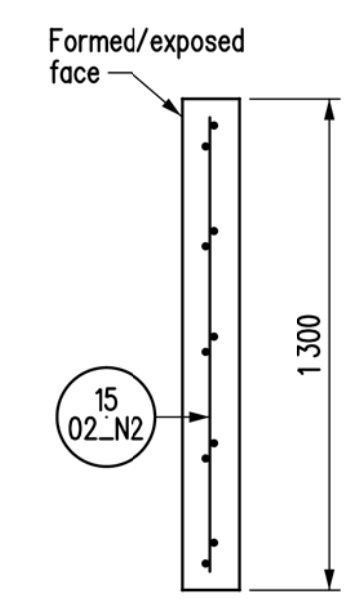
**ELEVATION
PRECAST PANEL Mk. "N1" & "N1a"**
Panel Mk."N1" shown, Panel Mk."N1a" opposite hand.



**ELEVATION
PRECAST PANEL Mk. "N4" & "N4a"**
Panel Mk."N4" shown, "N4a" similar except location of insert Mk."Q5" at opposite end.



**ELEVATION
PRECAST PANEL Mk. "N2"**



- NOTES:**
1. All panel exposed edges to be chamfered 20 mm except no chamfer on panels Mk. "N1" & "N1a", or if shown.
 2. Mark reinforcing steel location on the edges of the back face of panels after casting.
 3. Concrete cover shall be 50 mm unless noted otherwise.
 4. Formed face to be placed as exposed face during construction.
 5. After precast panel installation, all lifting devices to be cut-off flush and grouted as directed by Engineer.
 6. For DETAIL "A" see sheet No. P2.
 7. For BILL OF REINFORCING STEEL see Sheet No. P2.

REVISIONS		PRECAST PANEL DETAILS		
20_/_/_____	ISSUED FOR CONSTRUCTION	Manitoba Infrastructure Water Management and Structures	RELEASED FOR CONSTRUCTION BY: _____	
DATE	BY		DESIGN	EXECUTIVE DIRECTOR OF STRUCTURES DATE
DESIGN SEAL	RECORD SEAL		BY: <u> B.A.N. </u>	SCALE: <u> 1 : 20 </u> SHEET No. <u> P1 </u>
PLACE ENGINEERS ELECTRONIC SEAL HERE		DETAILS	BY: _____	
		DESIGN	CHECKED: _____	
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		DESIGN	CHECKED: _____	
		DETAILS	BY: _____	
		DESIGN	CHECKED: _____	
		DETAILS	BY: _____	
		DESIGN	CHECKED: _____	
		DETAILS	BY: _____	
		DESIGN	CHECKED: _____	
		DETAILS	BY: _____	

Site No. _____

BILL OF MISCELLANEOUS METAL for PRECAST PANELS

MARK No.	No.	DESCRIPTION	CORROSION PROTECTION	SIZE	LENGTH	REMARKS
Q5	4	insert units	Hot dip galvanized			
		Each unit is fabricated from:				
		Steel plate		PL 10 x 150	250	As detailed
		2 - Studs Mk. "A1"		13 dia.	75	Nelson headed concrete anchors, Type H4L, Part No. 101-053-002 - As detailed
		3 - Bars Mk. "A2"		10 dia.	300	Nelson deformed bar anchors, Type D2L, Part No. 101-064-537 - As detailed
		2 - Heavy hex. nuts		for 19 dia. bolt		Grade DH or 2H heavy hex. nut, c/w metal or plastic sleeve
R34	8	A325 bolt c/w F436 hardened washer		19 dia.	60	

NOTES:

- All material noted in the above Bill shall be hot dip galvanized after fabrication in accordance with CSA G164 for a minimum net retention of 610 g/m² unless otherwise stated in the specified material ASTM standards. The fabricator and galvanizer shall safeguard against embrittlement using recommended practices from applicable standards.
- Seal all welds prior to galvanizing.
- All structural steel to be CSA G40.21 Grade 300W.
- All bolts and inserts in the above Bill shall be Imperial thread.

TOP VIEW

FRONT VIEW
INSERT Mk. "Q5"
Scale 1:5

NOTES:

- For location of DETAIL "A" see sheet No. P1.
- Precast panel concrete strength: f'c = 35 MPa.

REVISIONS		PRECAST PANEL DETAILS		
DATE	DESCRIPTION			
ISSUED FOR CONSTRUCTION				
DATE	BY			
DESIGN SEAL		RECORD SEAL		EXECUTIVE DIRECTOR OF STRUCTURES DATE BY: _____
PLACE ENGINEERS ELECTRONIC SEAL HERE		DESIGN BY: <u>B.A.N./</u> CHECKED: _____		
DETAILS		SCALE: <u>1:2</u> BY: _____ CHECKED: _____		SHEET No. <u>P2</u> OF AS SHOWN <u> </u> SITE No. <u> </u>

Site No. _____

BILL OF REINFORCING FOR PRECAST PANELS

MARK	TYPE	PIN DIAMETER	LENGTH	PANEL TYPE	No. of PANELS	No. of BARS PER PANEL	TOTAL No. of BARS PER PANEL TYPE	BENDING DIAGRAM
1501_N1	STR		5 100	N1	2	6	12	BENDING DIAGRAM
1502_N1	STR		600	N1	2	18	36	
1501_N1a	STR		5 100	N1a	2	6	12	
1502_N1a	STR		600	N1a	2	18	36	
1501_N2	STR		5 100	N2	4	10	40	
1502_N2	STR		1 200	N2	4	18	72	
1501_N4	STR		4 900	N4	2	16	32	
1502_N4	STR		1 900	N4	2	17	34	
1501_N4a	STR		4 900	N4a	2	16	32	
1502_N4a	STR		1 900	N4a	2	17	34	

Total mass of reinforcing steel							1411.12	kg
Panel Type	N1	N1a	N2	N3	N4	N4a		
Area m ² /panel	3.60	3.60	6.80	--	10.00			
Total area of precast Panels							81.60	m ²

NOTES:

- All dimensions given in bending diagram are out to out, except radii and extensions on 90°, 135° & 180° hooks. Extensions on 90°, 135° & 180° hooks are the "A" of "C" dimensions for standard 90°, 135° & 180° hooks referenced from the RSIC "Manual of Standard Practice". Radii are inside dimensions. All reinforcing steel bends and hooks shall conform to Clause 6.6.2 of C.S.A. A23.1-04, unless noted otherwise in the BILL OF REINFORCING STEEL.
- All reinforcing steel shall be deformed steel, unless noted otherwise in the BILL OF REINFORCING STEEL.
- All reinforcing steel shall conform to CSA G30.18-M92 "Billet Steel Bars for Concrete Reinforcement" Grade 400W, unless noted otherwise in the BILL OF REINFORCING STEEL.
- Like bars shall be bundled, securely tied and identified as to Mark and Site No. by appropriate means. All other items to be identified in a similar fashion.
- All bars shall be bent in accordance with the following detail:

