

PLANS OF PROPOSED P.P.C.C. BRIDGE OVER ON

LENGTH 12 352 OUT TO OUT OF ABUTMENT PRECAST BACKWALL PANELS

SUPERSTRUCTURE ONE SIMPLY SUPPORTED SPAN OF PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS WITH ASPHALT OVERLAY

SUBSTRUCTURE TWO PRECAST CONCRETE ABUTMENTS WITH STEEL H-PILES

ROADWAY WIDTH 12 000 OUT TO OUT OF GIRDERS

LOCATION IN R.M. OF

SHEET LEGEND

1. COVER SHEET
2. GENERAL ELEVATION
3. BORING LOGS
4. SITE AND EROSION CONTROL DETAILS
5. ASSEMBLY DETAILS
6. ASSEMBLY DETAILS
7. STEEL PILE CAP DETAILS
8. BEARING AND ERECTION DETAILS
9. RAILING LAYOUT AND DETAILS
10. RAILING DETAILS
11. RAILPOST DETAILS

- P1. PRECAST PANEL DETAILS
- P2. PRECAST PANEL DETAILS

- G1. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G2. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G3. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G4. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G5. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS

DESIGN DATA

SPECIFICATIONS

AASHTO LRFD Bridge Design Specifications, First Edition, 1994 plus 1996/97 Interims

VEHICULAR LIVE LOADING

1. Modified AASHTO HSS-25 Truck
2. AASHTO LRFD "HL-93" Loading

STRUCTURAL CONCRETE

CSA A23.1, Exposure Class C-1 Air content category 1

1. PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS - $f_c = 45$ MPa at 28 days
 $f_{ci} = 35$ MPa at time of de-stressing
2. PRECAST PANELS - $f_c = 35$ MPa

REINFORCING STEEL

1. PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS - CAN/CSA-G30.18-M92 Grade 400W black (i.e no epoxy coating)
2. PRECAST PANELS - CAN/CSA-G30.18-M92 Grade 400W black (i.e no epoxy coating)

STRUCTURAL STEEL

1. All Structural Steel shall conform to CAN/CSA G40.21-M92 Grade 300W
2. HSS Tubing for Bridge Rail shall conform to CAN/CSA G40.21-M92 Grade 350W

PRESTRESSING STRAND

20-13 \emptyset low relaxation strands, $f_{pu} = 1860$ MPa

PILE LOADING

	END PILE BENTS	INTERMEDIATE PILE BENTS
MAXIMUM FACTORED LOAD	597 kN	668 kN
FACTORED BEARING RESISTANCE		

HYDRAULIC DESIGN DATA

DESIGN DISCHARGE

$Q = \text{-----} \text{ m}^3/\text{sec}$

SURVEY CONTROL

HORIZONTAL DATUM: NAD83CSRS

VERTICAL DATUM: CGVD28

ELLIPSOID: GRS 1980

GEOID (HT2.0): -----

UTM: ZONE ----

SCALE FACTOR: -----

SITE CONTROL POINT DATA

CONTROL POINT #-----	NORTHING: -----	EASTING: -----	ELEVATION: -----	DATE: -----
CONTROL POINT #-----	NORTHING: -----	EASTING: -----	ELEVATION: -----	DATE: -----
CONTROL POINT #-----	NORTHING: -----	EASTING: -----	ELEVATION: -----	DATE: -----

PLACE LOCATION
MAP HERE



TP. -

RGE. -

LOCATION MAP

Not to Scale

MANITOBA INFRASTRUCTURE

WATER MANAGEMENT AND STRUCTURES

RELEASED FOR CONSTRUCTION BY :

EXECUTIVE DIRECTOR OF STRUCTURES

DATE -----

ENVIRONMENTAL APPROVALS

- | | |
|--------------------------|---|
| <input type="checkbox"/> | MANITOBA ENVIRONMENT ACT LICENCE |
| | DATE : _____ |
| | FILE # : _____ |
| <input type="checkbox"/> | FISHERIES AND OCEANS CANADA - AUTHORIZATION OR REVIEW |
| | DATE : _____ |
| | FILE # : _____ |
| <input type="checkbox"/> | TRANSPORT CANADA - NAVIGATION ACT |
| | DATE : _____ |
| | FILE # : _____ |
| <input type="checkbox"/> | MANITOBA INFRASTRUCTURE ENVIRONMENTAL APPROVAL |
| | DATE : _____ |
| | FILE # : _____ |
| <input type="checkbox"/> | ENVIRONMENTAL REVIEW COMPLETED |
| | DATE : _____ |
| | COMPLETED BY : _____ |

ALL DIMENSIONS ARE IN MILLIMETRES (mm) AND ALL ELEVATIONS AND STATIONS ARE IN METRES (m) UNLESS SHOWN OTHERWISE.

DRAWN BY:

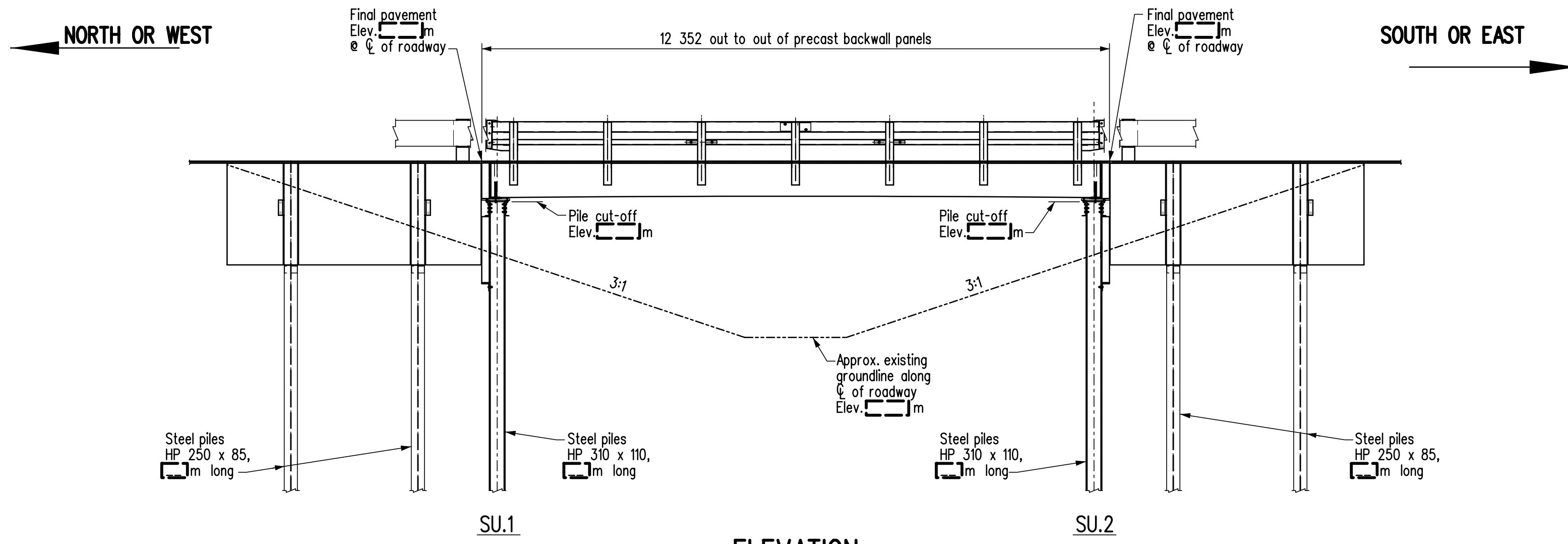
DATE:

SHEET No. 1

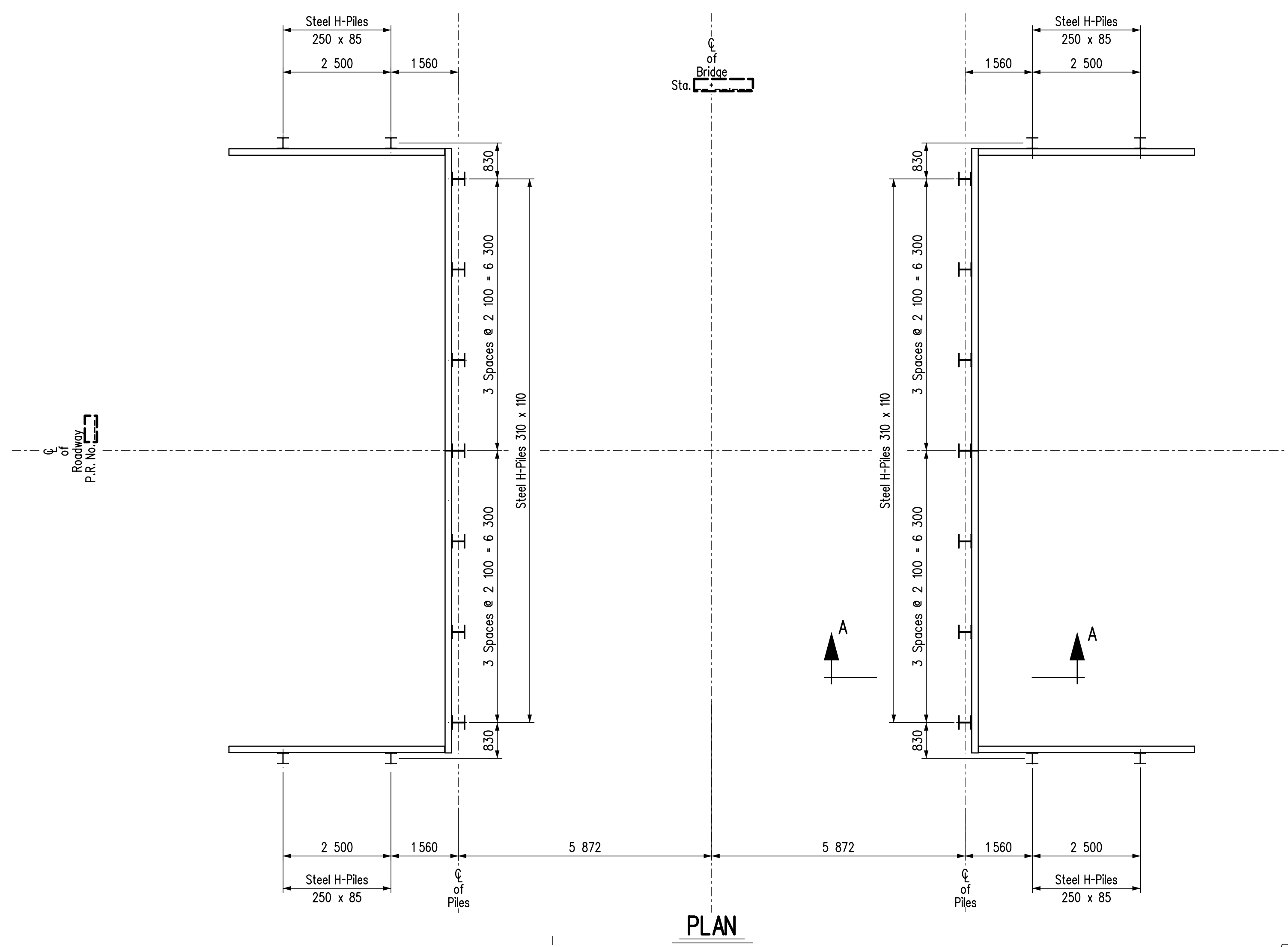
CHECKED BY:

DATE:

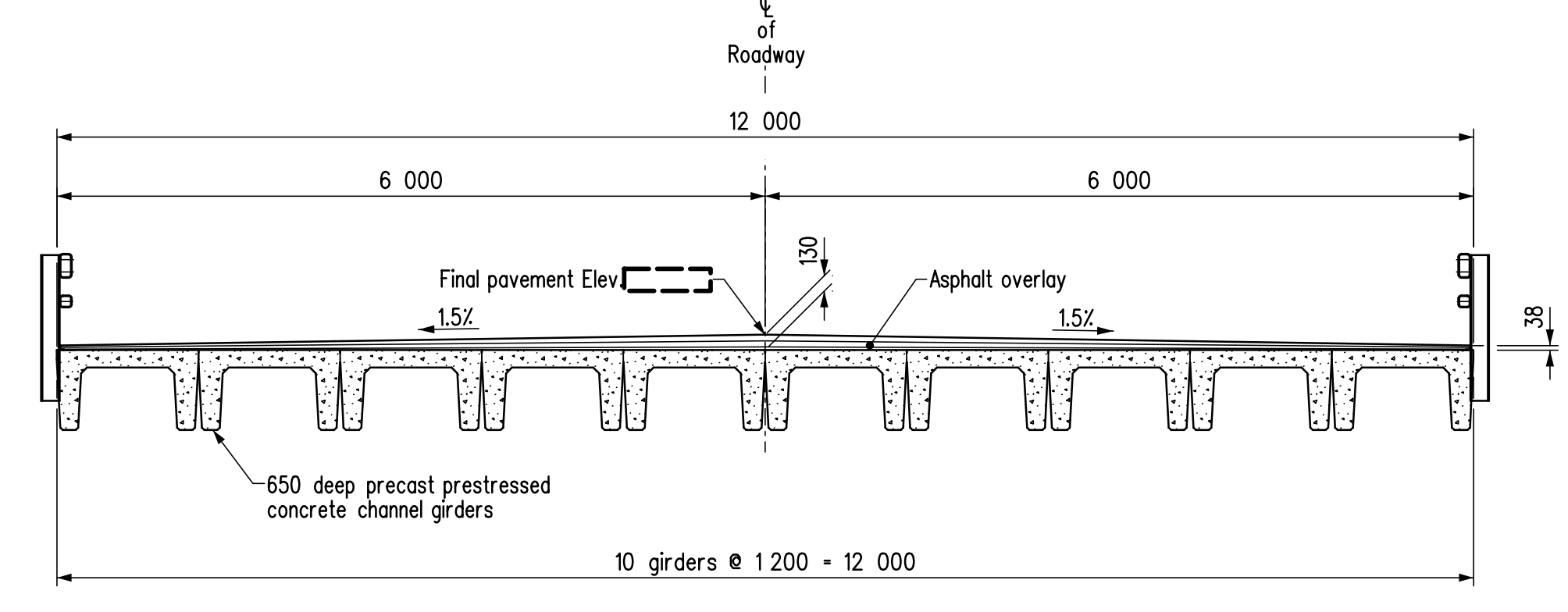
SITE No.



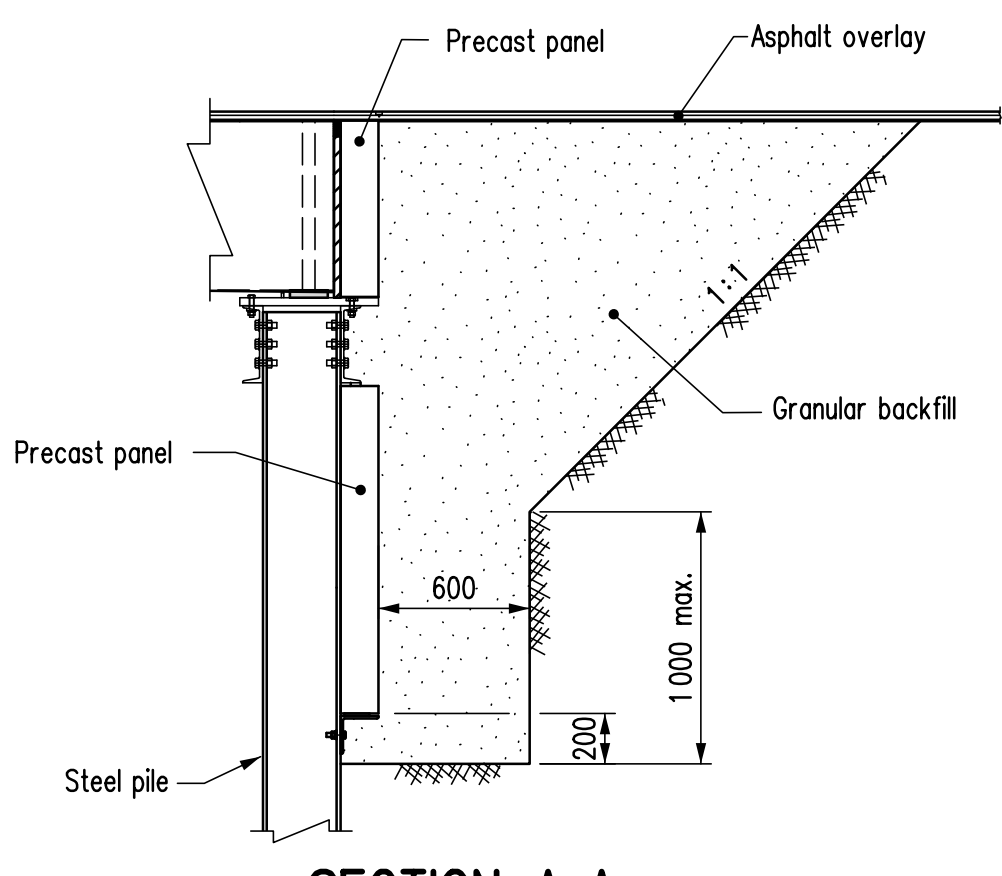
ELEVATION



PLAN



CROSS SECTION
Scale 1:50

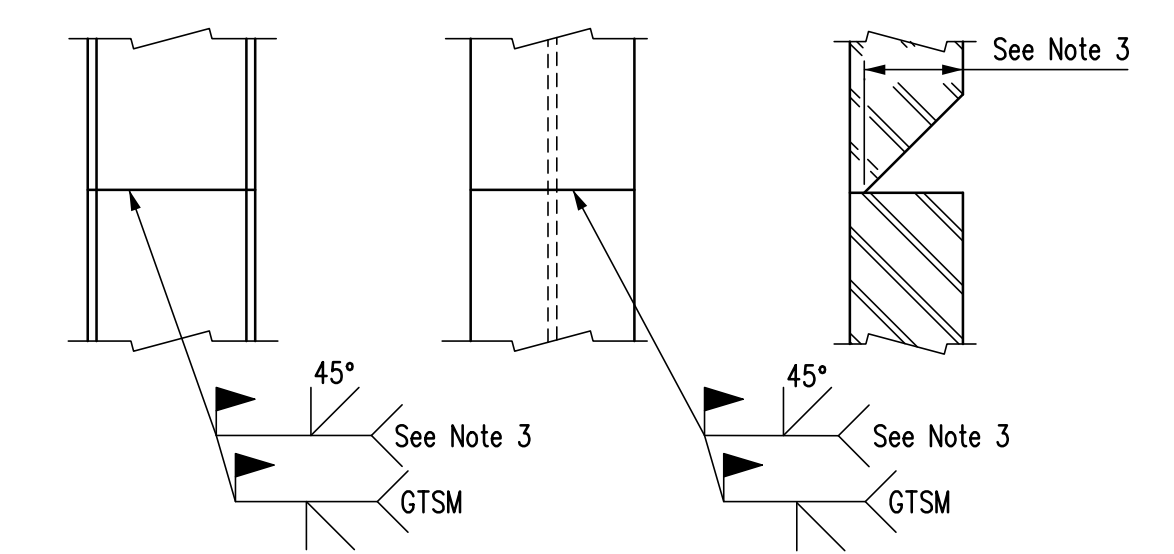


SECTION A-A
Typical at Su.1
Scale 1:30

- NOTES :**
re: Backfill Behind Abutment Ballast Walls
- Backfill behind ballast wall and wingwall panels shall be Type 1 - Granular backfill supplied and placed in accordance with Bridge Specification 1001 (I). The granular backfill shall be placed and compacted in lifts not exceeding 150 mm.
 - Compaction equipment used within 2 m of ballast walls and wingwalls shall be limited to light vibratory equipment with a mass not exceeding 120 kg unless otherwise approved.
 - Steel pile tip to be PRUYN "Hard-Bite" or equivalent.

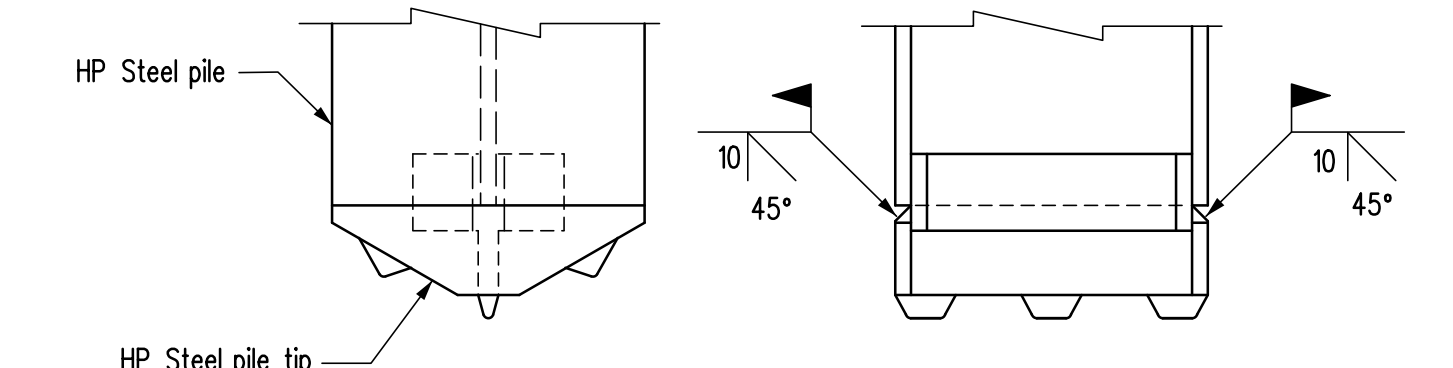
BILL OF PILES		Site No.		
LOCATION	DESCRIPTION	No. OF PILES	LENGTH	TOTAL LENGTH (m)
SU.1 & SU.2	Steel piles - HP310 x 110 (abutments)	14		0
SU.1 & SU.2	Steel piles - HP250 x 85 (wingwalls)	8		0
TOTAL LENGTH OF PILES (m) = 0				

BILL OF PILE TIPS		No. OF PILES
LOCATION	DESCRIPTION	No. OF PILES
SU.1 & SU.2	Hard-Bite Point HP-77750-B for HP310 x 110 (Abutments)	14



DETAIL OF STEEL HP PILE SPLICE

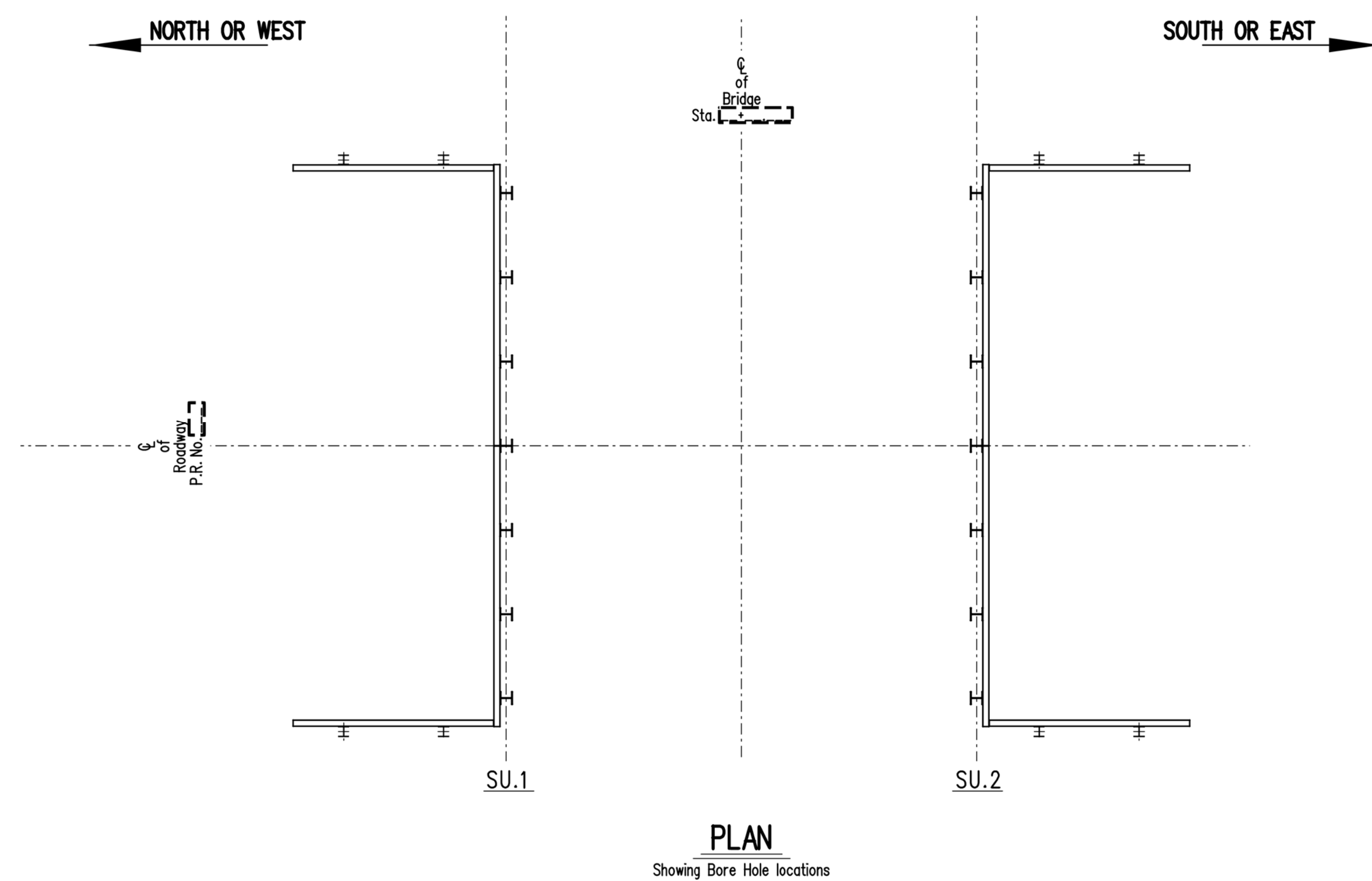
- NOTES:**
re: Welding
Not To Scale
- Low hydrogen E70 series electrodes shall be used.
 - The minimum root pass shall be 6 mm.
 - Preparation for welding requires 13 mm bevel for HP 250 piles and 14 mm bevel for HP 310 piles.
 - Weld both flanges and web as shown. The inside bevelling and welds to be completed first.
 - Before undertaking the back welds, the weld preparation shall be carried out with a carbon Arc-Air gouger.



DETAIL OF STEEL HP PILE TIP

- NOTES :**
Not to Scale
- Edges of HP Steel pile tip to be ground on 45° bevel for 10 mm.
 - Low hydrogen E70 series electrodes shall be used.
 - The minimum root pass shall be 6 mm.

REVISIONS		GENERAL ELEVATION	
DATE	BY	DESIGN SEAL	RECORD SEAL
<p>PLACE ENGINEERS ELECTRONIC SEAL HERE</p>		<p>Manitoba Infrastructure Water Management and Structures</p>	<p>RELEASD FOR CONSTRUCTION BY:</p> <p>EXECUTIVE DIRECTOR OF STRUCTURES DATE</p> <p>SCALE: 1:75 SHEET No. 2</p> <p>or as shown SITE No.</p>

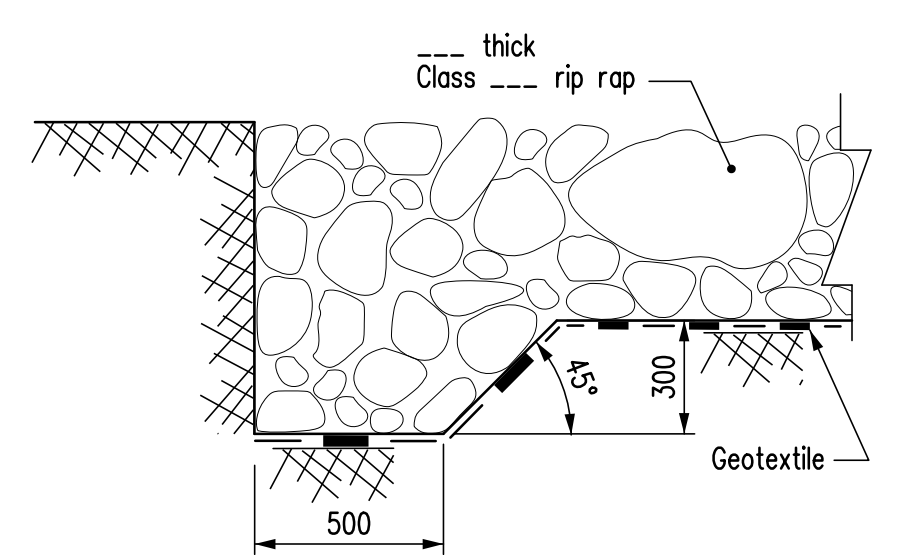
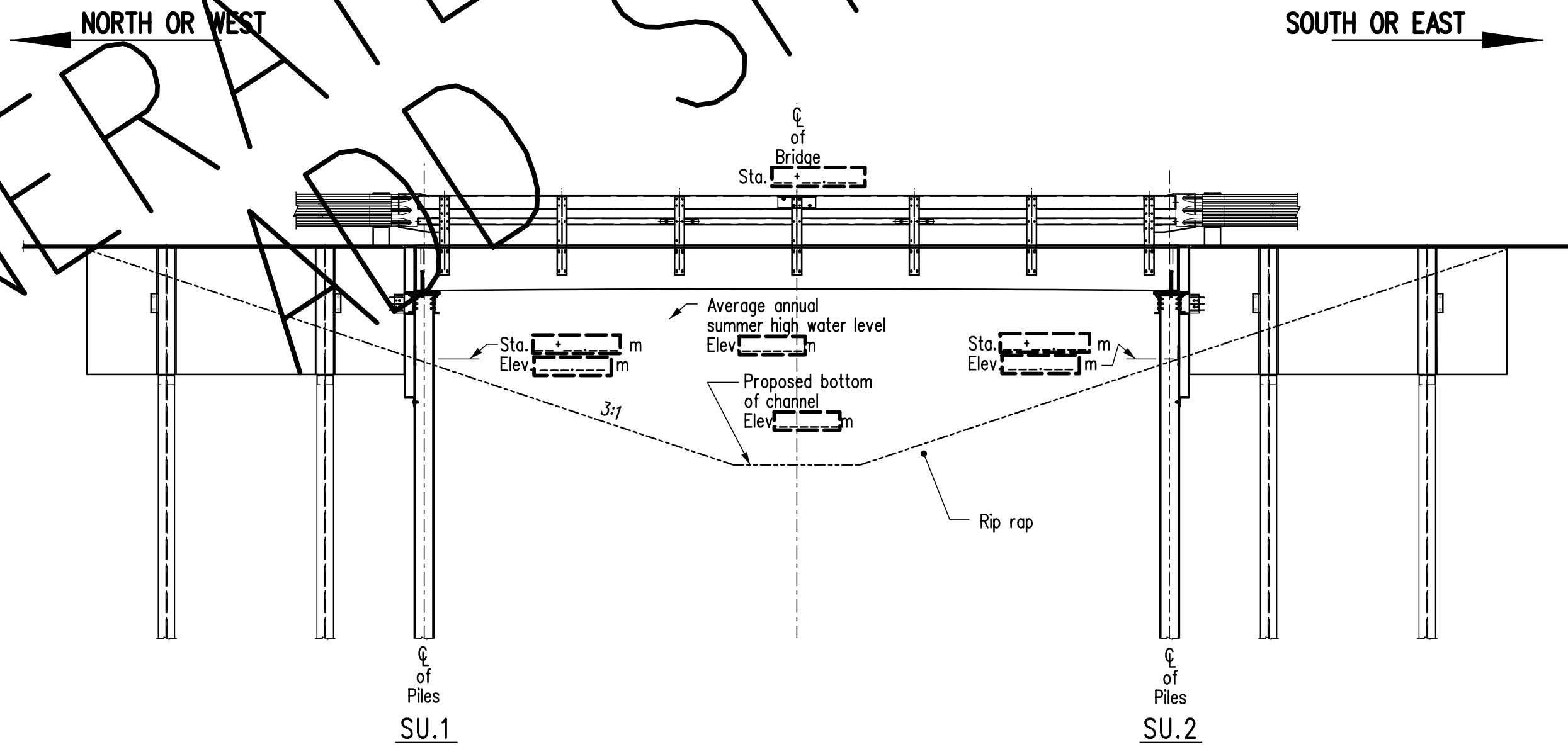
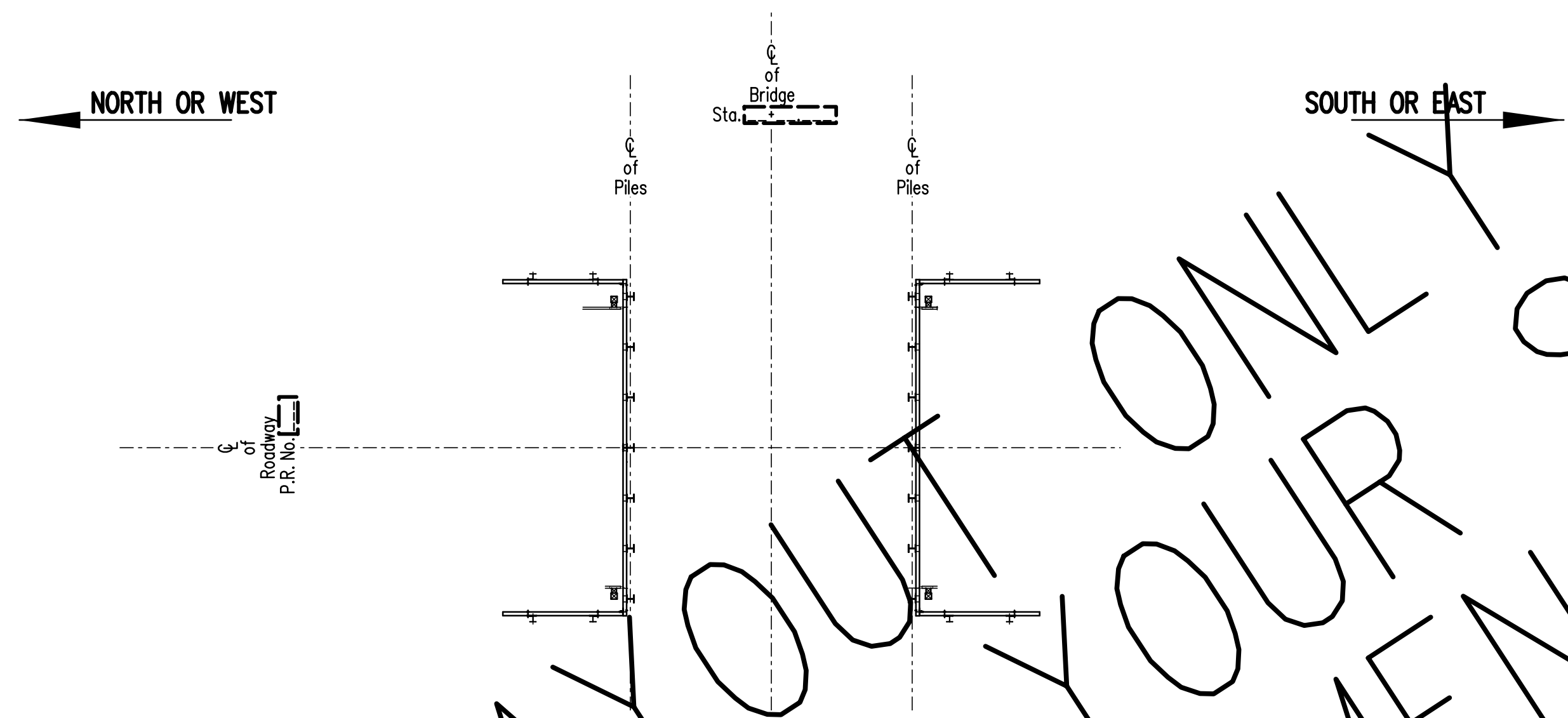


NOTES - re: Boring Logs

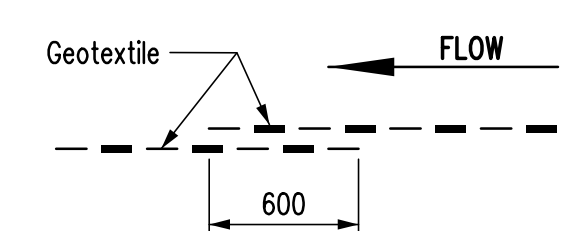
1. The Department provides log boring information shown on the Plans. This information may not be representative of the soil conditions throughout the site. Contractors may peruse all available soil information in the Water Management and Structures Branch located at 6th floor, 215 Garry Street, Winnipeg.
2. The following abbreviations apply to bore hole information:
 - Qu - Laboratory unconfined compressive strength in kPa
 - SPT (N) - Number of blows per 300 mm - Standard Penetration Test
 - USC - Unified Soil Classification
 - M.C. - Moisture Content
3. All stations, elevations, offsets and depths as shown are in meters. All dimensions are in millimeters.
4. All bore hole locations shown in plan view are approximate.
5. Elevations on boring logs are at a vertical scale of 1:100.

REVISIONS		BORING LOGS	
DATE	BY	DESCRIPTION	
		DESIGN SEAL	RECORD SEAL
PLACE ENGINEERS ELECTRONIC SEAL HERE		Manitoba <small>Infrastructure</small> <small>Water Management and Structures</small>	
BY: _____ B.A.N. _____ CHECKED: _____		RELEASED FOR CONSTRUCTION BY: _____ DATE: _____	
BY: _____ K.P. _____ CHECKED: _____		SCALE: 1:100	SHEET No. 3
CHECKED: _____		or as shown	SITE No. 111

GENERATE EXAMPLE SHEET FROM ONLY YOUR SITE PLAN
ADD SITE SPECIFIC DIMENSIONS



EDGE TREATMENT



OVERLAPPING DETAILS

RIP RAP DETAILS

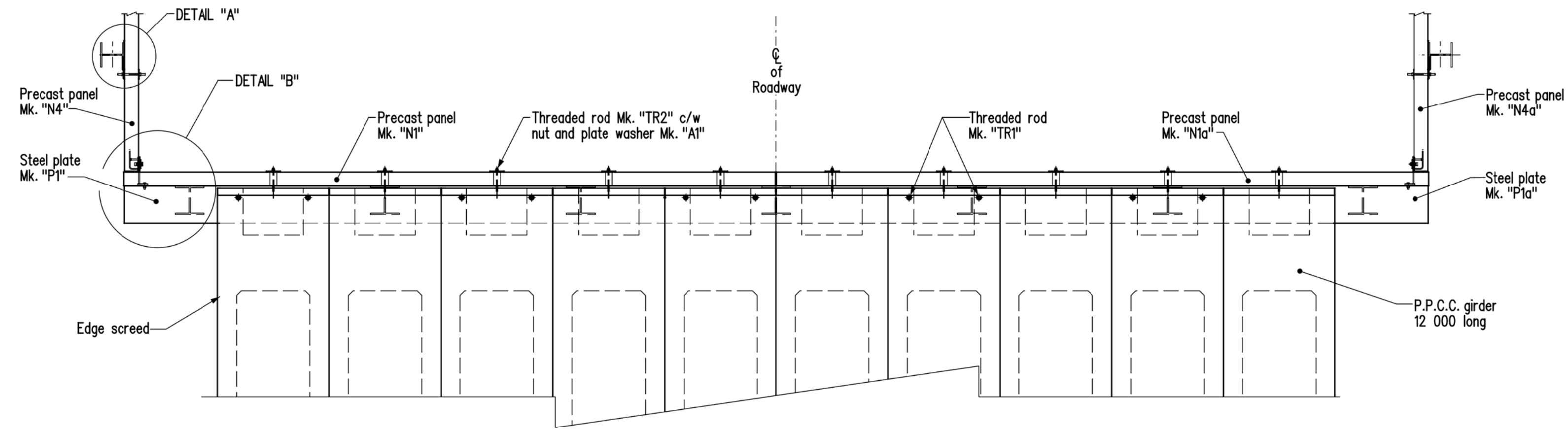
Not To Scale

- NOTES:
- All geotextile shall be Non-Woven Geotextile, Class 1 (Heavy Duty) from the Manitoba Infrastructure's Approved Product List.
 - Geotextile shall be placed under all rip rap, overlapping 600mm in direction of flow.

NOTE:
Existing pile bents to be removed by Bridge Contractor.

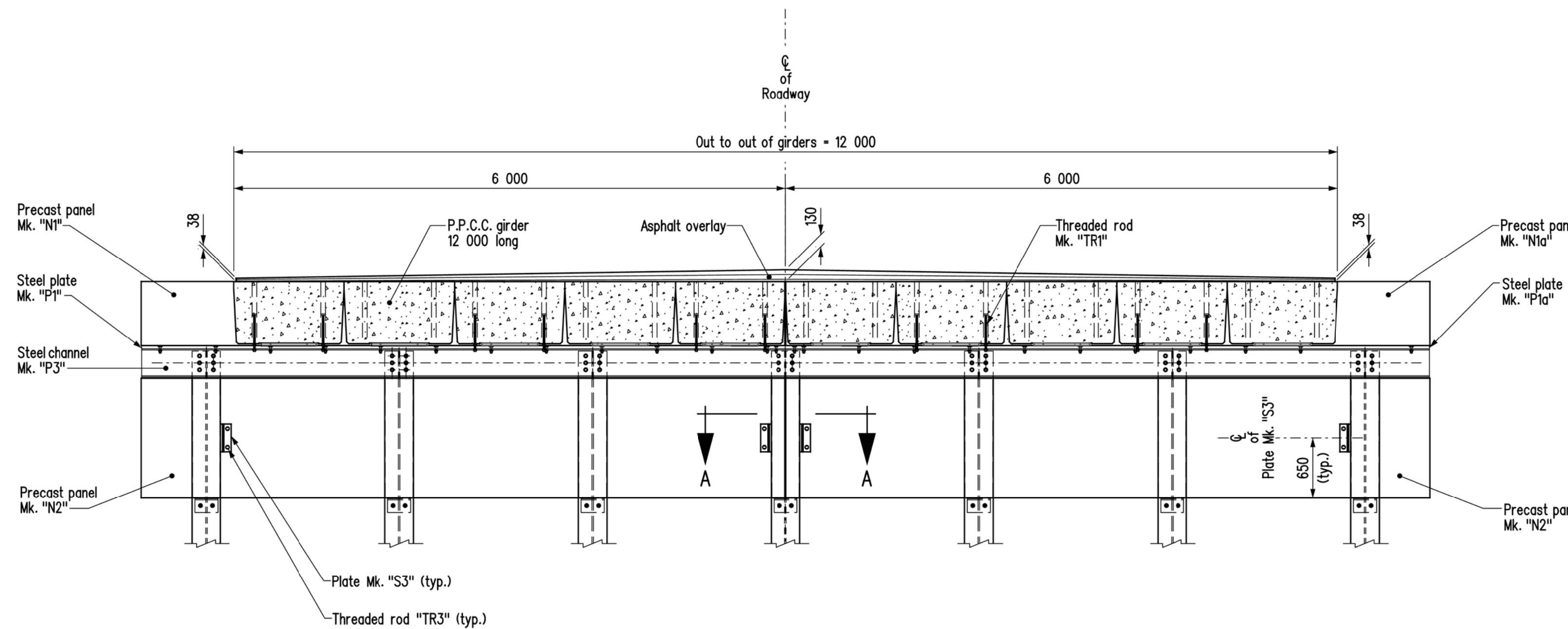
UTILITY DISCLAIMER:
LOCATIONS OF UTILITIES AS SHOWN ARE BASED ON READILY AVAILABLE INFORMATION. NO GUARANTEE IS GIVEN THAT ALL UTILITIES ARE SHOWN OR THAT THE GIVEN LOCATIONS ARE EXACT. CONTRACTOR SHALL CONFIRM THE EXISTENCE AND LOCATION OF UTILITIES BY OBTAINING FROM THE INDIVIDUAL UTILITIES BEFORE PROCEEDING WITH CONSTRUCTION.

REVISIONS		SITE AND EROSION CONTROL DETAILS	
DATE	BY	DESCRIPTION	
DESIGN SEAL	RECORD SEAL		
PLACE ENGINEERS ELECTRONIC SEAL HERE			
		Water Management and Structures	
DESIGN		BY: _____	RELEASED FOR CONSTRUCTION BY: _____
DETAILS		CHECKED: _____	EXECUTIVE DIRECTOR OF STRUCTURES DATE _____
		BY: _____ K.P.	SCALE: 1:200 SHEET No. 4
		CHECKED: _____	or as shown SITE No. _____



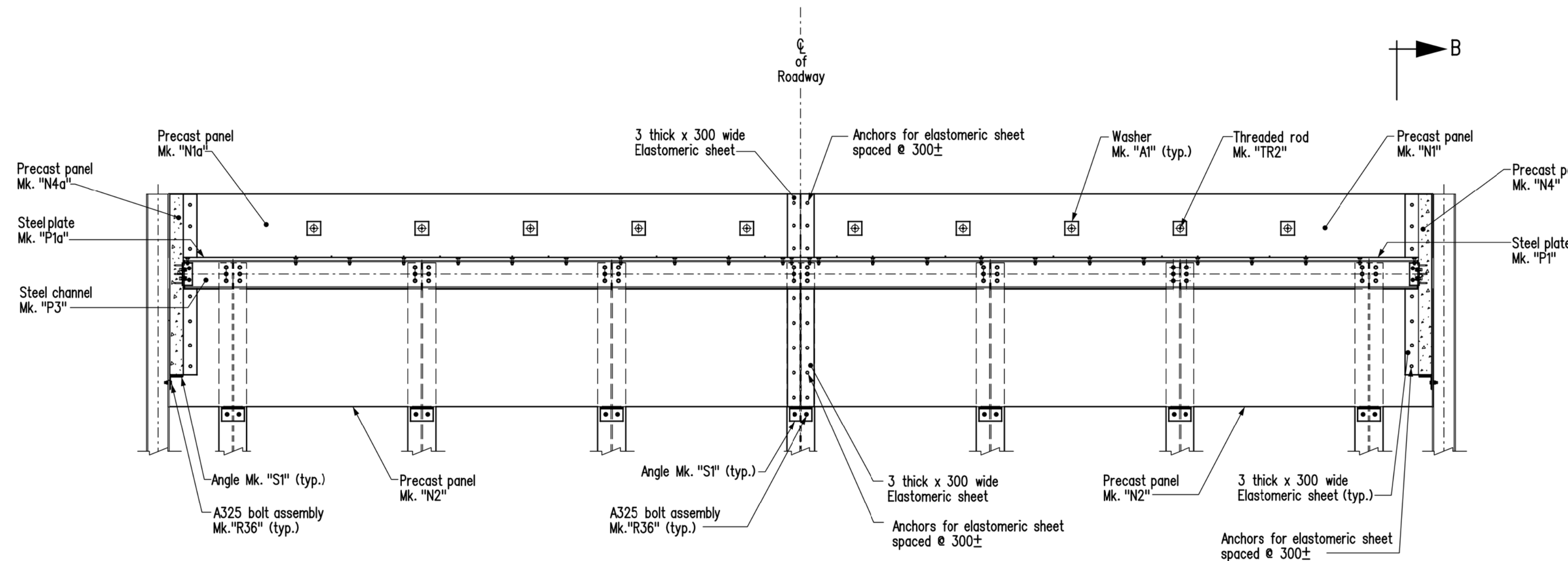
PART PLAN

Showing abutment SU.1
Asphalt and bridge railing not shown for clarity



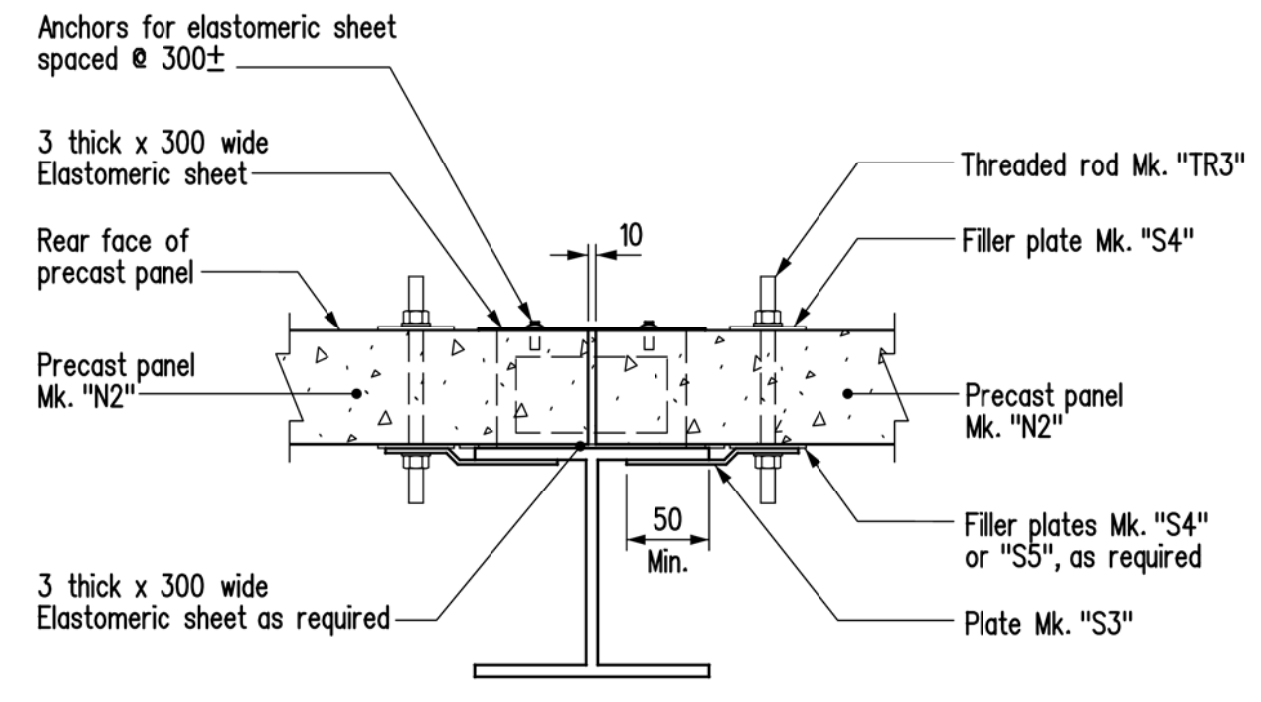
FRONT VIEW AT ABUTMENT

Bridge railing not shown for clarity



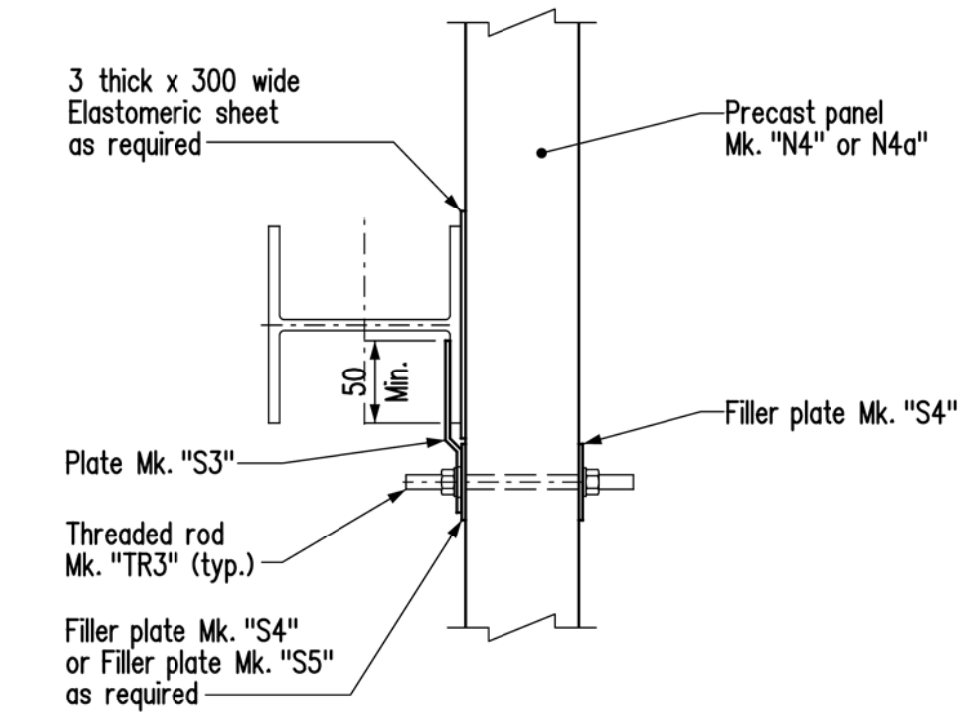
REAR VIEW AT ABUTMENT

Bridge railing not shown for clarity



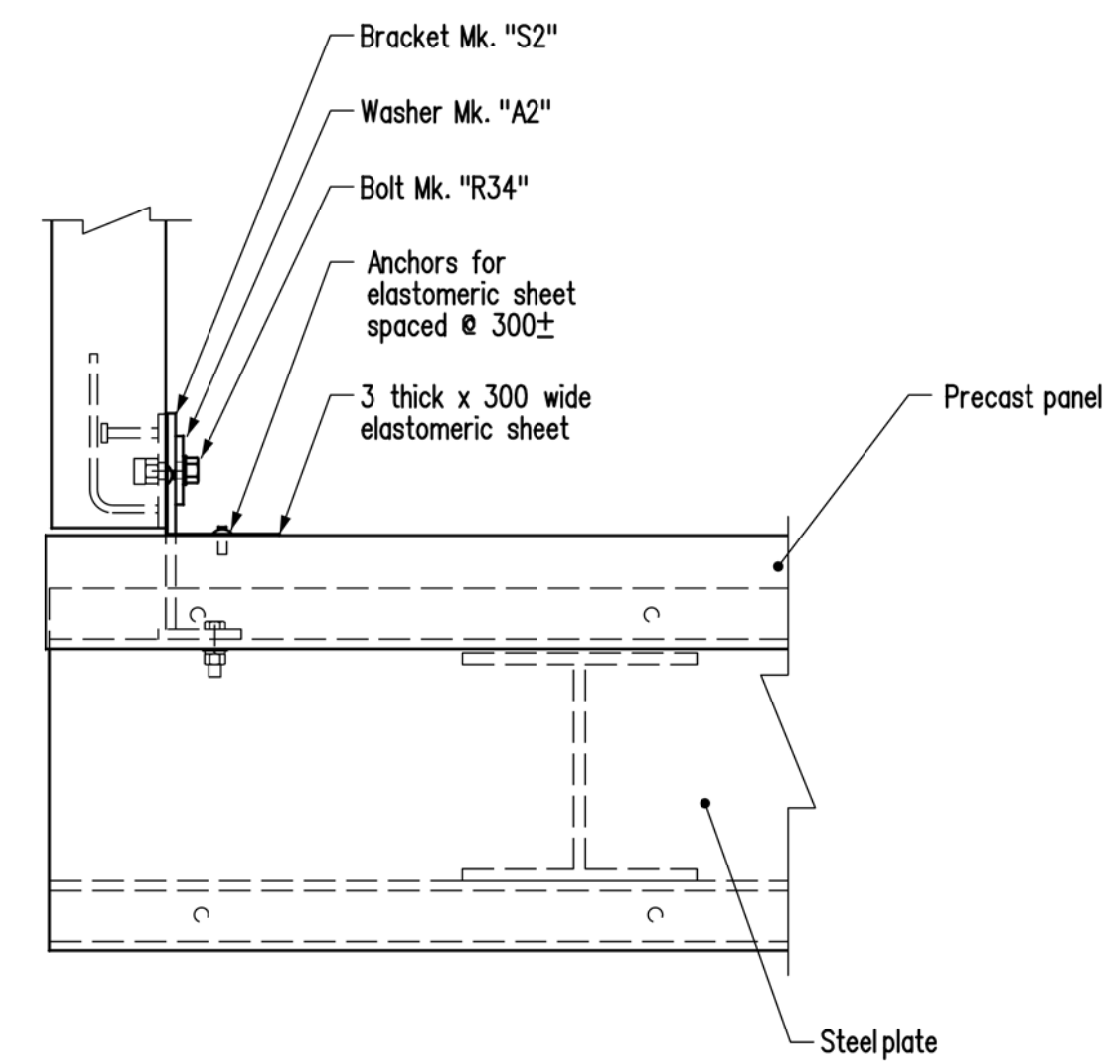
SECTION A-A

Scale 1:10



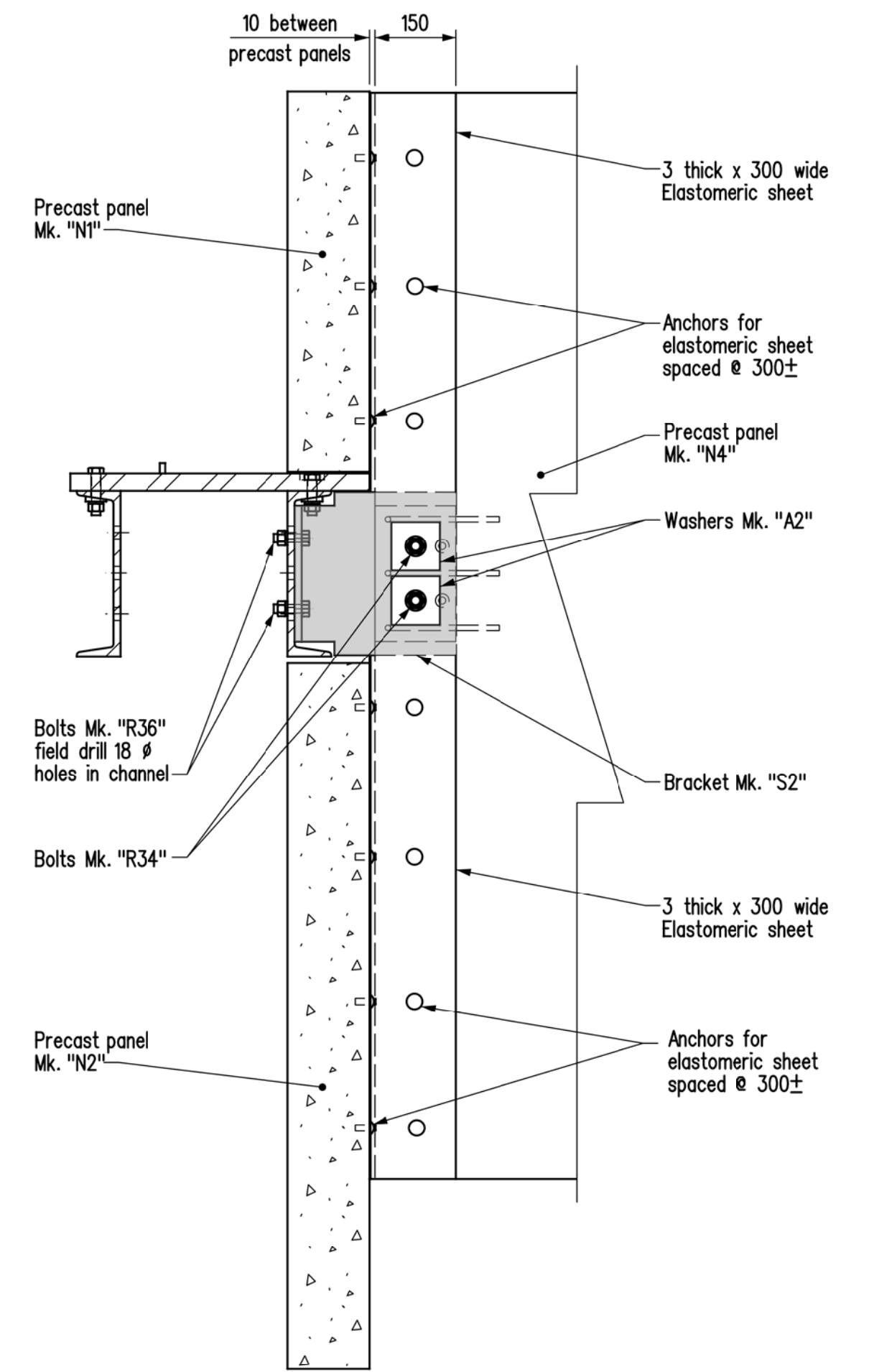
DETAIL A

Scale 1:10



DETAIL B

Scale 1:10




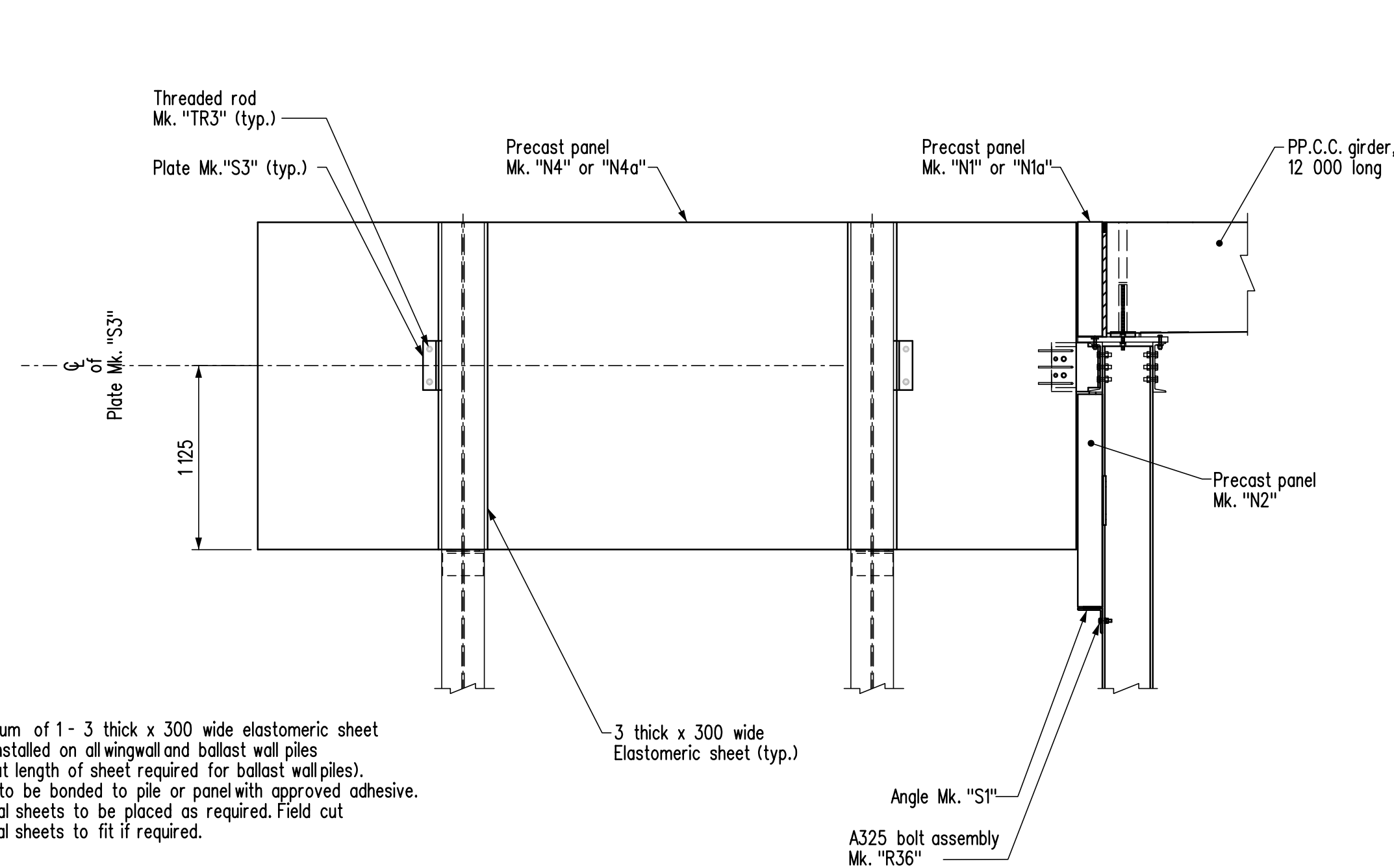
SECTION B-B

Scale 1:10

NOTES:

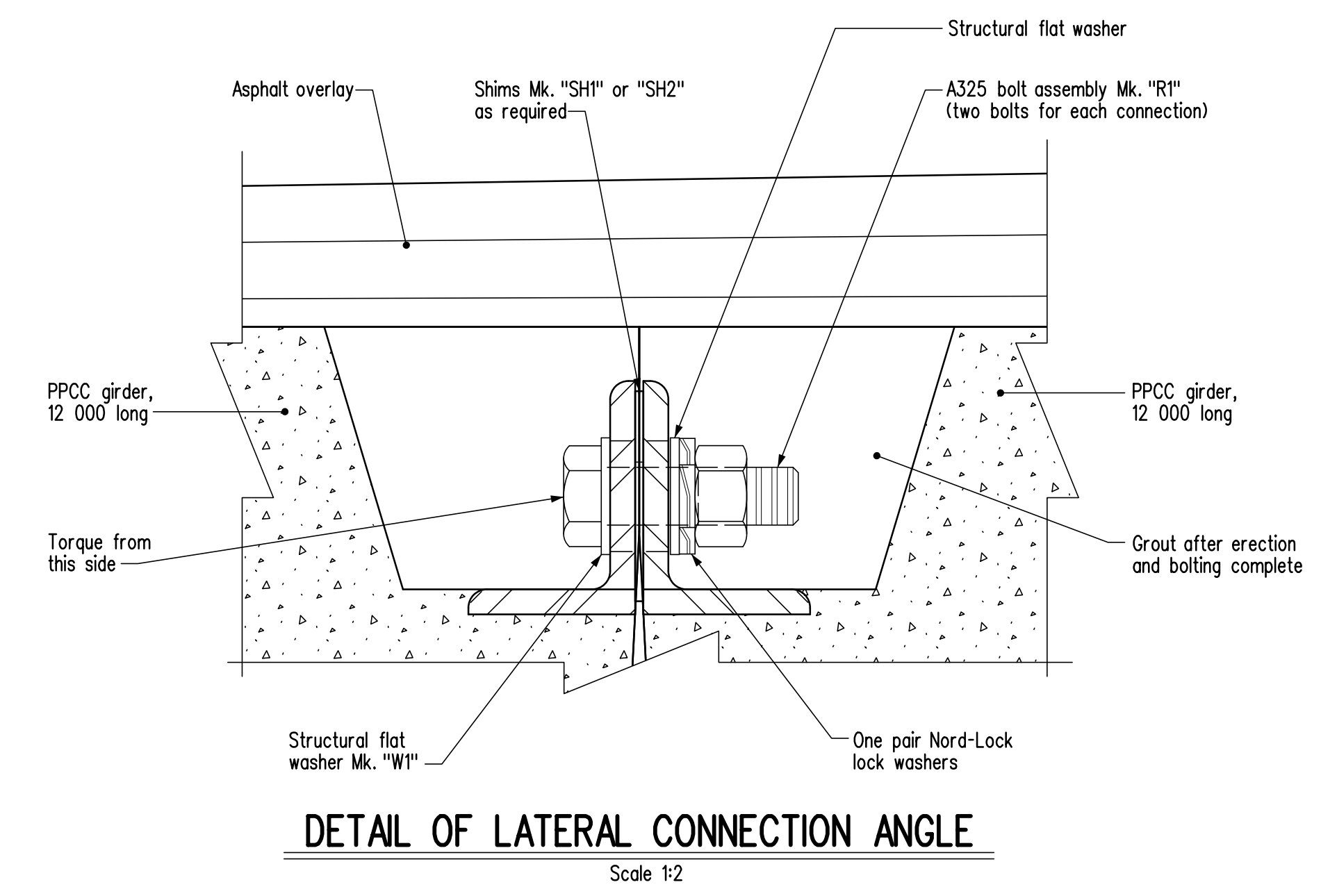
- For Section "B-B" and DETAIL "B" see Sheet No. .
- For "BILL OF MISCELLANEOUS METAL" see Sheet No. .
- The Contractor shall field drill 22 # holes in the precast panels for threaded rods Mk. "TR3". Should rebar be encountered, abandon hole, patch and drill in new location. Rebar locations are marked on the panels by the Panel Fabricator.
- Back faces of the upper and lower ballast walls shall be aligned in the same vertical plane.
- The Contractor shall ensure that the upper ballast walls are placed with the edge 5mm from \bar{C} of roadway.

REVISIONS		ASSEMBLY DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY: _____ DATE _____
DESIGN SEAL		RECORD SEAL	
PLACE ENGINEERS ELECTRONIC SEAL HERE			
		EXECUTIVE DIRECTOR OF STRUCTURES	
		DESIGN BY: _____ B.A.N.	SCALE: 1:40
		CHECKED: _____	SHEET No. 6
DETAILS		BY: _____ K.P. CHECKED: _____ or as shown SITE No. _____	

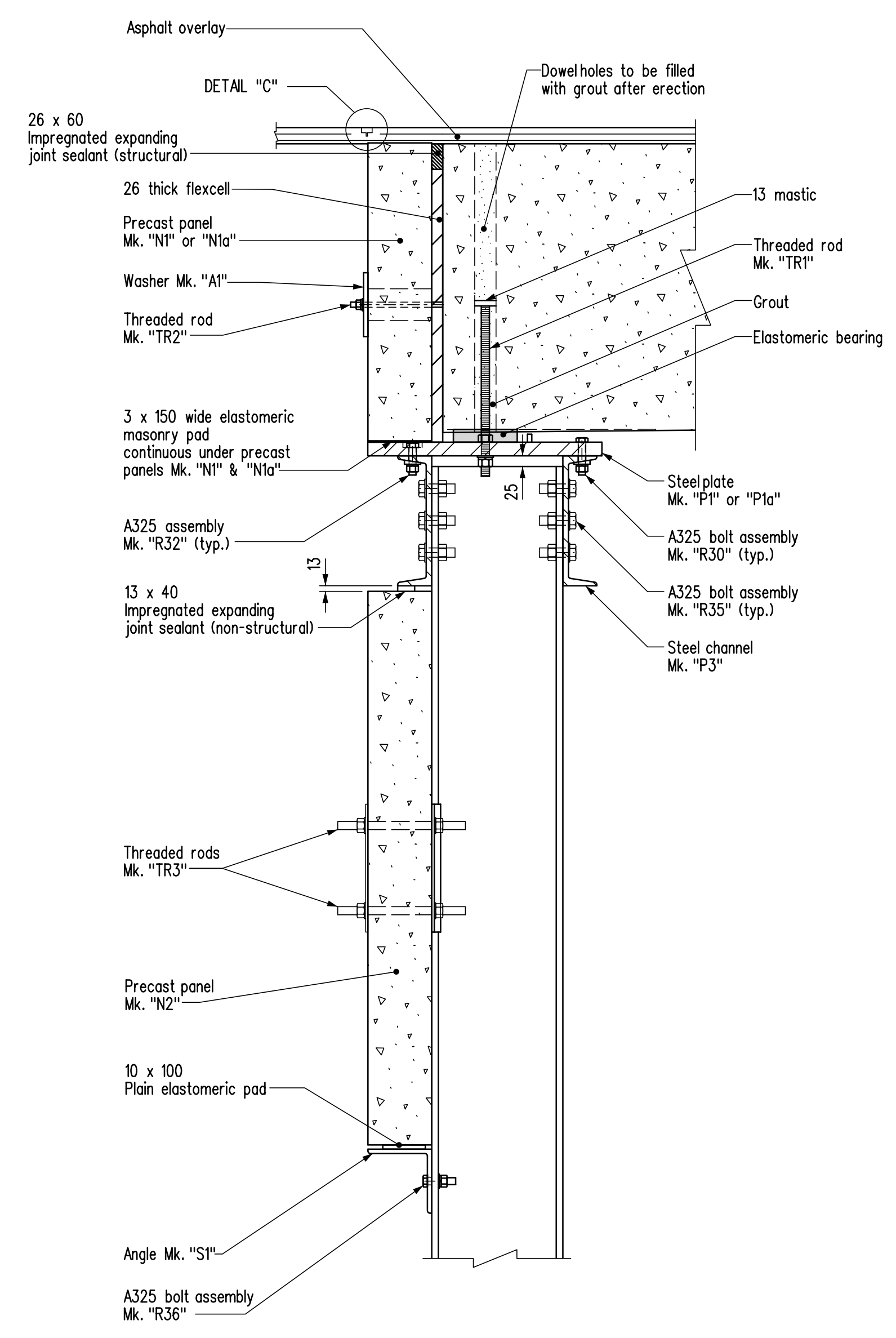


NOTE:
 A minimum of 1- 3 thick x 300 wide elastomeric sheet to be installed on all wingwall and ballast wall piles (field cut length of sheet required for ballast wall piles). Sheets to be bonded to pile or panel with approved adhesive. Additional sheets to be placed as required. Field cut additional sheets to fit if required.

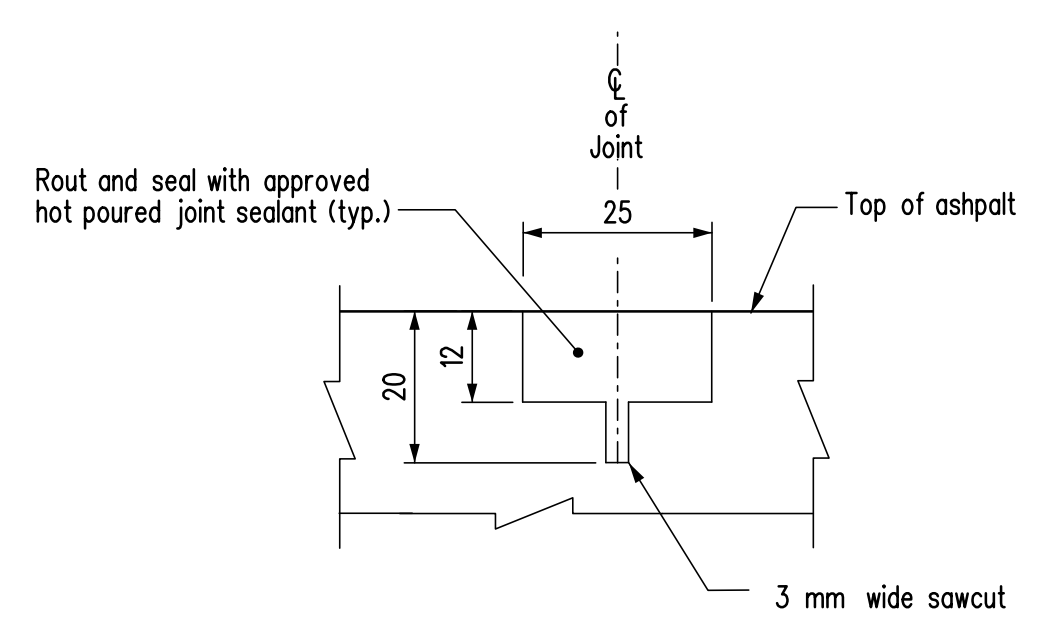
PART SIDE ELEVATION
 Bridge railing not shown for clarity



DETAIL OF LATERAL CONNECTION ANGLE
 Scale 1:2



SECTION AT ABUTMENT
 Scale 1:10



DETAIL "C"
 Scale 1:1

NOTES:

1. RE: BOLTING
 - a) GIRDER LATERAL CONNECTION
 - Bolts Mk. "R1" - c/w one F436 hardened washer, one structural plate washer Mk. "W1", one pair Nord-Lock washers and one Grade DH heavy hex. nut.
 - b) STEEL CAP
 - Bolts Mk. "R30" - One F436 hardened washer, one hardened bevel washer and one Grade DH heavy hex. nut.
 - Bolts Mk. "R32" - One hardened bevel washer and one Grade DH heavy hex. nut.
 - Bolts Mk. "R35" - Two F436 hardened washers and one Grade DH heavy hex. nut.
 - c) PRECAST PANELS
 - Bolts Mk. "R36" - Two F436 hardened washers and one Grade DH heavy hex. nut.
 - Bolts Mk. "R34" - One F436 hardened washer and one structural plate washer Mk. "A2", no nuts.
 - Threaded rod Mk. "TR2" - One standard flat washer, one structural lock washer, structural plate washer Mk. "A1" and one stainless steel hex. nut.
 - Threaded rod Mk. "TR3" - two filler plates Mk. "S4", one structural lock washer, two standard flat washers and two hex. nuts, Filler plate Mk. "S5" if required.
 - d) GIRDER TO STEEL CAP
 - Threaded rod Mk. "TR1" - one standard flat washer and structural lock washer and two hex. nuts.
 - e) High strength bolts shall be tightened by the turn-of-nut method as per Bridge Specifications. Ensure nuts are lubricated prior to bolting.
 - f) Fill counter bored holes with mastic filler after tightening bolts.
2. When grouting dowel holes in girders, ensure that there is no grout between bottom of girder and bearing plate.
3. Apply galvalloy to all field welds & areas where galvanizing has been damaged.
4. Impregnated expanding joint sealant shall be installed as per manufacturer's recommendations.

REVISIONS		ASSEMBLY DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:

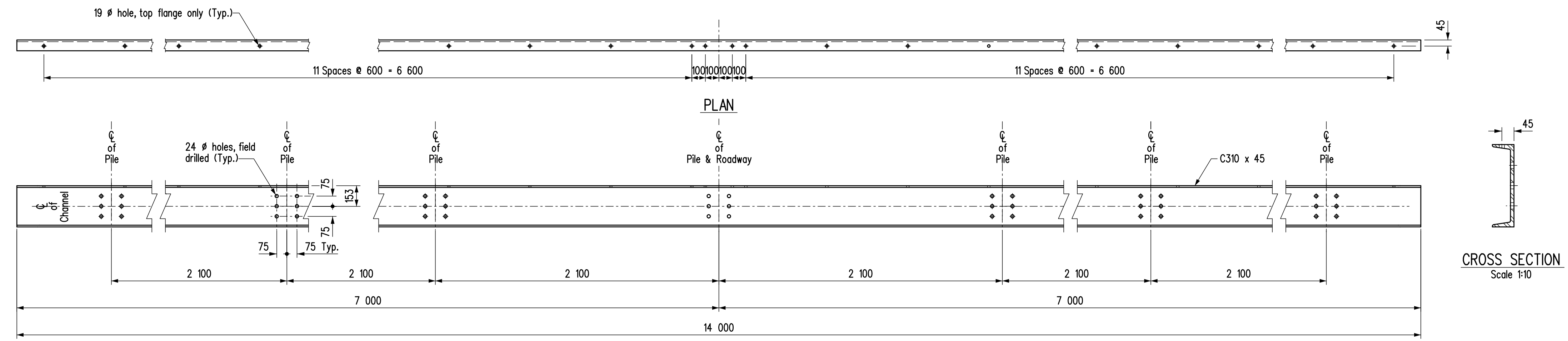
PLACE ENGINEERS ELECTRONIC SEAL HERE

Manitoba
Infrastructure
Water Management and Structures

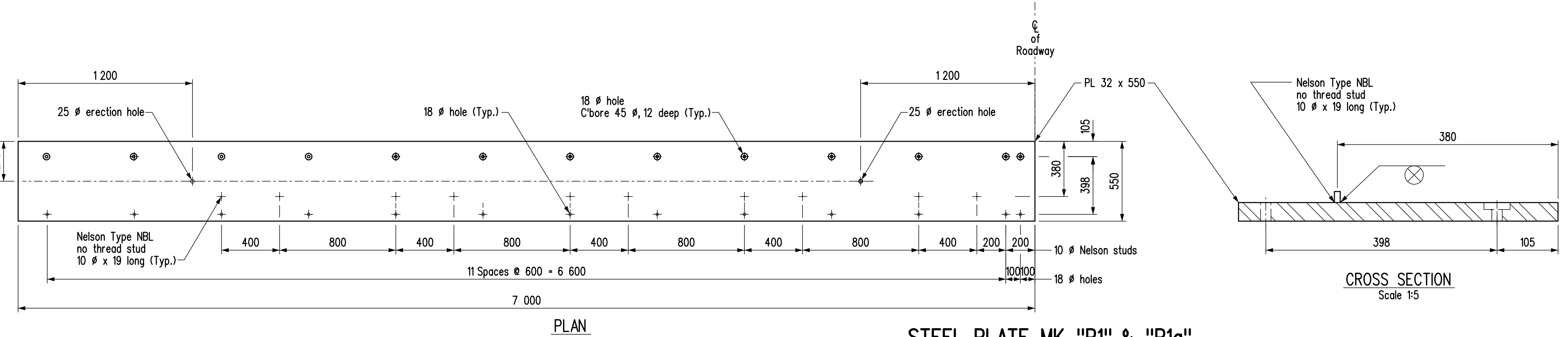
BY: _____ B.A.N.
CHECKED: _____
BY: _____ K.P.
CHECKED: _____

EXECUTIVE DIRECTOR OF STRUCTURES DATE

SCALE: 1:30 SHEET No. 7
or as shown SITE No. _____



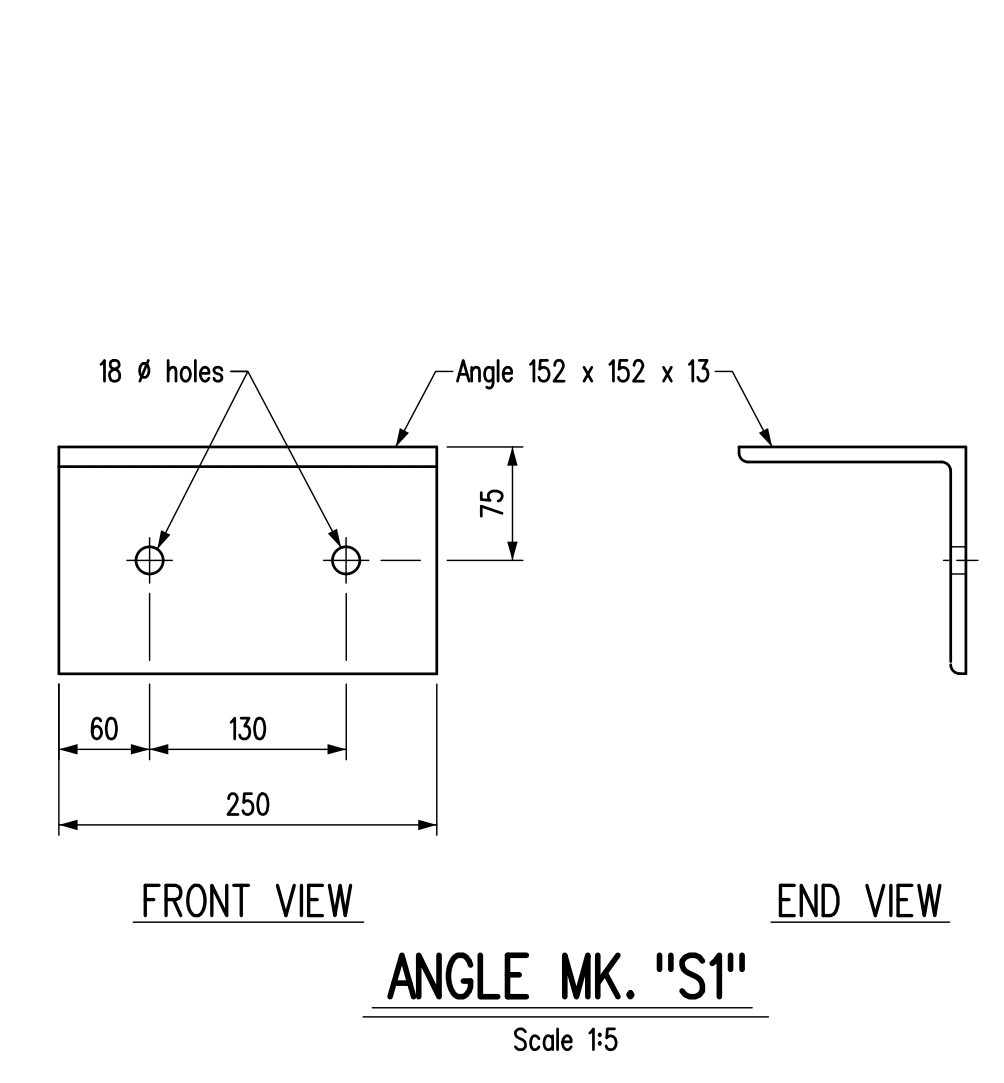
ELEVATION
STEEL CHANNEL MK "P3"



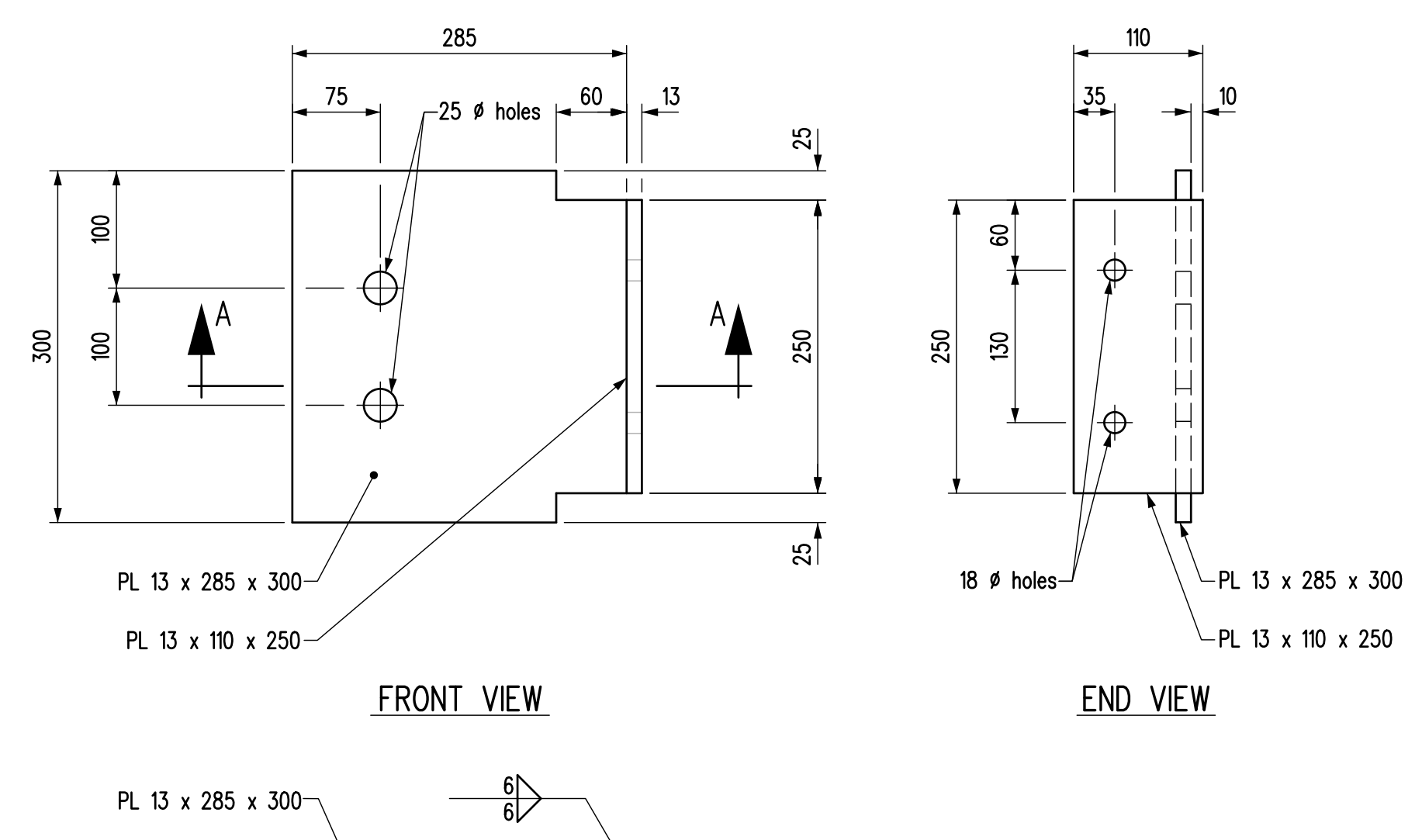
FOR ABUTMENTS
STEEL PLATE MK "P1" & "P1a"
Plate Mk. "P1" as shown, Plate "P1a" opposite hand

BILL OF MISCELLANEOUS METAL						12 000 ROADWAY WIDTH - 1 SPAN		Site No.		
MARK No.	No.	DESCRIPTION	CORROSION PROTECTION	SIZE	LENGTH	REMARKS	COMPONENT MASS	MASS PER UNIT	TOTAL MASS	
P1	2	Steel plate	Hot dip galvanized						1934.48	
		Each unit to be fabricated from:								
		1 - Steel plate		PL 32x550	7 000	See detail for Abutment	967.120	967.120		
		10 - Nelson Type NBL, no thread studs		10 dia.	19	Part No. 101-063-167	0.012	0.120		
								967.240		
P1a	2	Steel plate	Hot dip galvanized						1934.48	
		Each unit to be fabricated from:								
		1 - Steel plate		PL 32x550	7 000	See detail for Abutment	967.120	967.120		
		10 - Nelson Type NBL, no thread studs		10 dia.	19	Part No. 101-063-167	0.012	0.120		
								967.240		
P3	4	Steel channel	Hot dip galvanized	C310x45	14 000	See detail for Abutment			2503.20	
R30	52	A325 bolt assembly	Hot dip galvanized	16 dia.	89	Steel plate to channels		0.245	12.74	
R32	52	A325 bolt assembly	Hot dip galvanized	16 dia.	76	Steel plate to channels C bore holes		0.225	11.70	
R35	168	A325 bolt assembly	Hot dip galvanized	22 dia.	64	Channels to piles		0.461	77.45	
R36	52	A325 bolt assembly	Hot dip galvanized	16 dia.	64	Angles Mk. "S1" to piles & bracket Mk. "S2" to cap		0.205	10.66	
S1	22	Angle	Hot dip galvanized	L 152x152x13	250	As detailed		7.250	159.59	
S2	4	Bracket	Hot dip galvanized	As detailed		As detailed		11.226	44.90	
SS	16	Plate	Hot dip galvanized	PL 6x300		As detailed		3.223	51.57	
S4	32	Filler plate	Hot dip galvanized	PL 6x100		As detailed		1.413	45.22	
SS	16	Filler plate	Hot dip galvanized	PL 3x100		As detailed		0.707	11.31	
A1	16	Structural plate washer	Hot dip galvanized	PL 10x150	150	As detailed - One to threaded rod Mk. "TR2"		1.766	28.26	
A2	8	Structural plate washer	Hot dip galvanized	PL 10x90	90	As detailed - One to bolt Mk. "R34"		0.636	5.09	
TR1	20	Threaded rods c/w tv o hex. nuts	Hot dip galvanized	19 dia.	0	Girder to steel cap plate		0.940	18.80	
TR3	32	Threaded rods c/w tv o hex. nuts	Hot dip galvanized	19 dia.	0	Steel plates Mk. "S3" to precast panels		0.660	21.12	
		104 Hardened bevel washer	Hot dip galvanized	for 16 dia. bolts		One to bolts Mk. "R30" & "R32"		0.110	11.44	
		20 Standard flat washer	Hot dip galvanized	for 13 dia. rod		One to threaded rod Mk. "TR2"		0.010	0.20	
		84 Standard flat washer	Hot dip galvanized	for 19 dia. rod		One to "TR1", two to "TR3"		0.020	1.68	
		20 Structural lock washer	Hot dip galvanized	for 12 dia. rod		One to threaded rod Mk. "TR2"		0.010	0.20	
		52 Structural lock washer	Hot dip galvanized	for 19 dia. rod		One to "TR1" & "TR3"		0.020	1.04	
		168 F436 Hardened washer	Hot dip galvanized	for 22 dia. bolts		One to bolt Mk. "R35"		0.032	5.38	
		52 F436 Hardened washer	Hot dip galvanized	for 16 dia. bolts		One to bolt Mk. "R36"		0.014	0.73	
R1	72	A325 bolt assembly	Hot dip galvanized	22 dia.	76	R.C. girder connection		0.499	35.93	
W1	72	Structural flat washer	Hot dip galvanized	for 22 dia. bolts		One to bolt Mk. "R1"		0.050	3.60	
		72 Pair Nord-Lock lock washers		for 22 dia. bolts		One pair to bolt Mk. "R1"		0.020	1.44	
SH1	36	Shim plate	Hot dip galvanized	PL 2.5x80	180	As detailed - use as required		0.231	8.32	
SH2	36	Shim plate	Hot dip galvanized	PL 5x80	180	As detailed - use as required		0.463	16.67	
							TOTAL MASS (kg) =	6957.09		

NOTES:
 1. All material noted in the above Bill shall be hot dip galvanized after fabrication in accordance with CSA G164 for a minimum net retention of 610 g/m² unless otherwise stated in the specified material ASTM standards. The fabricator and galvanizer shall safeguard against embrittlement using recommended practices from applicable standards.
 2. Seal all welds prior to galvanizing.
 3. Apply Galvaloy to all field welds and areas where galvanizing has been damaged.
 4. All bolts and threaded rod in the above Bill shall be Imperial thread.



ANGLE MK. "S1"
Scale 1:5



BRACKET MK. "S2"
Scale 1:5

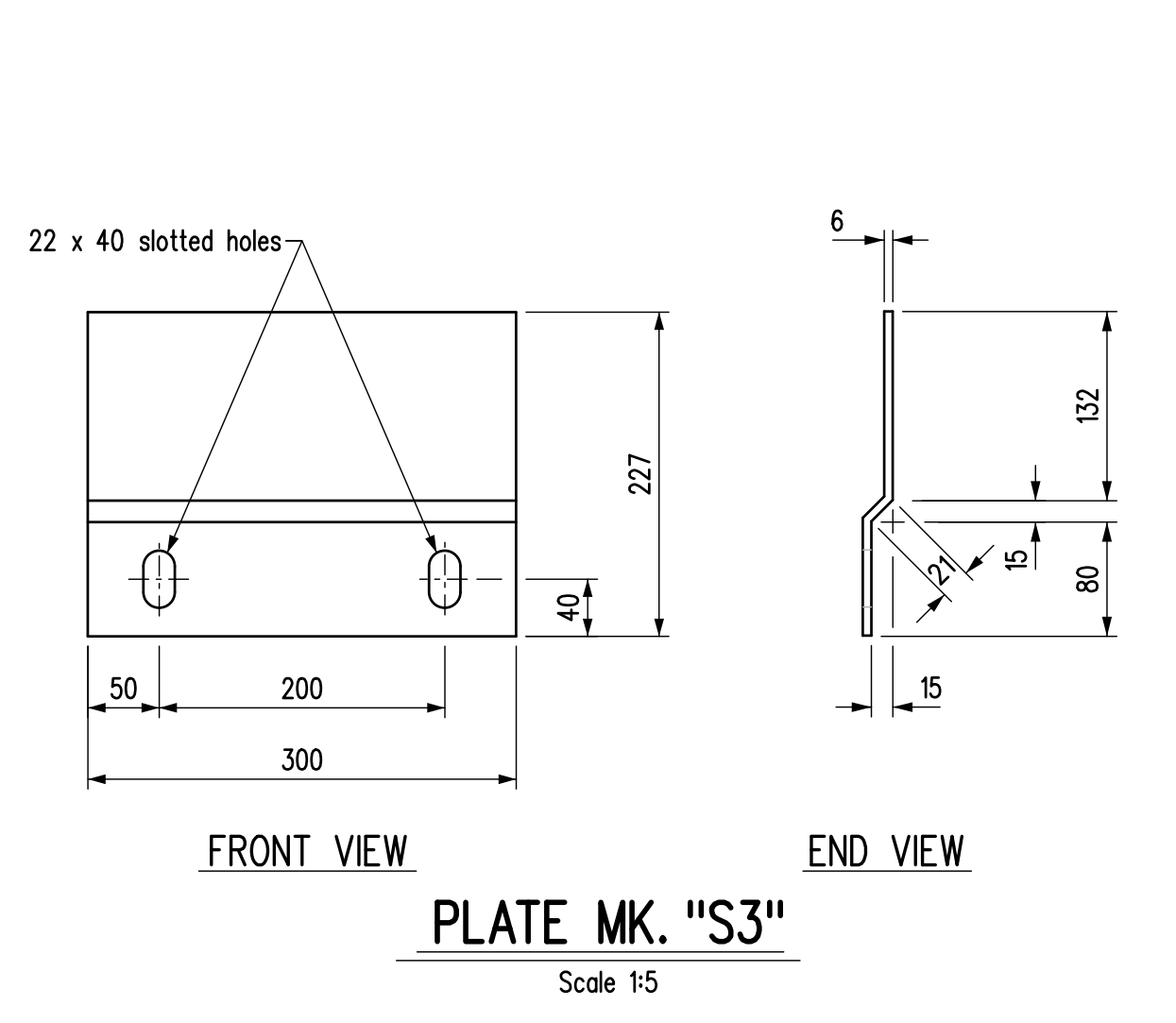
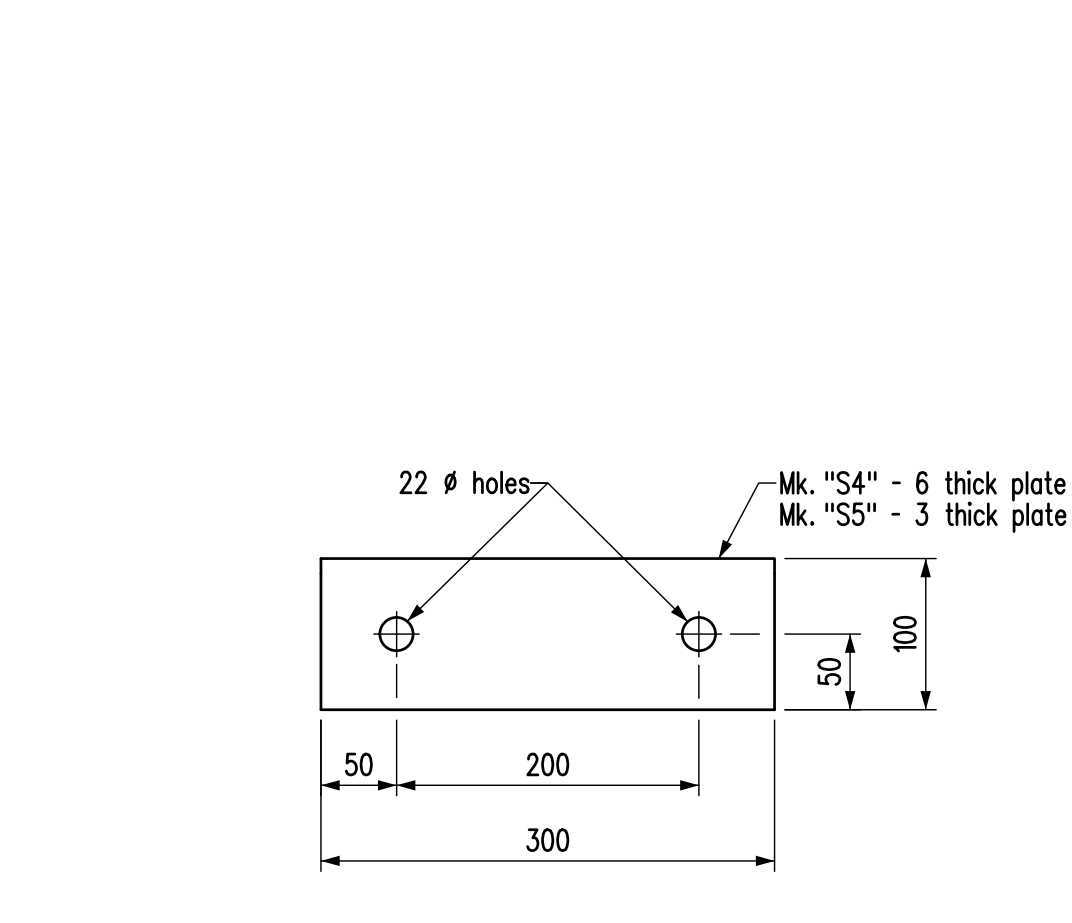
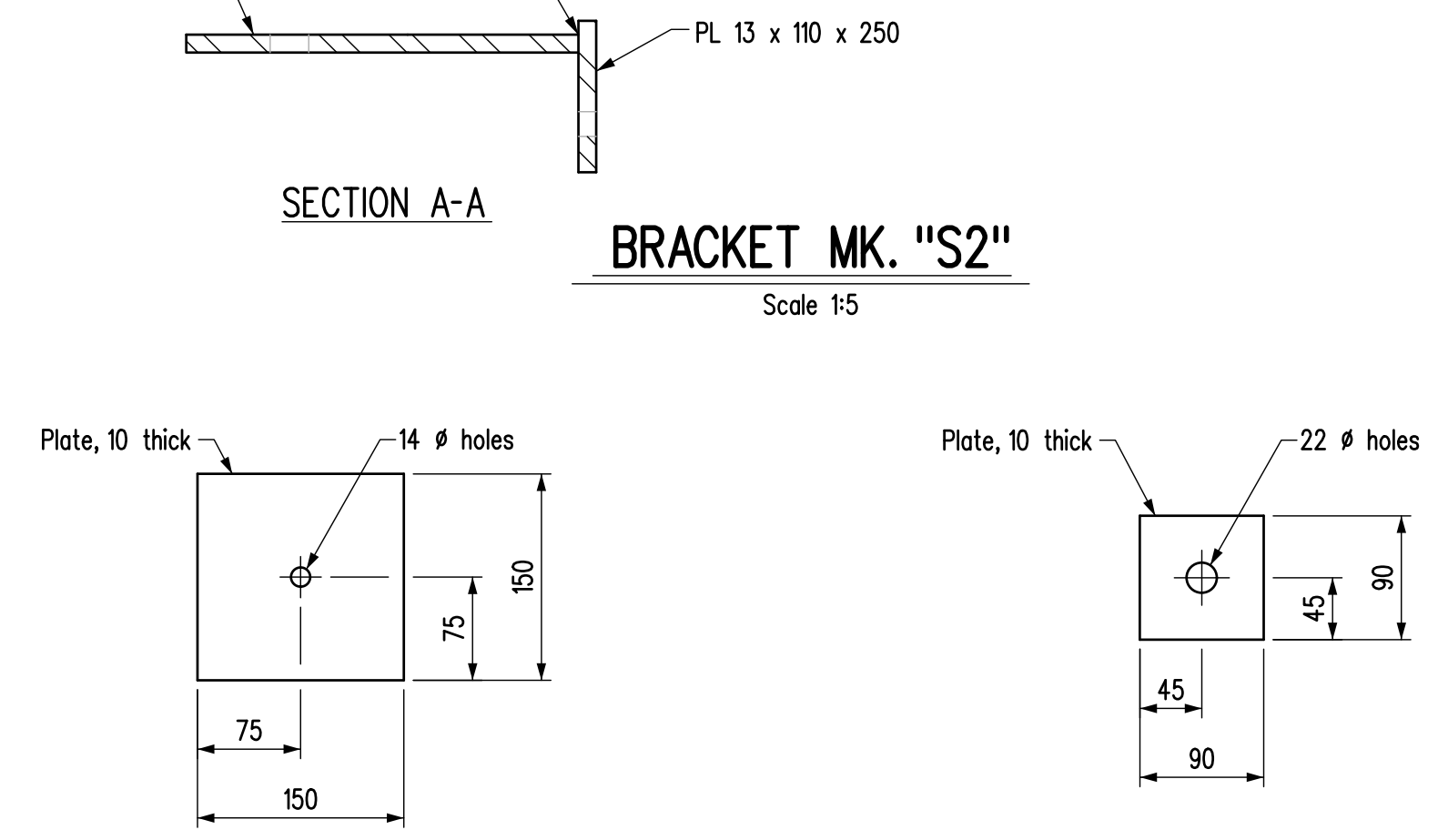


PLATE MK. "S3"
Scale 1:5

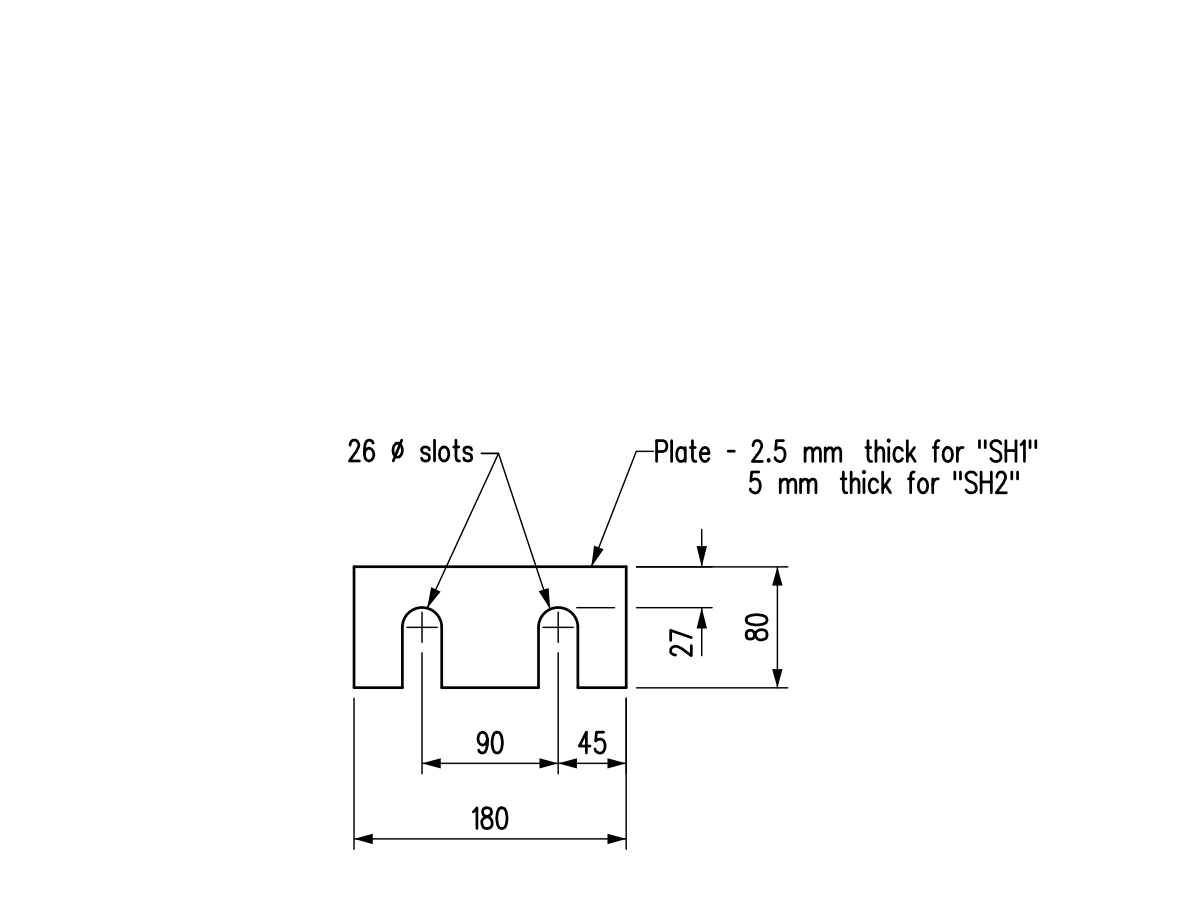


FILLER PLATES MK. "S4" & "S5"
Scale 1:5



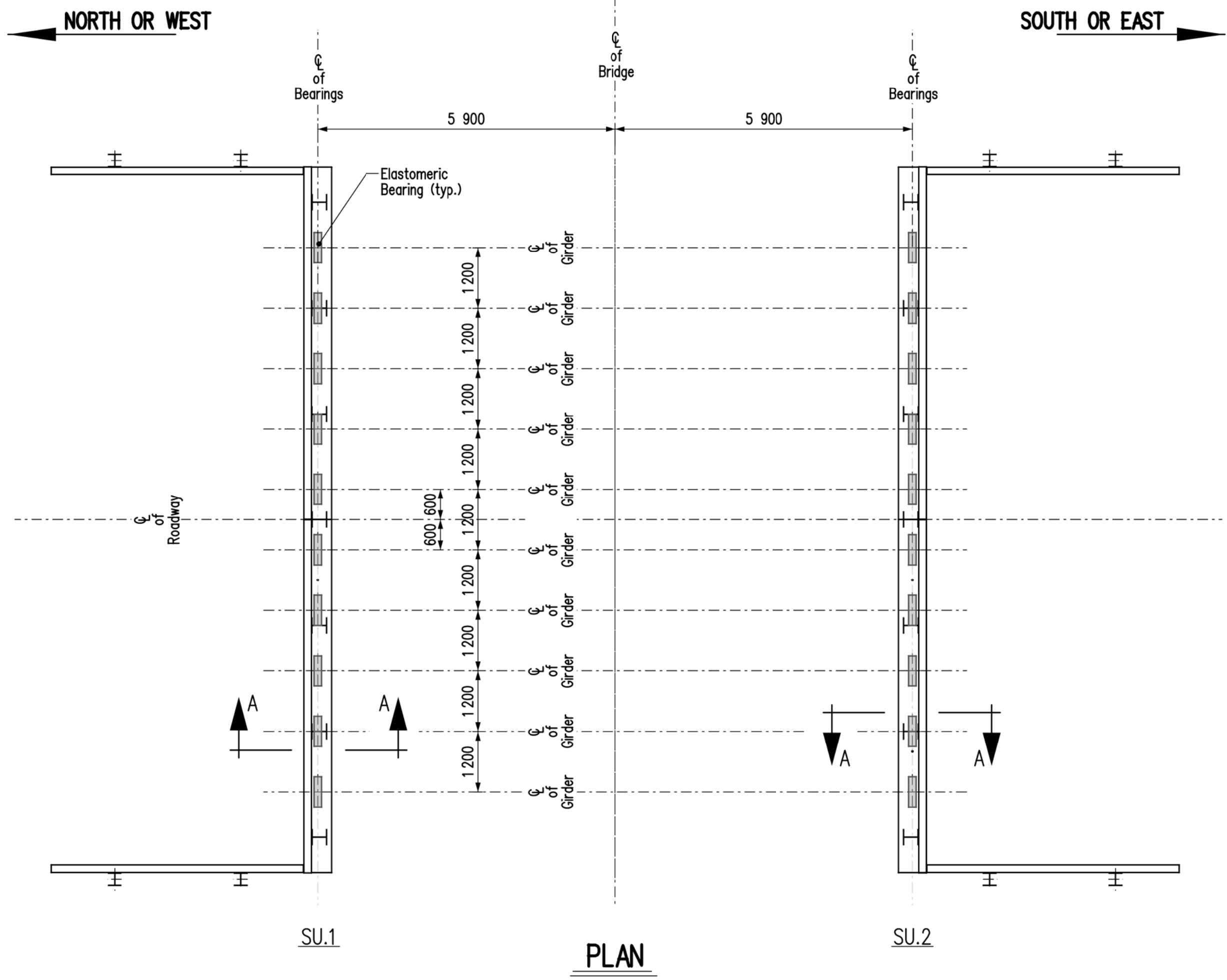
WASHER MK. "A1"
Scale 1:5

WASHER MK. "A2"
Scale 1:5



SHIM PLATES MK. "SH1" & "SH2"
Scale 1:5

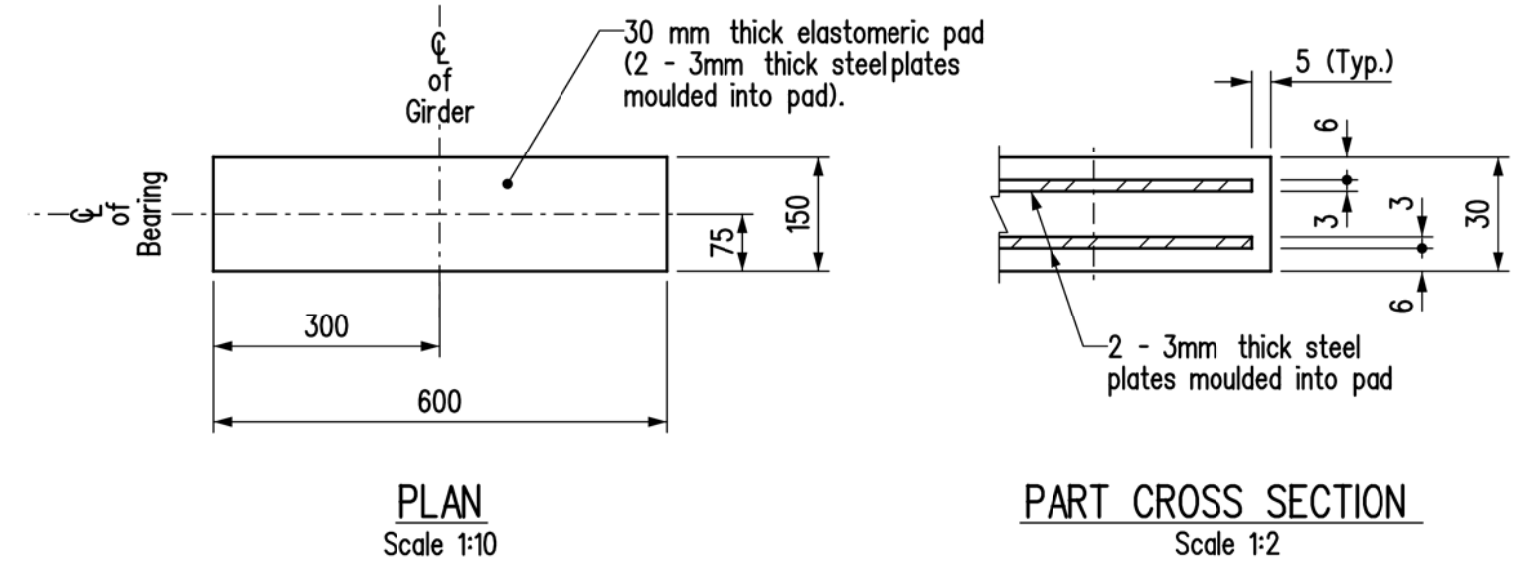
REVISIONS		STEEL PILE CAP DETAILS	
DATE	BY	DESIGN SEAL	RECORD SEAL
<p>Manitoba Infrastructure Water Management and Structures</p>		<p>EXECUTIVE DIRECTOR OF STRUCTURES DATE</p>	
		<p>SCALE: 1:20 SHEET No. 8</p>	
<p>DESIGN BY: B.A.N.</p>		<p>RELEASED FOR CONSTRUCTION BY:</p>	
<p>CHECKED: K.P.</p>		<p>or as shown SITE No. 222</p>	
<p>DETAILS BY: K.P.</p>			
<p>CHECKED:</p>			



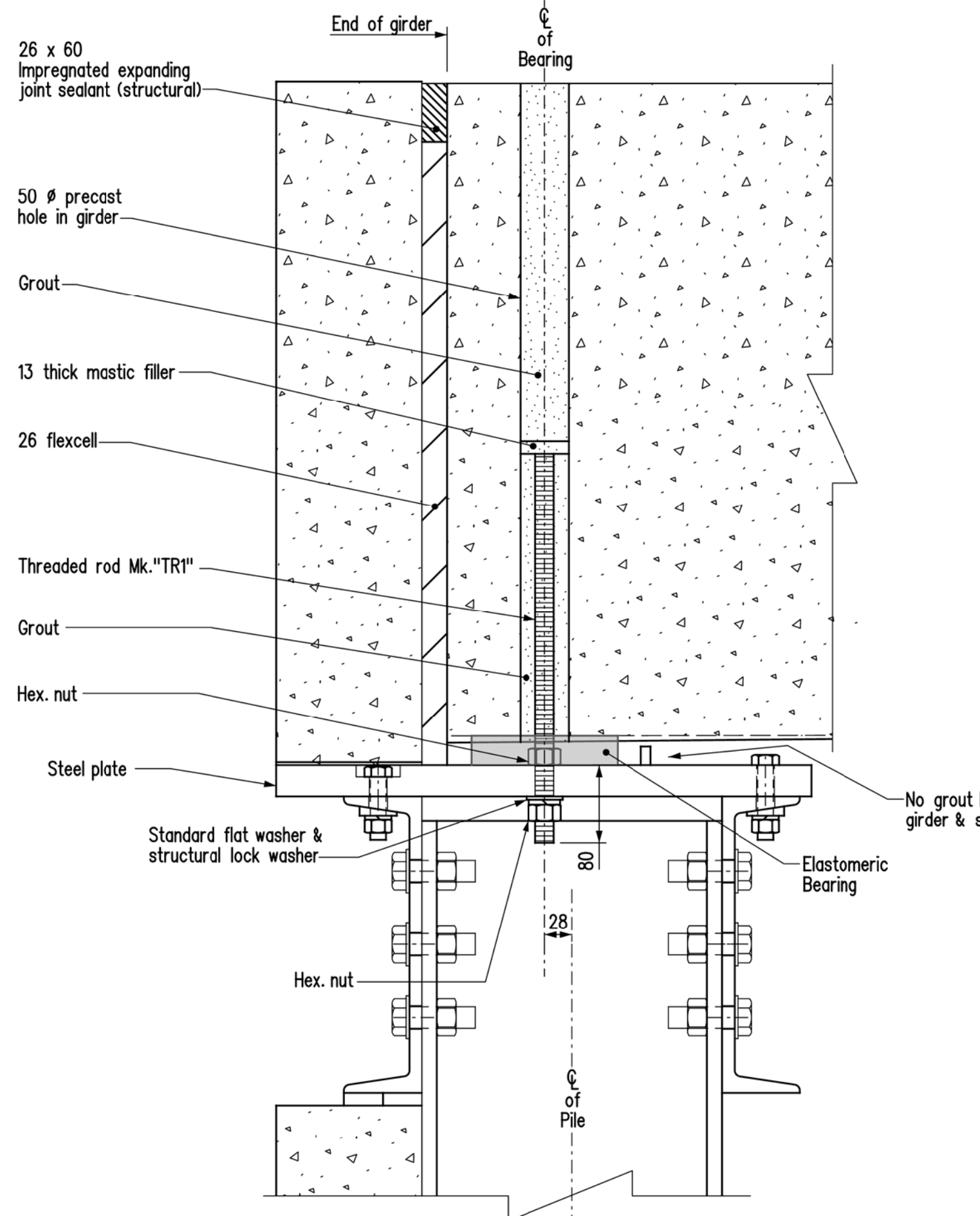
BILL OF BEARINGS			12 000 ROADWAY WIDTH - 1	Site No.
SPAN				
No.	LOCATION	DESCRIPTION	REMARKS	
20	SU.1 - SU.2	Elastomeric bearings	As detailed	

NOTE:

- Elastomer shall be natural rubber. Elastomer shall be AASHTO low temperature Grade 5 with a minimum shear modulus $G \geq 0.9$ MPa and a 60 durometer Shore A hardness.
- Internal steel reinforcing plates for laminate bearings shall be rolled mild steel with a minimum yield strength of 300 Mpa.



ELASTOMERIC BEARINGS



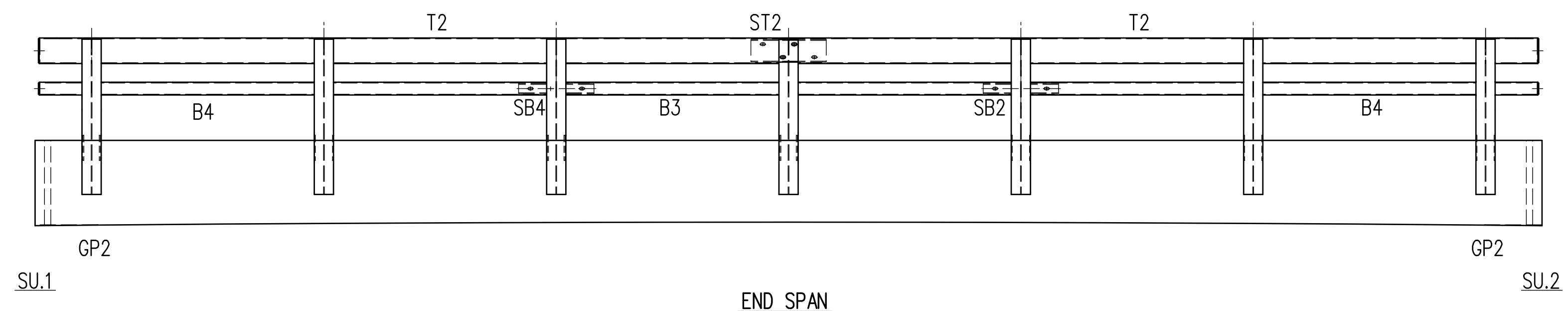
SECTION "A-A"

Threaded rods at SU.1 & SU.2. See sheet No. 6 for layout.
Scale 1:5

NOTE :
Threaded rods Mk. "TR1", set exactly on centreline of holes in precast girders and grouted. Field drill 21 dia. hole in steel plate to match girders.

- NOTES:**
- Re: Girder Erection Operations Behind Abutment Ballast Walls
- Surcharge loading on the backfill resulting from girder erection operations shall be minimized near the precast concrete ballast walls and wingwalls.
 - Where possible, girder erection equipment shall be positioned such that there are no surcharge loads behind the back face of the precast panels within a distance equal to the depth of backfill to the bottom of the panels at the time of girder erection.
 - Should the Contractor propose to encroach on this zone, the following requirements must be satisfied:
 - Submit a girder erection procedure for approval outlining type, configuration, weights and locations of equipment including expected tipping forces on crane outriggers, etc.
 - Perform all precautionary measures outlined by the Department as a result of that submission.
 - All surcharge loads encroaching in this zone must be distributed over an area not less than 2.0 m².

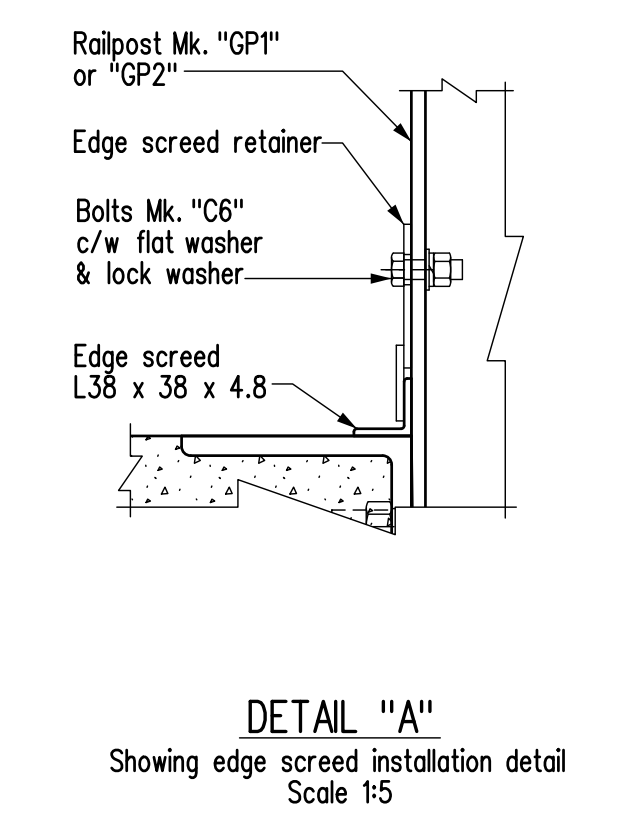
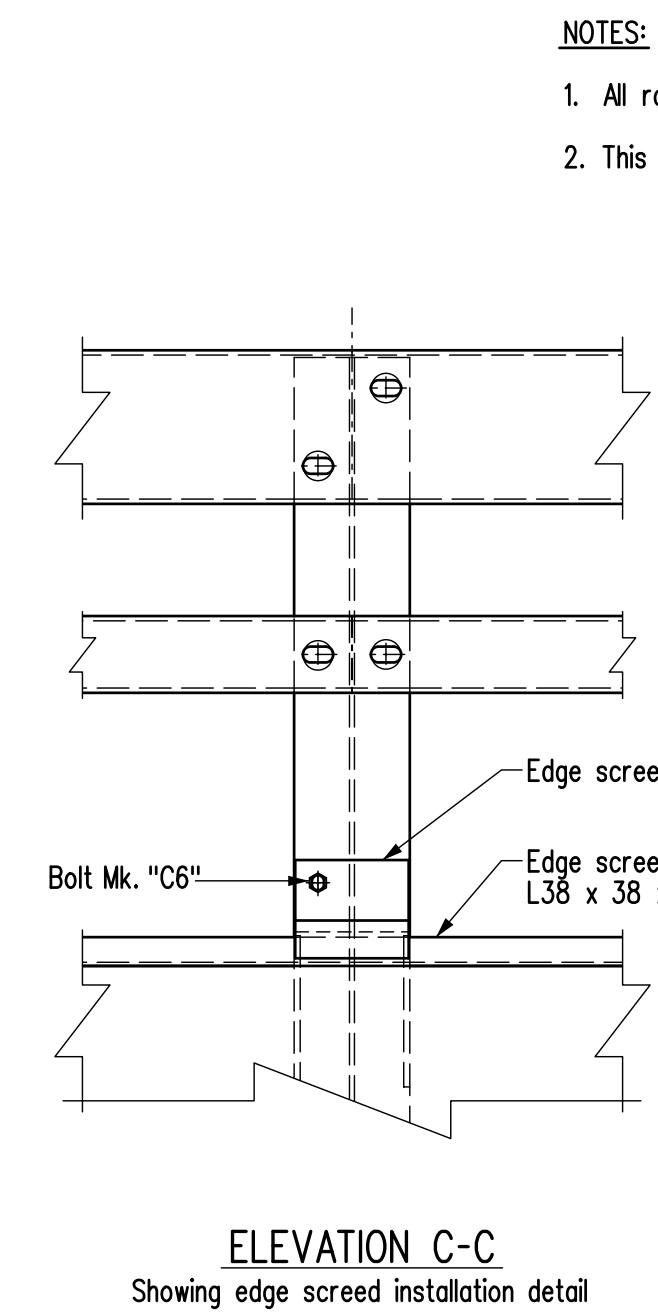
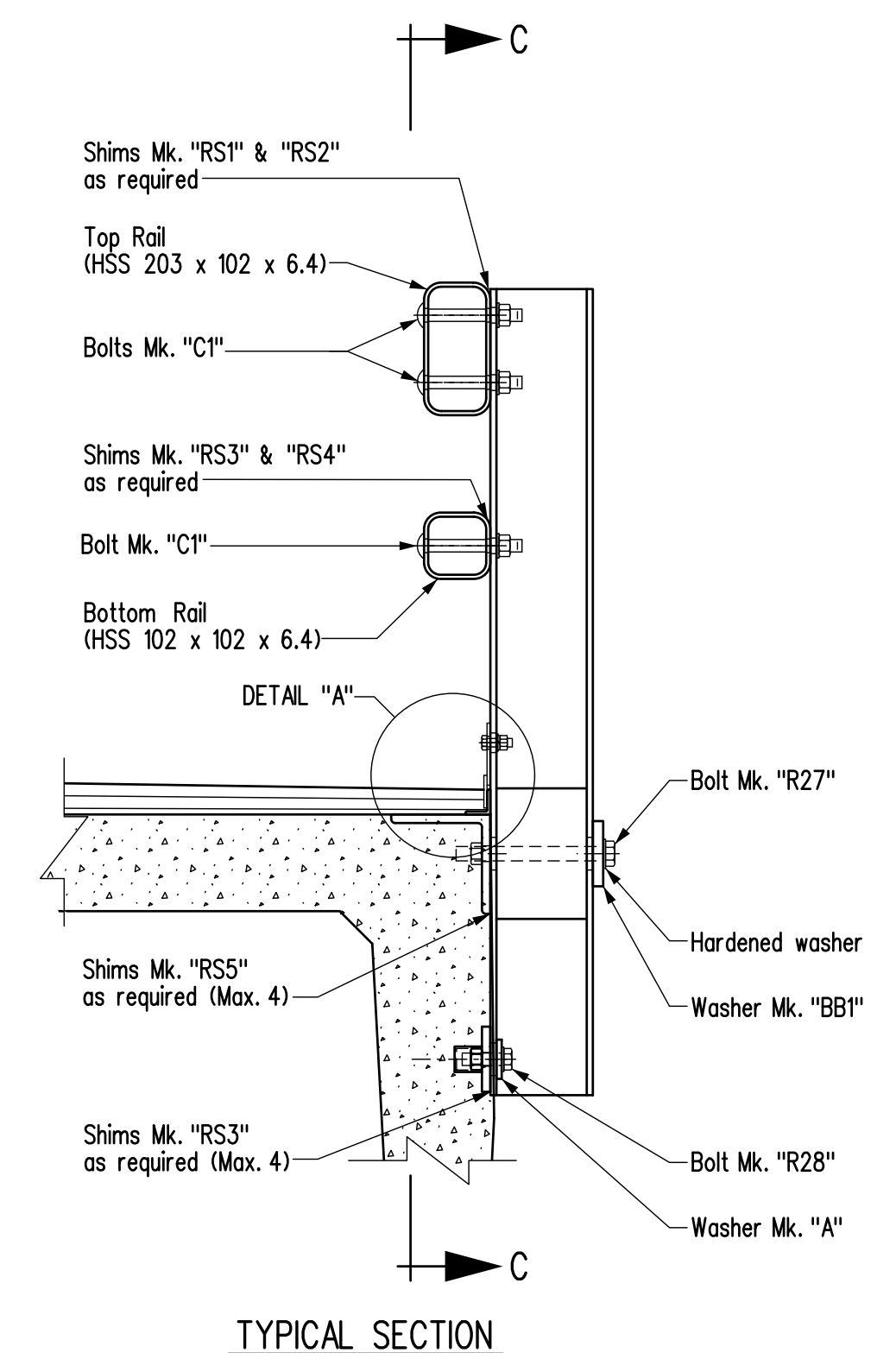
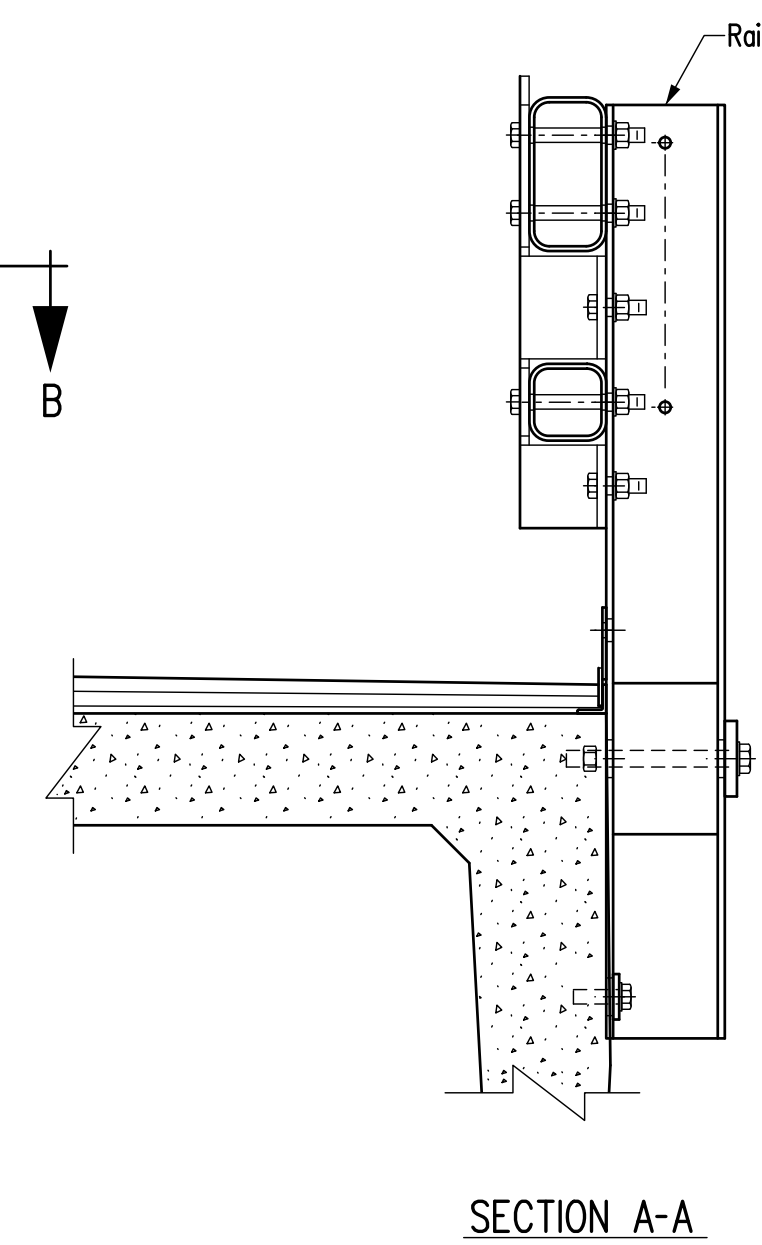
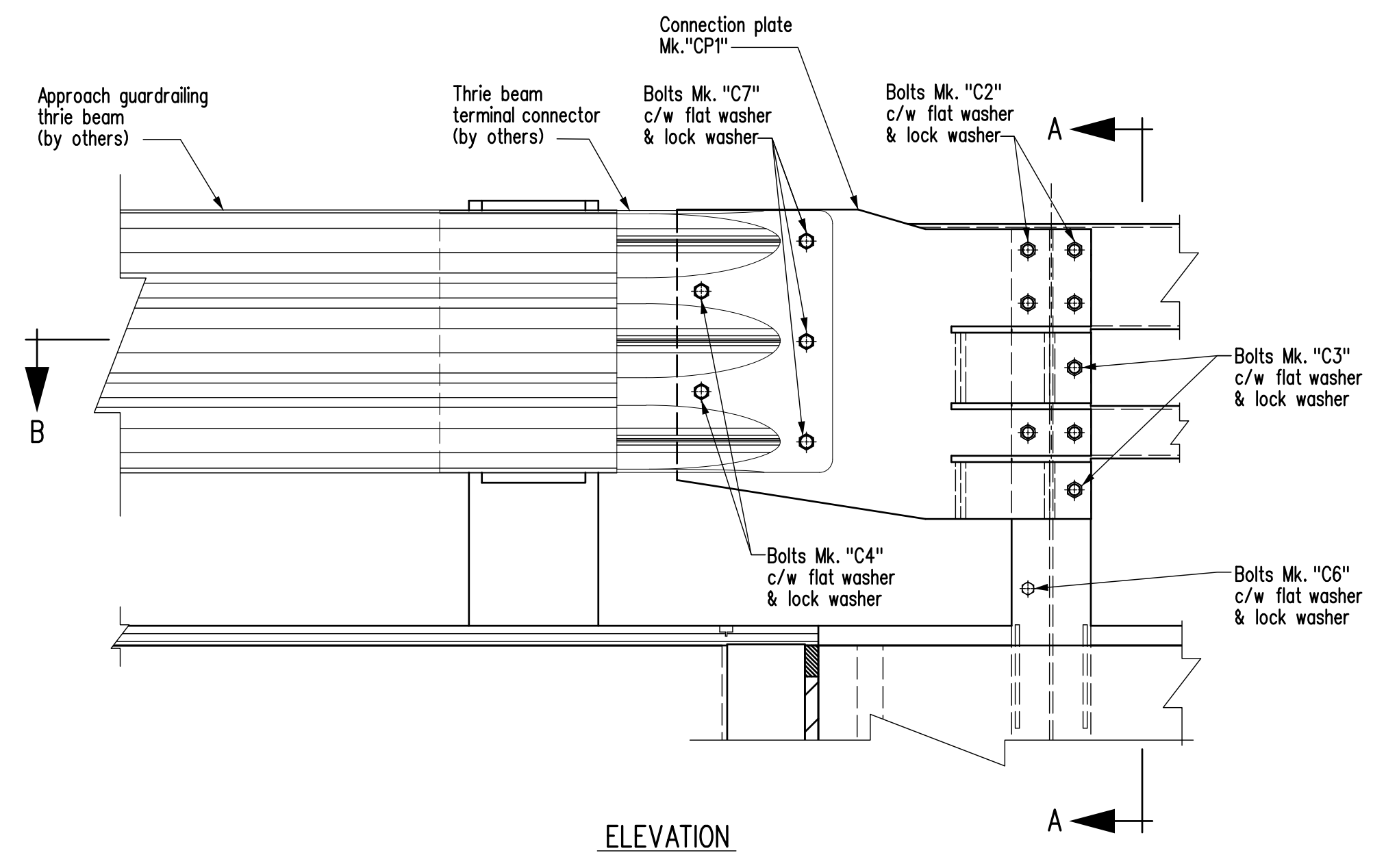
REVISIONS		BEARING AND ERECTION DETAILS		
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:	
DESIGN SEAL	RECORD SEAL			
PLACE ENGINEERS ELECTRONIC SEAL HERE			Manitoba Infrastructure Water Management and Structures	
			DESIGN BY: <u>B.A.N.</u>	EXECUTIVE DIRECTOR OF STRUCTURES DATE
			CHECKED: _____	SCALE: 1:75 SHEET No. 9
			DETAILS BY: <u>K.P.</u>	or as shown SITE No. _____
CHECKED: _____				



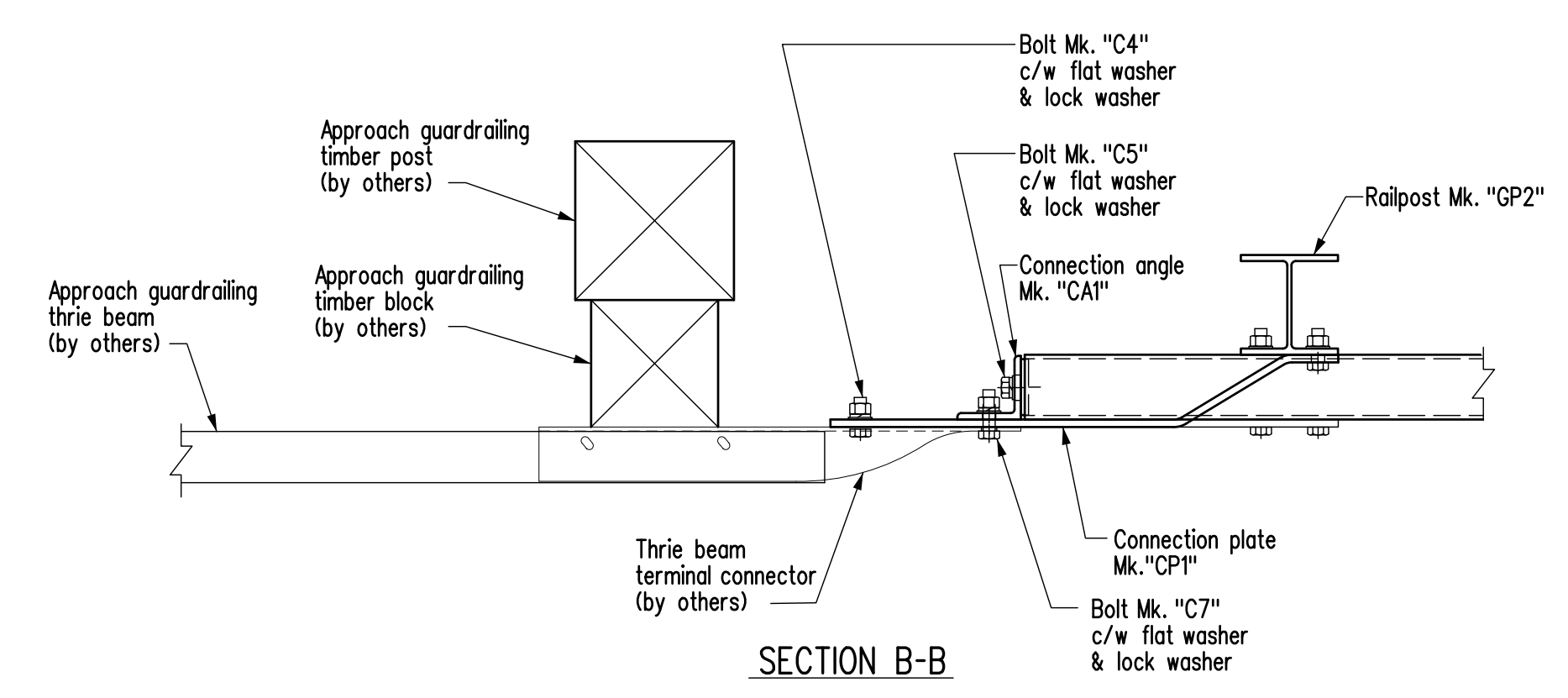
RAILS		SLEEVES		RAILPOSTS		
T2	B3	B4	ST2	SB2	GP1	GP2
4	2	4	2	4	10	4

RAILING LAYOUT

Not to Scale



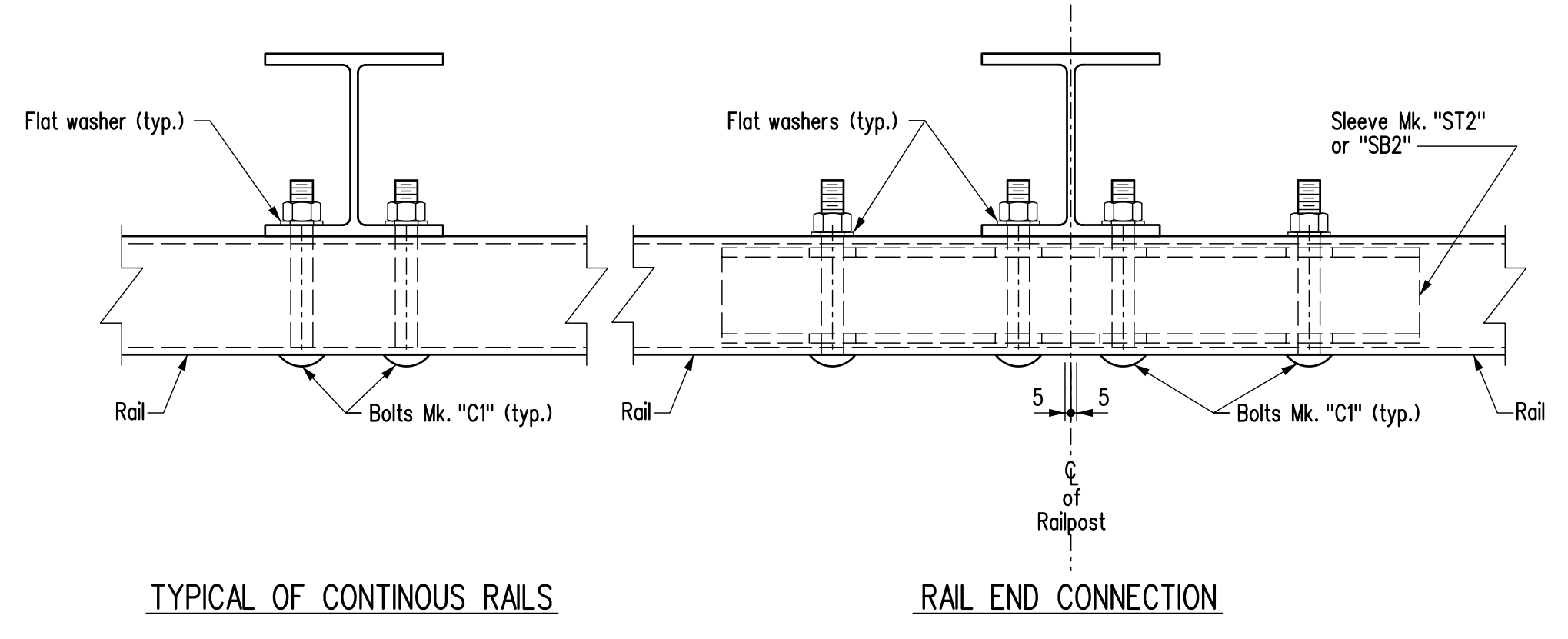
- NOTES:**
1. All railposts shall be Mk. "GP1" unless noted otherwise.
 2. This sheet to be read in conjunction with Sheets & .



APPROACH RAIL CONNECTION DETAILS

RAILPOST ERECTION DETAILS

- NOTES:**
1. High strength bolts Mk. "R27" & "R28" shall be tightened by turn-of-nut method as per Specification 1061. These bolts to be supplied by the Girder Fabricator. For quantities see Bill of Miscellaneous Metal on Girder sheet.
 2. High strength bolted connection may be shimmed to a maximum of 12 mm with shims Mk. "RS3" & "RS4".



RAILING ERECTION DETAILS

Scale 1:5

REVISIONS		RAILING LAYOUT AND DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:

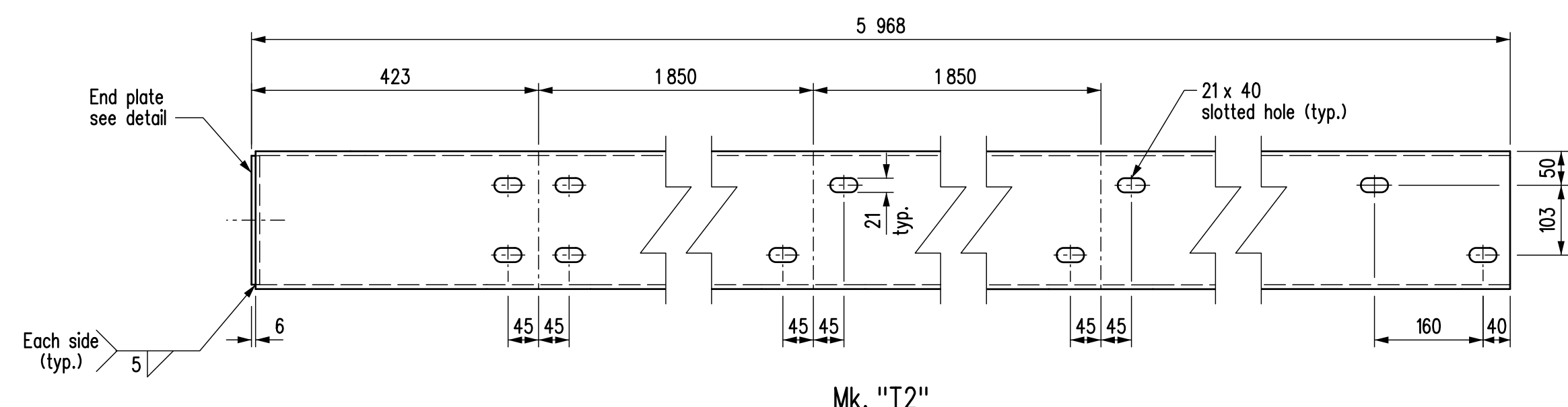
Manitoba
Infrastructure
Water Management and Structures

EXECUTIVE DIRECTOR OF STRUCTURES DATE

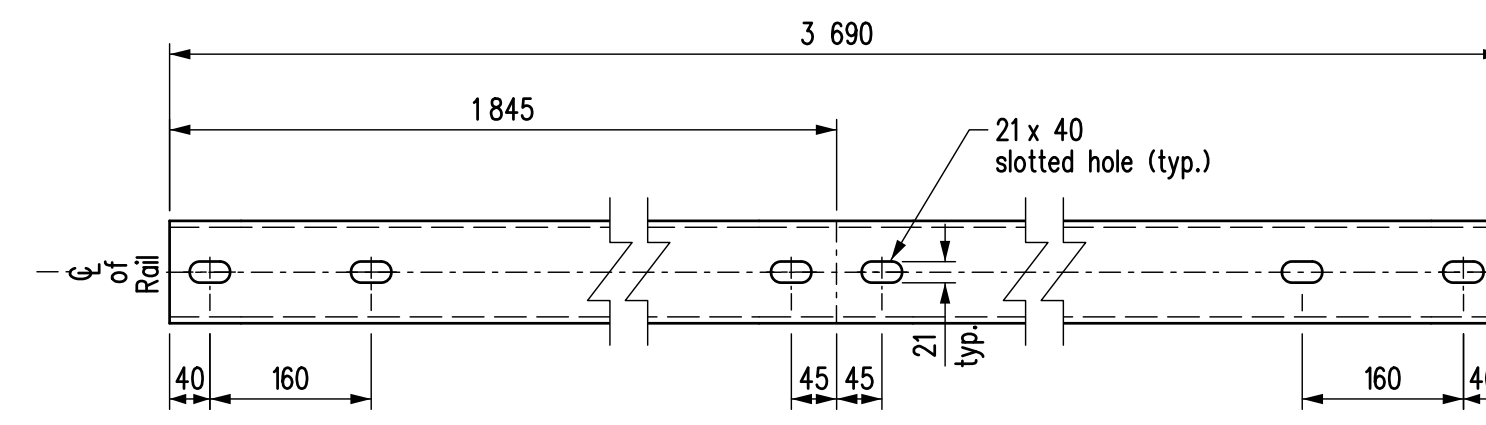
SCALE: 1:10

SHEET No. _____

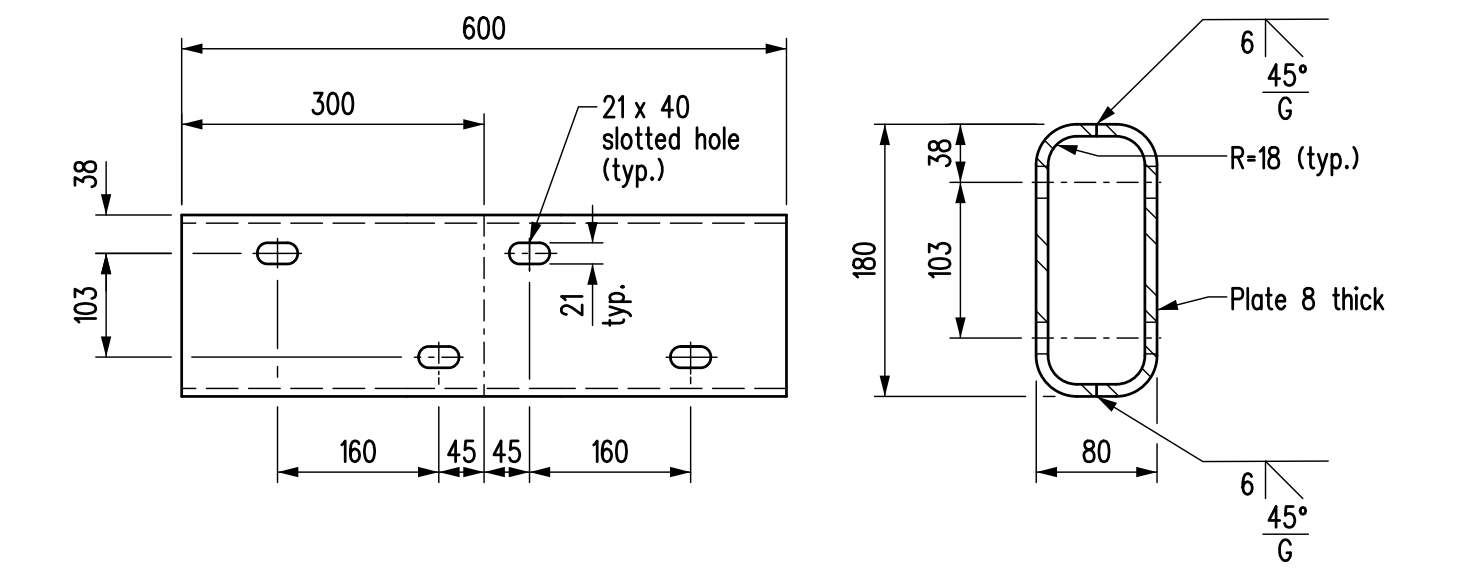
or as shown SITE No. _____



Mk. "T2"

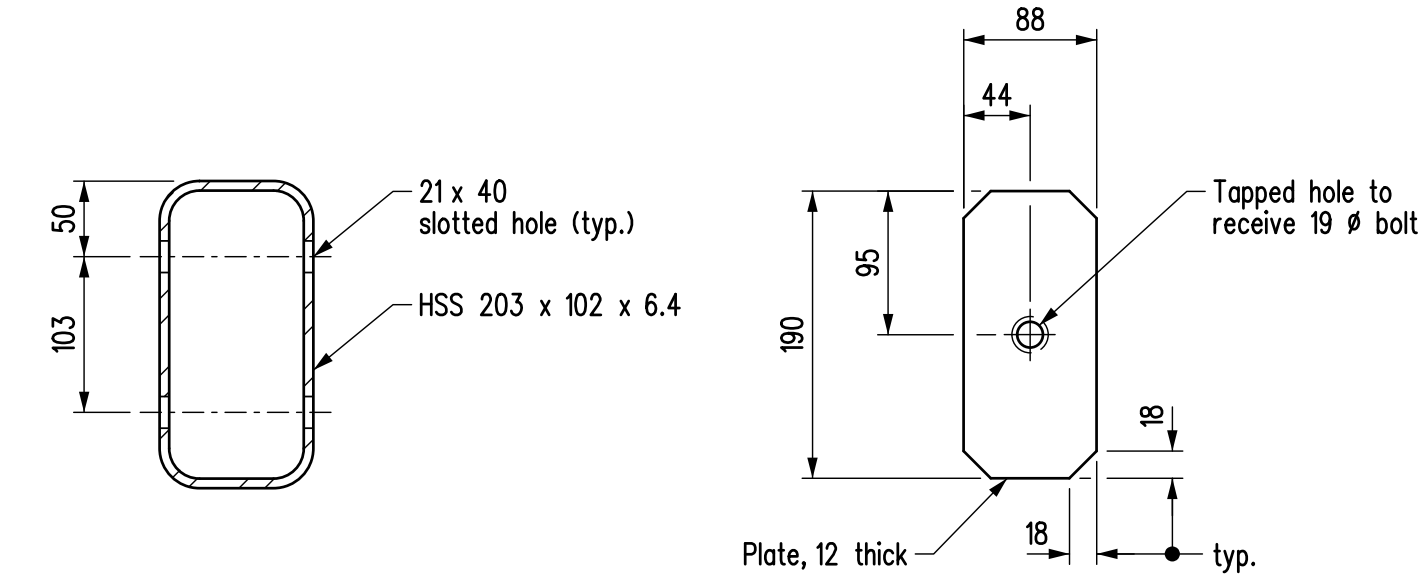


Mk. "B3"



Mk. "ST2"

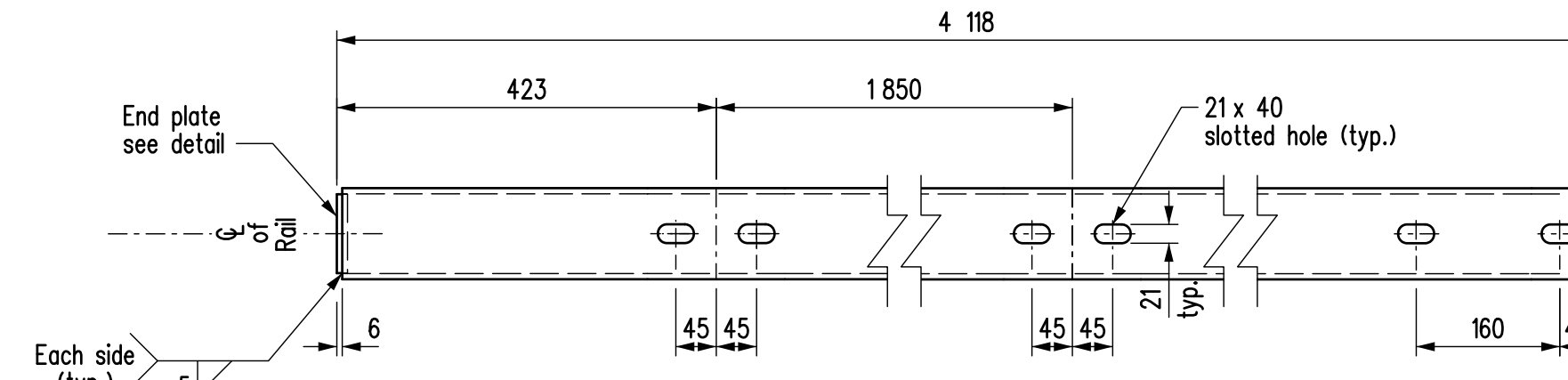
TYPICAL CROSS SECTION
Scale 1:5



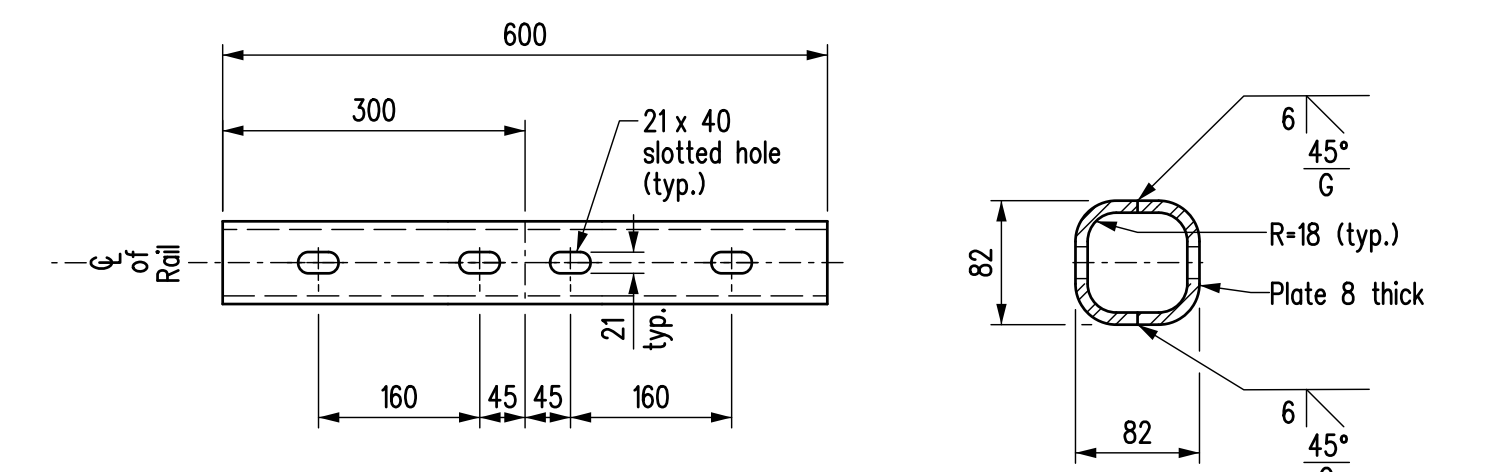
TYPICAL CROSS SECTION
Typical for rail Mk. "T2"
Scale 1:5

END PLATE
For rail Mk. "T2"
Scale 1:5

DETAILS OF TOP RAILS

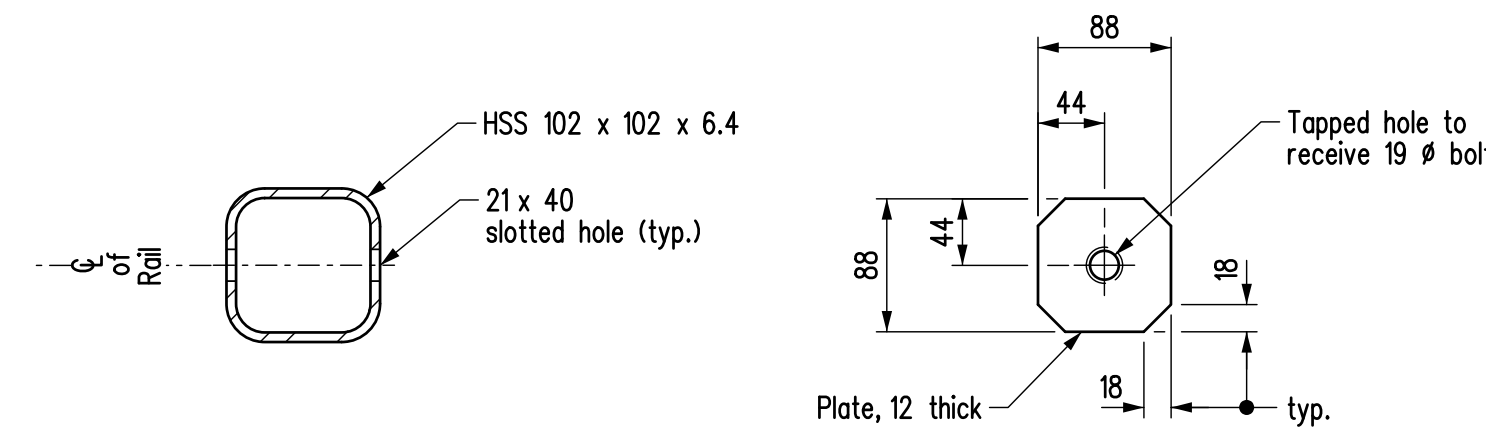


Mk. "B4"



Mk. "SB2"

TYPICAL CROSS SECTION
Scale 1:5



TYPICAL CROSS SECTION
Typical for rails Mk. "B3" & "B4"
Scale 1:5

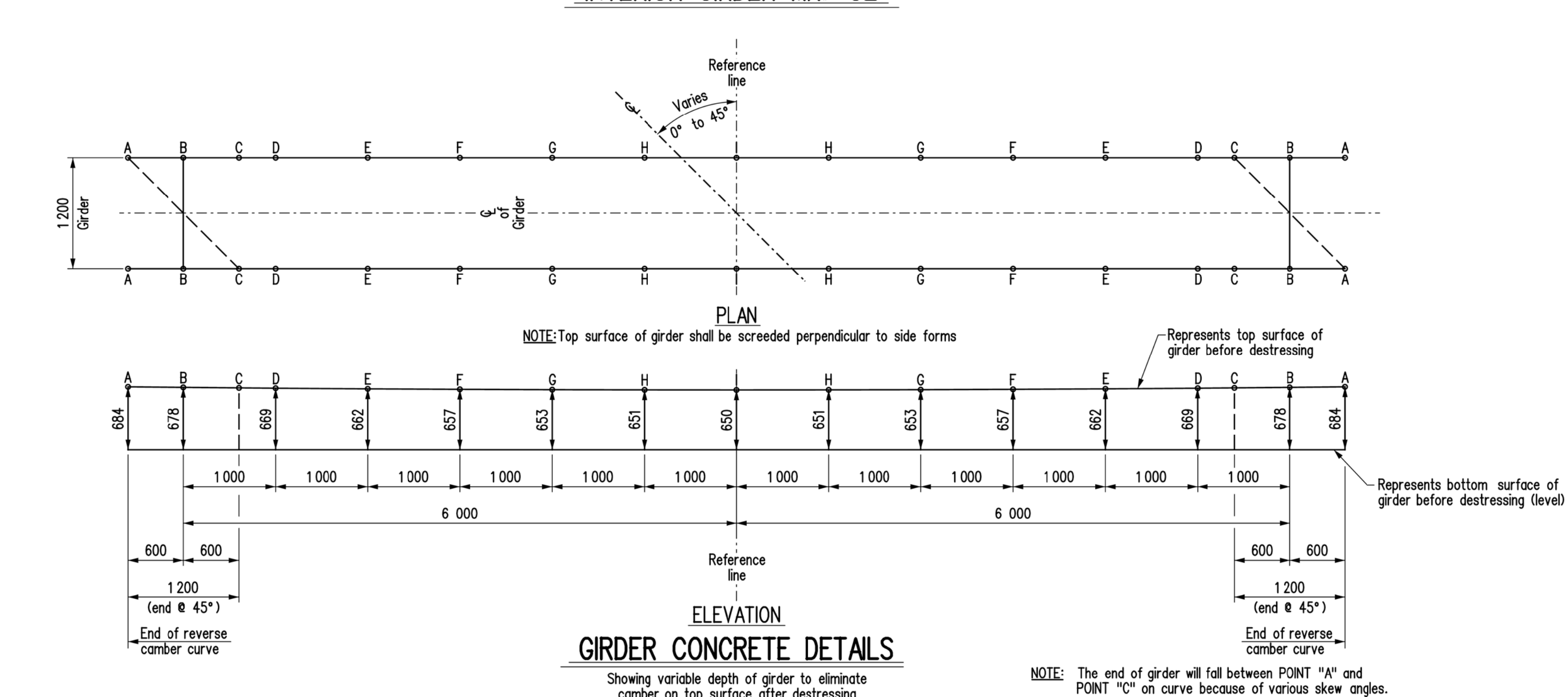
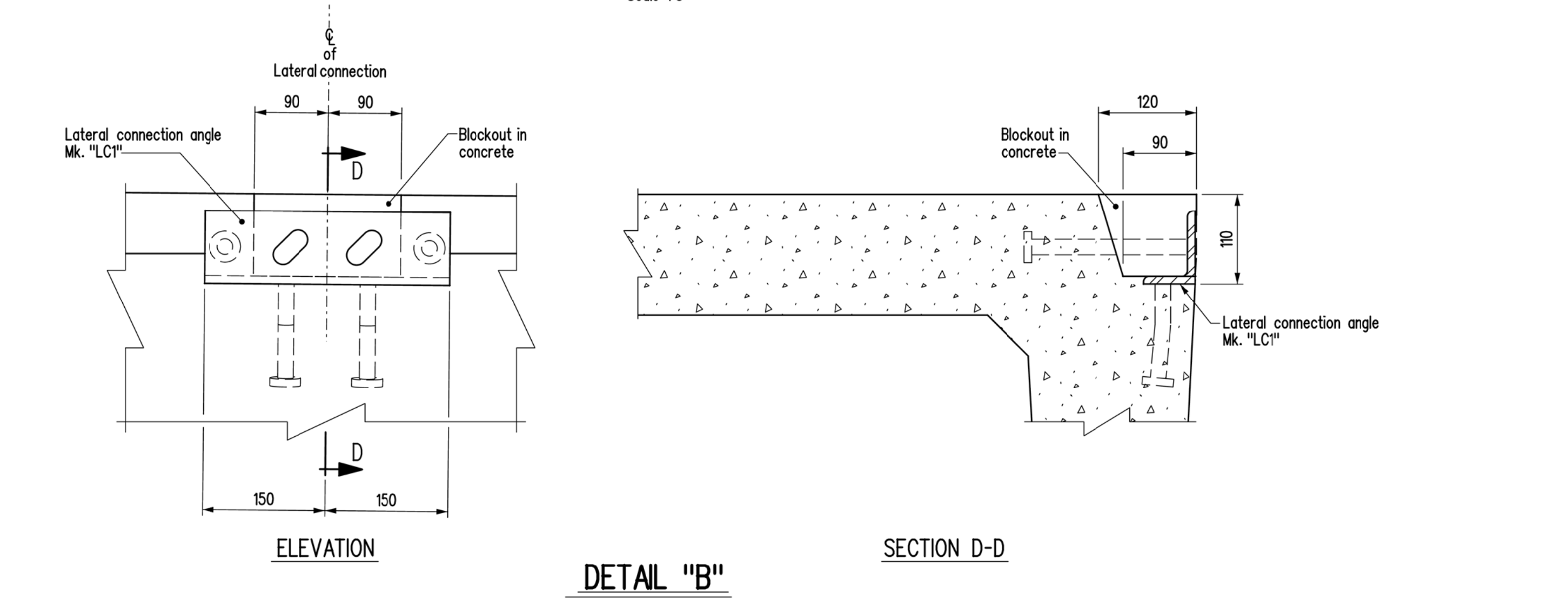
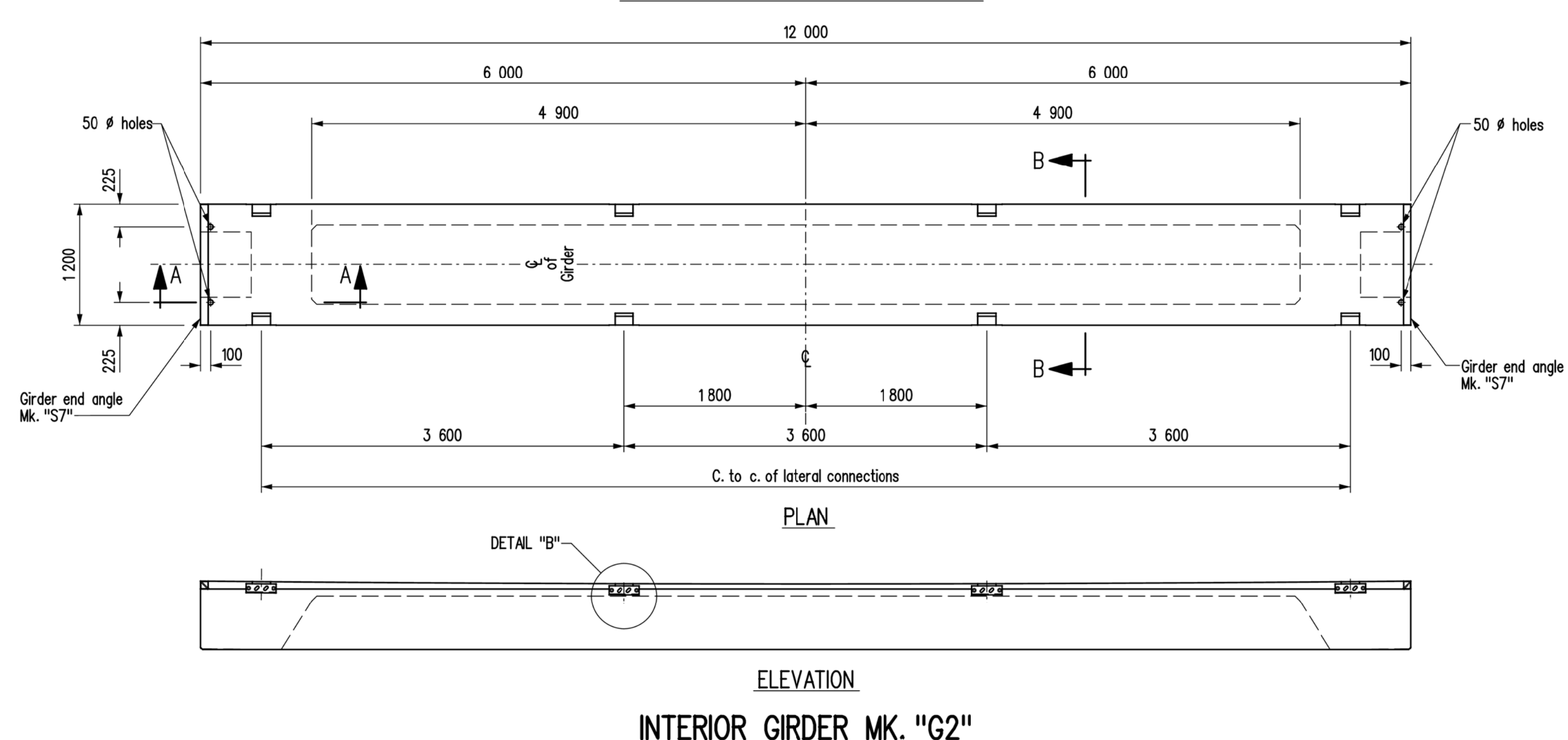
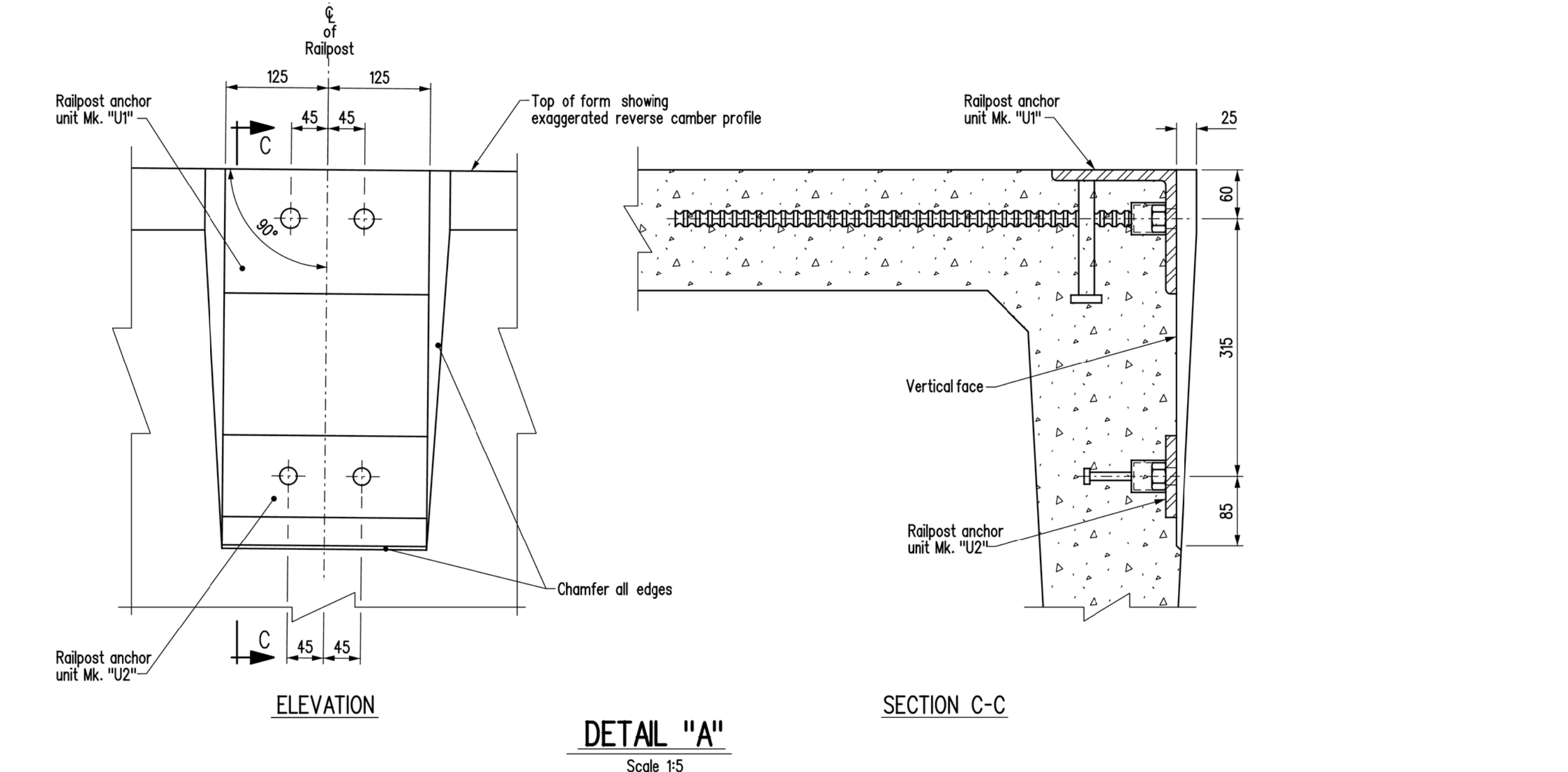
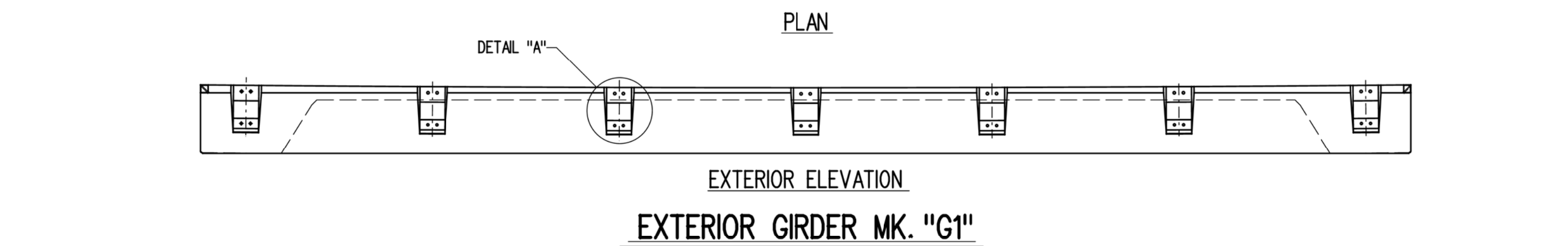
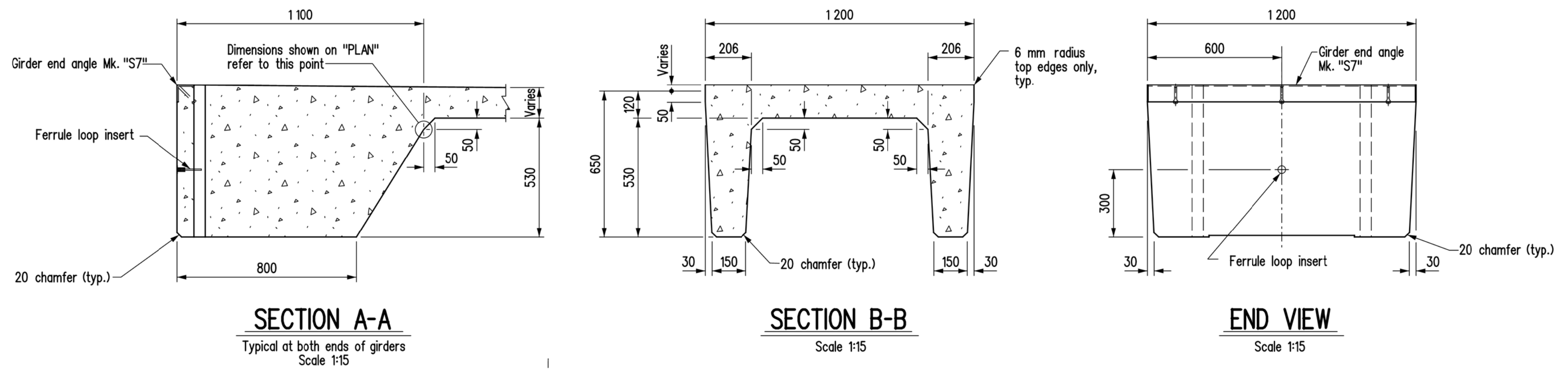
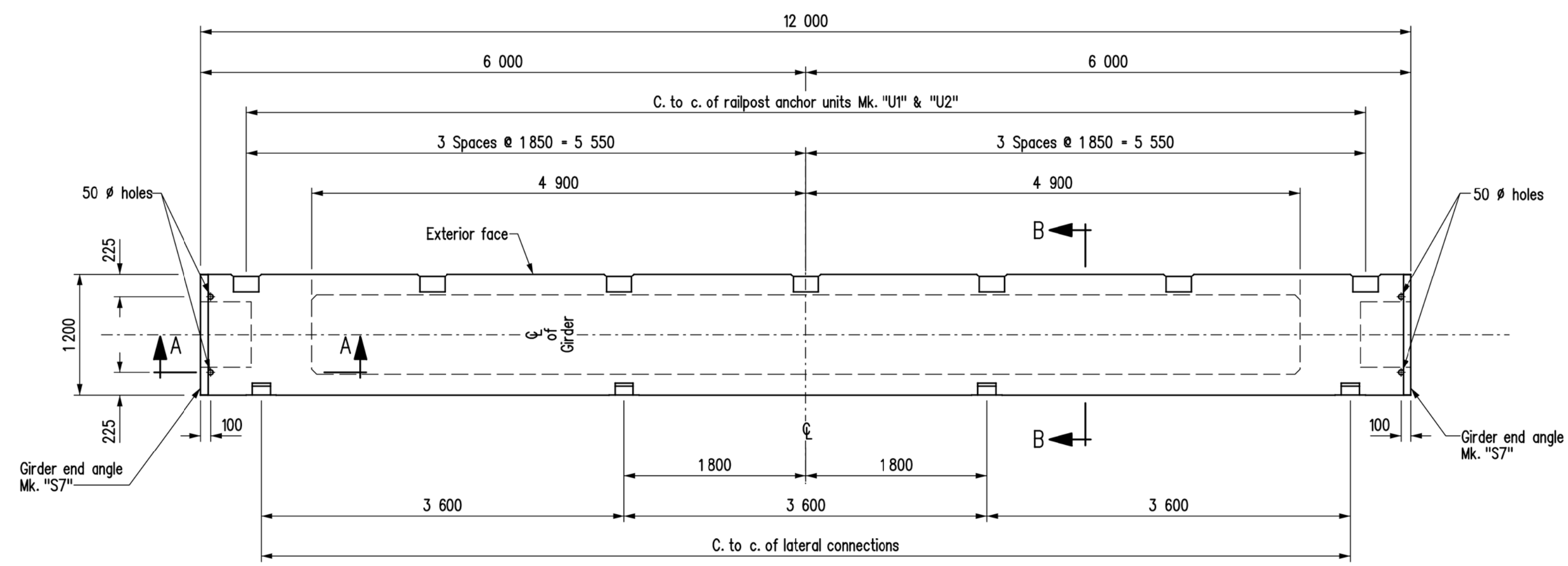
END PLATE
For rail Mk. "B4"
Scale 1:5

DETAILS OF BOTTOM RAILS

NOTES:

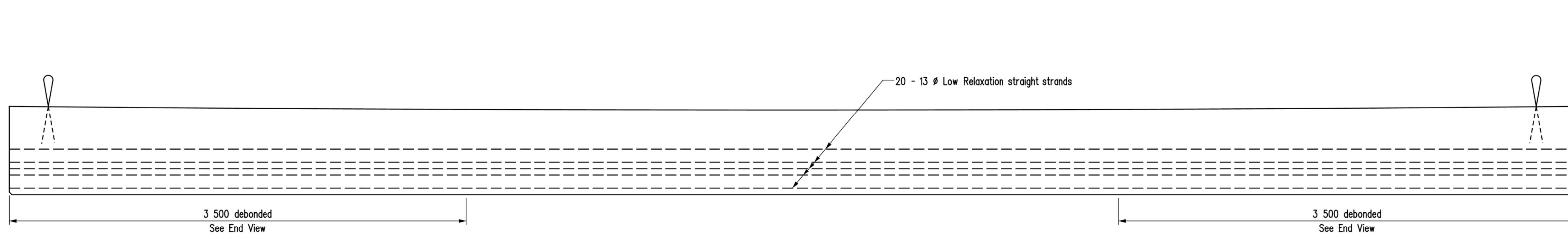
1. It is imperative that all rail and sleeve holes in each pair of holes be opposite to each other.
2. The length of slotted holes shall not be less than shown.
3. The width and height of the sleeves shall not exceed the dimensions shown.
4. All dimensions are in millimeters (mm).

REVISIONS		RAILING DETAILS	
DATE	BY		
		<p>Infrastructure Water Management and Structures</p>	RELEASED FOR CONSTRUCTION BY: _____
			EXECUTIVE DIRECTOR OF STRUCTURES DATE _____
			SCALE: 1:7.5 SHEET No. _____
			or as shown SITE No. _____
DATE	BY	DESCRIPTION	
		DESIGN	
		CHECKED: _____	
		DETAILS	
		BY: _____	
		CHECKED: _____	

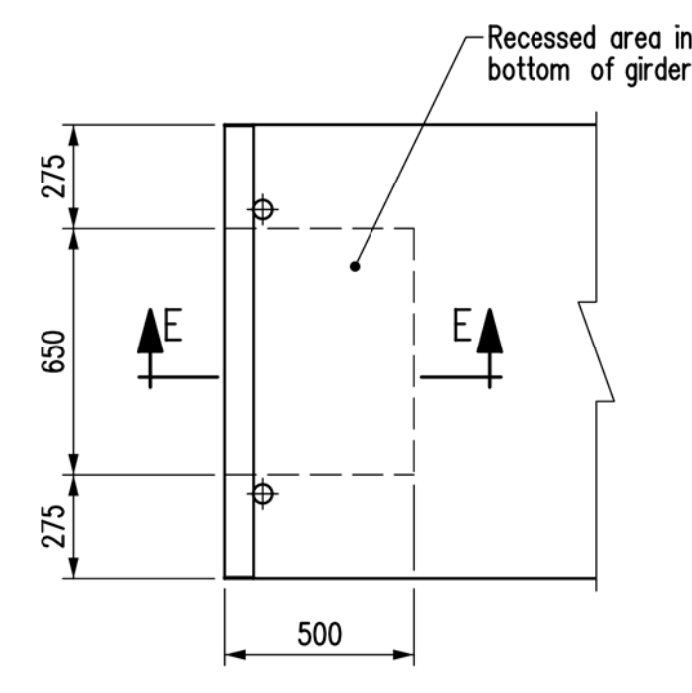
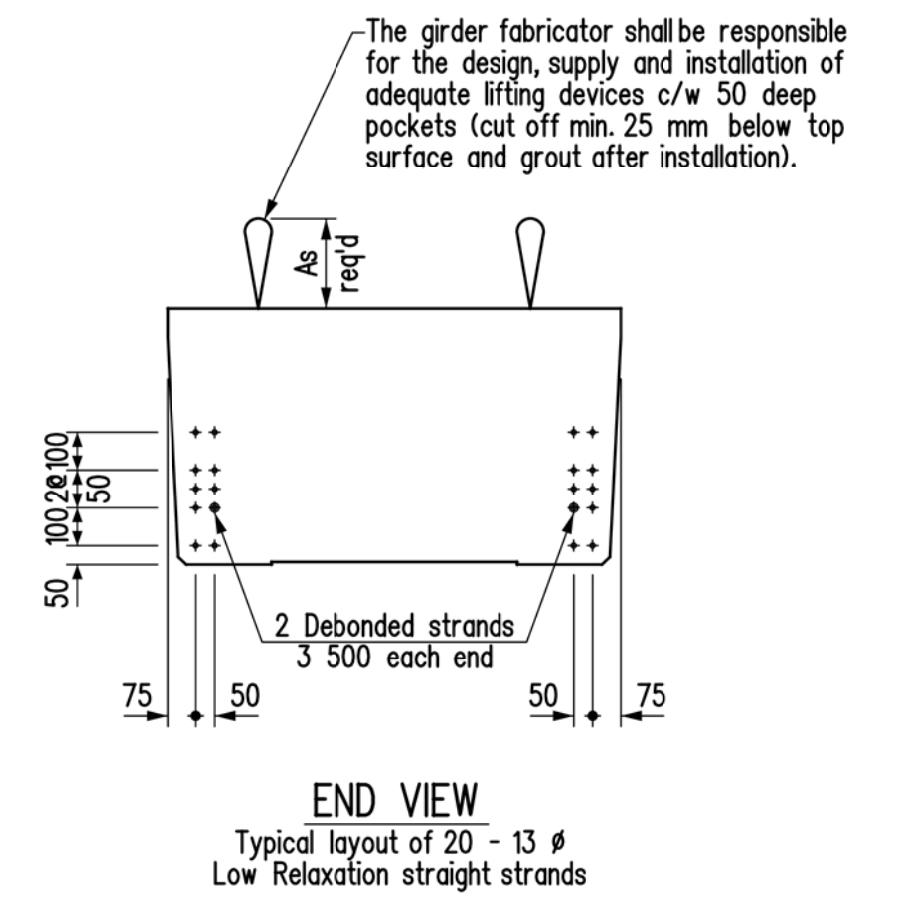


- NOTES:
- Design in accordance with AASHTO LRFD Bridge Design Specifications, First Edition, 1994 plus 1996/1997 interim's.
 - Design Vehicular Live Load: Modified AASHTO HSS-25 AASHTO LRFD "HL-93"
 - Design distribution factor = 0.5 lanes/girder.
 - Concrete strength: f_{ci} transfer, $f_{ci} = 35 \text{ MPa}$
 f_c 28 days, $f_c = 45 \text{ MPa}$
 - Prestressing steel: 13 mm ϕ low relaxation strands
Minimum ultimate strength, $f_{pu} = 1860 \text{ MPa}$
Jacking force/strand, $f_{pj} = 128.5 \text{ kN/strand}$
 - Girder dimensioning tolerances: Length 3 mm \pm
Cross section 2 mm \pm
 - Approximate mass per girder = 12 000 kg

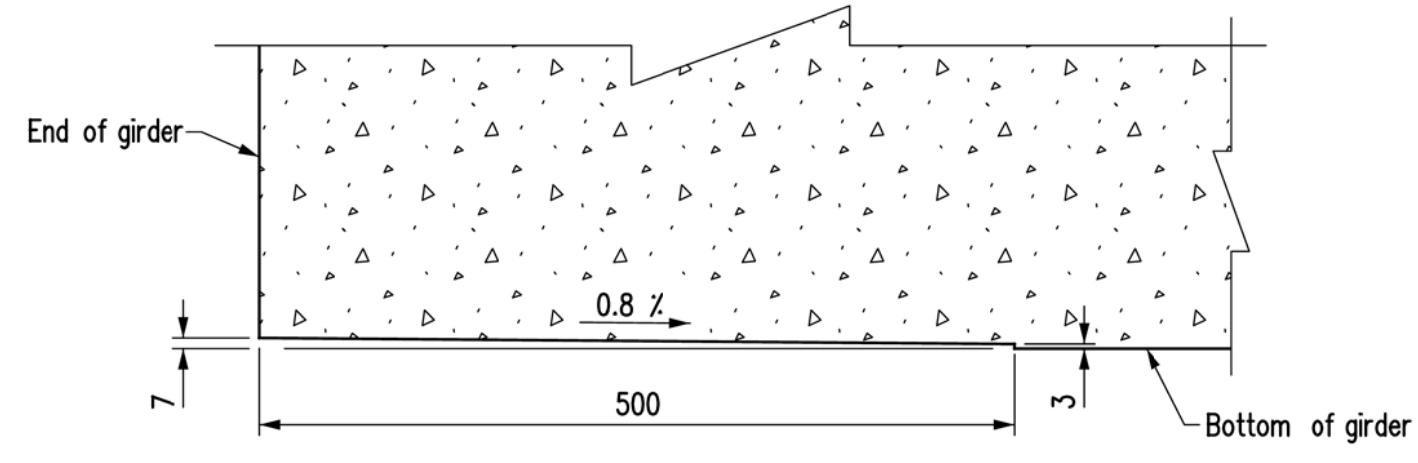
REVISIONS		PRECAST PRESTRESSED CHANNEL GIRDER DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:
PLACE ENGINEERS ELECTRONIC SEAL HERE			EXECUTIVE DIRECTOR OF STRUCTURES DATE
		DESIGN BY: B.A.N.	SCALE:
		CHECKED:	Scale 1:40 SHEET No. G1
		DETAILS BY: K.P.	or as shown SITE No.



ELEVATION
GIRDER STRAND LAYOUT




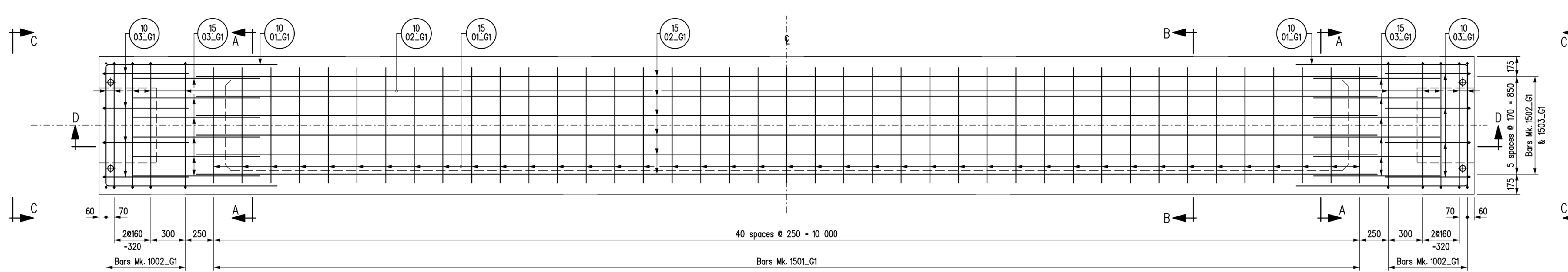
PART PLAN
Typical at both ends of girders



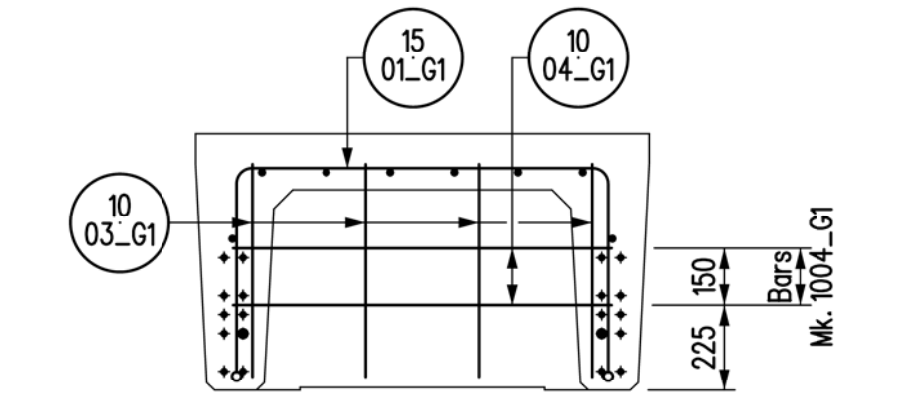
SECTION E-E
Scale 1:5

BEARING RECESS DETAILS

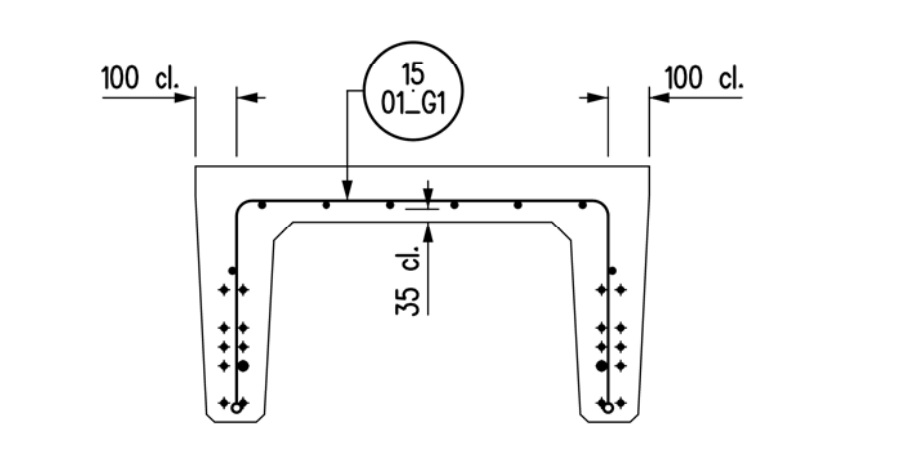
REVISIONS		<p>PRECAST PRESTRESSED CHANNEL GIRDER DETAILS</p> 		RELEASED FOR CONSTRUCTION	
				BY: _____	
				EXECUTIVE DIRECTOR OF STRUCTURES DATE	
				SCALE:	SHEET No. <u>G2</u>
				BY: _____	or as shown SITE No. <u> </u>
				CHECKED: _____	
DATE	BY	DESCRIPTION			
DESIGN SEAL	RECORD SEAL				
<p>PLACE ENGINEERS ELECTRONIC SEAL HERE</p>		DESIGN	BY: _____ B.A.N.		
		DETAILS	BY: _____ K.P.		



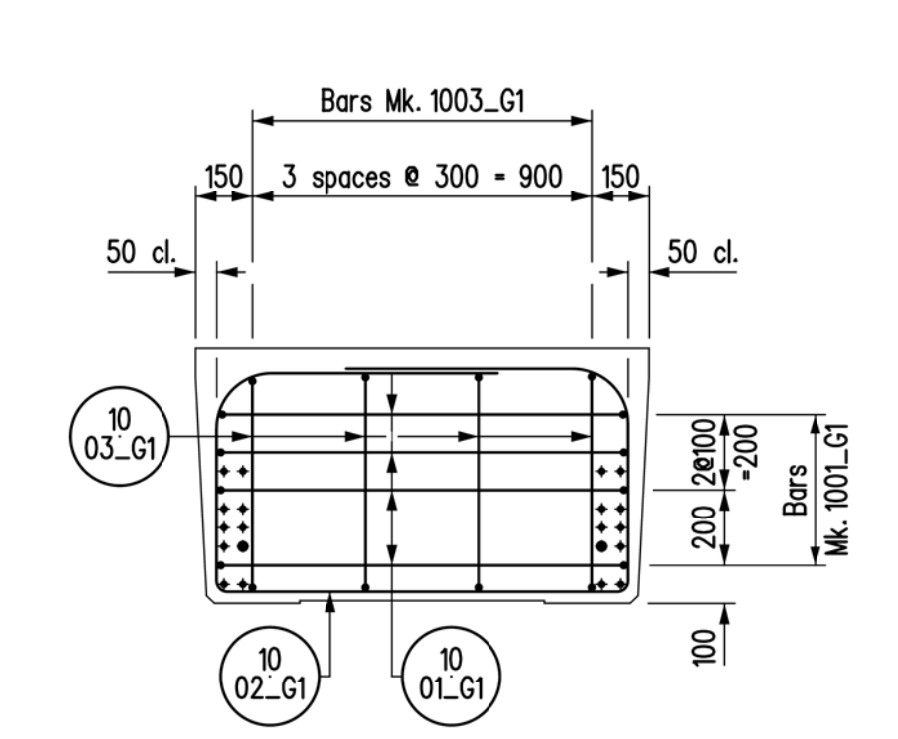
PLAN OF GIRDER



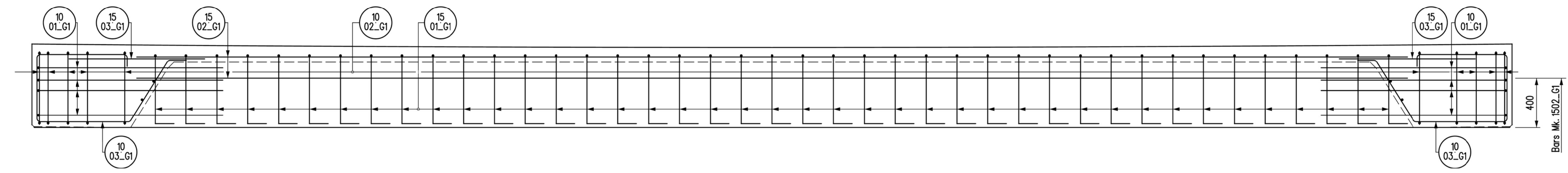
SECTION A-A



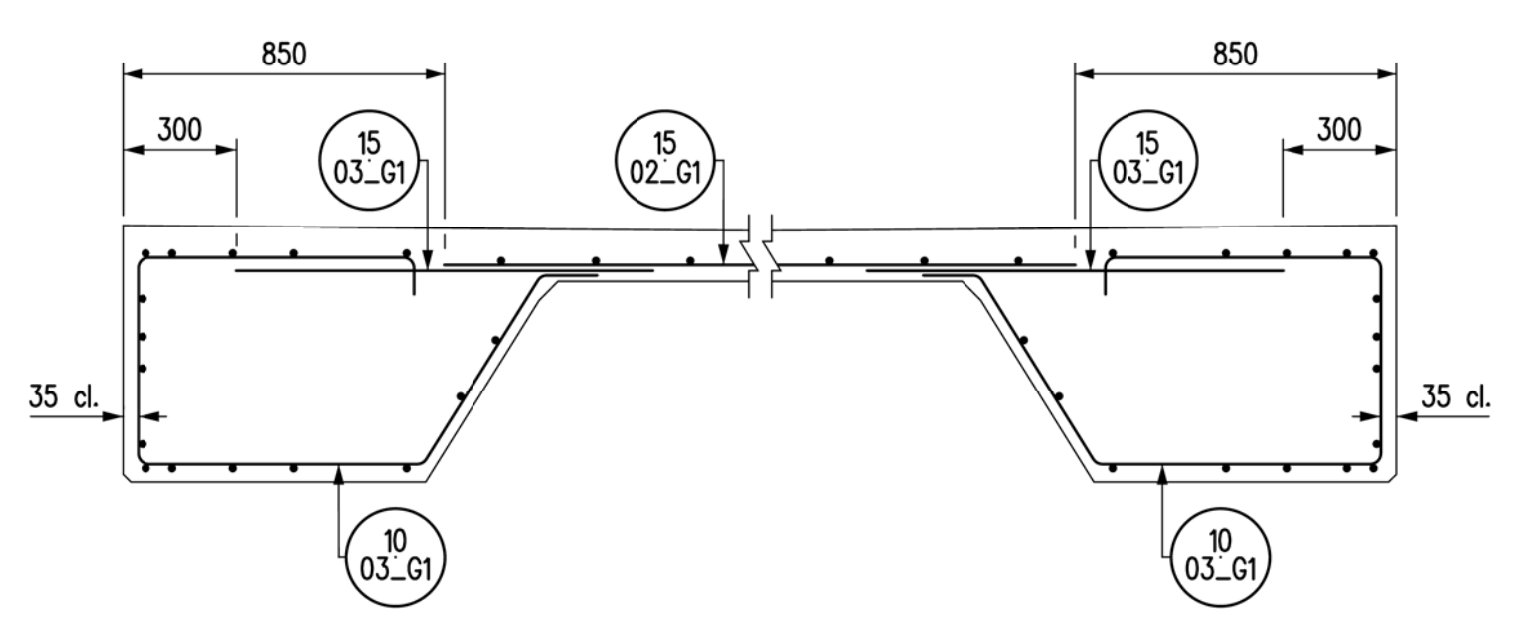
SECTION B-B



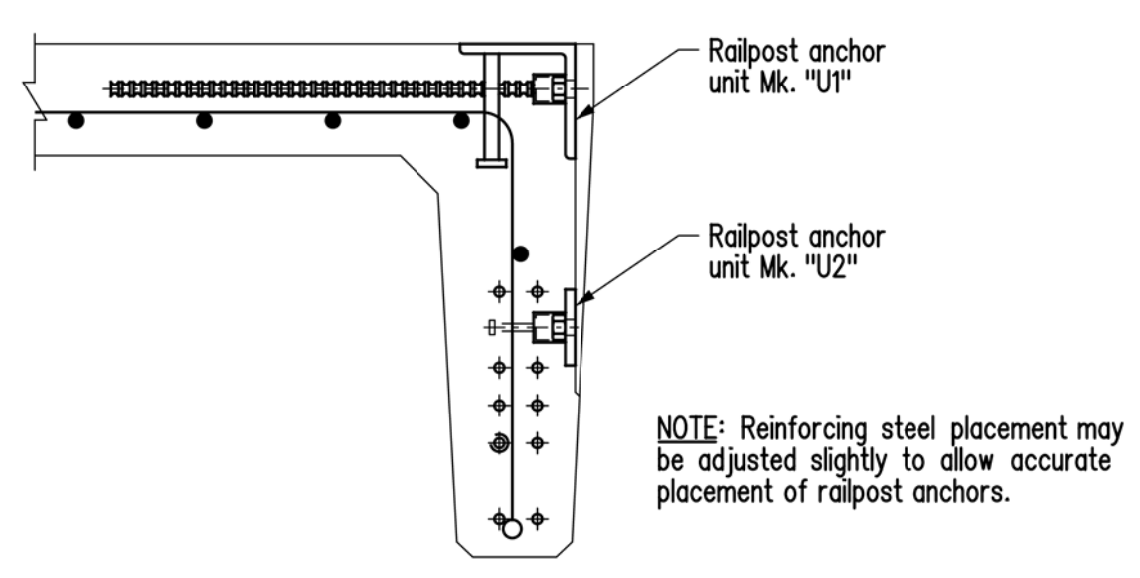
END VIEW C-C



ELEVATION OF GIRDER



PART SECTION D-D



DETAIL AT RAILPOST ANCHOR

Scale 1:10

- NOTES:**
- Concrete cover shall be 25 mm unless noted otherwise.
 - Reinforcing details are typical for all 12 m girders unless noted otherwise.
 - Bar Mark labels with suffix _G1 are Exterior girders and suffix _G2 are Interior girders. See Bill of Reinforcing Sheet No. G

REVISIONS		PRECAST PRESTRESSED CHANNEL GIRDER DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:
			EXECUTIVE DIRECTOR OF STRUCTURES DATE
			SCALE: Scale 1:20 SHEET No. G4
			or as shown SITE No. 1111

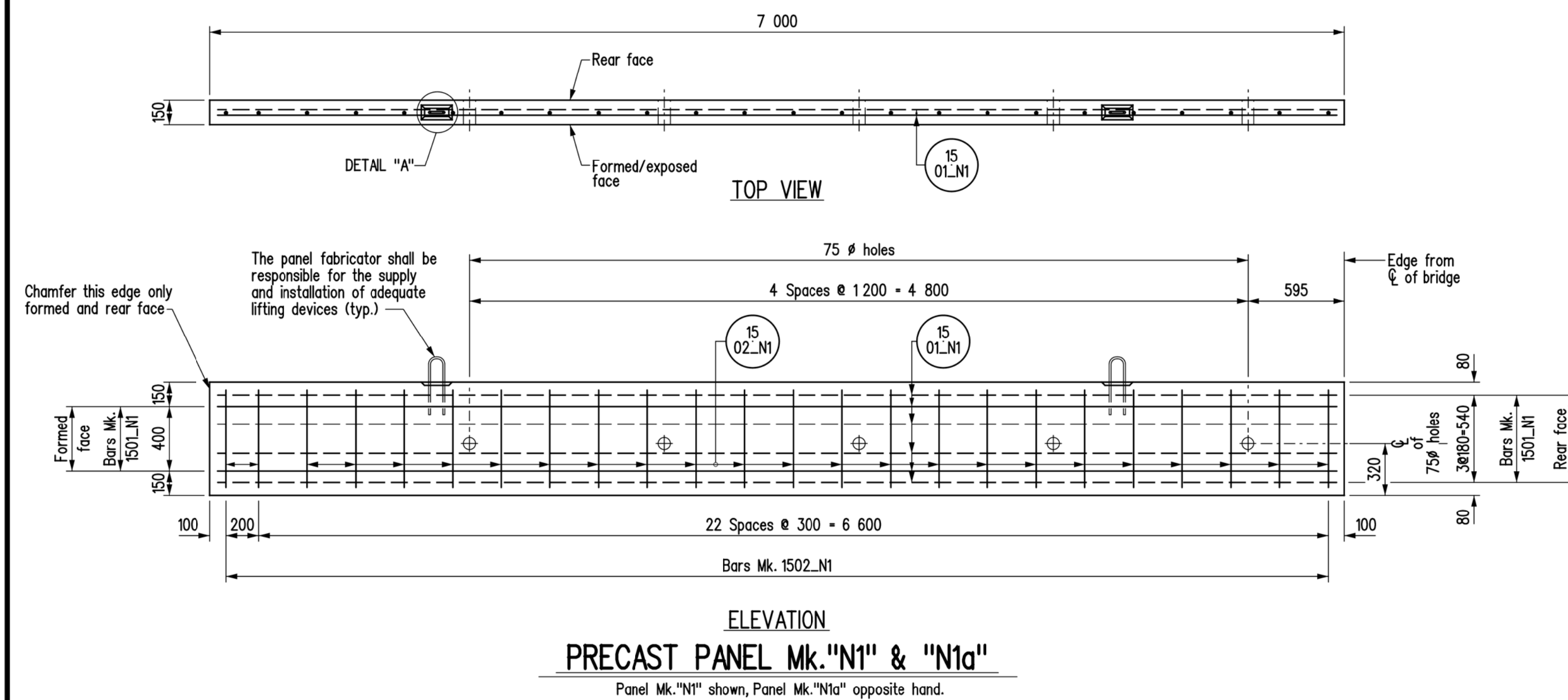
PLACE ENGINEERS ELECTRONIC SEAL HERE	DESIGN	BY: B.A.N.
	CHECKED:	
	DETAILS	BY: K.P.
	CHECKED:	



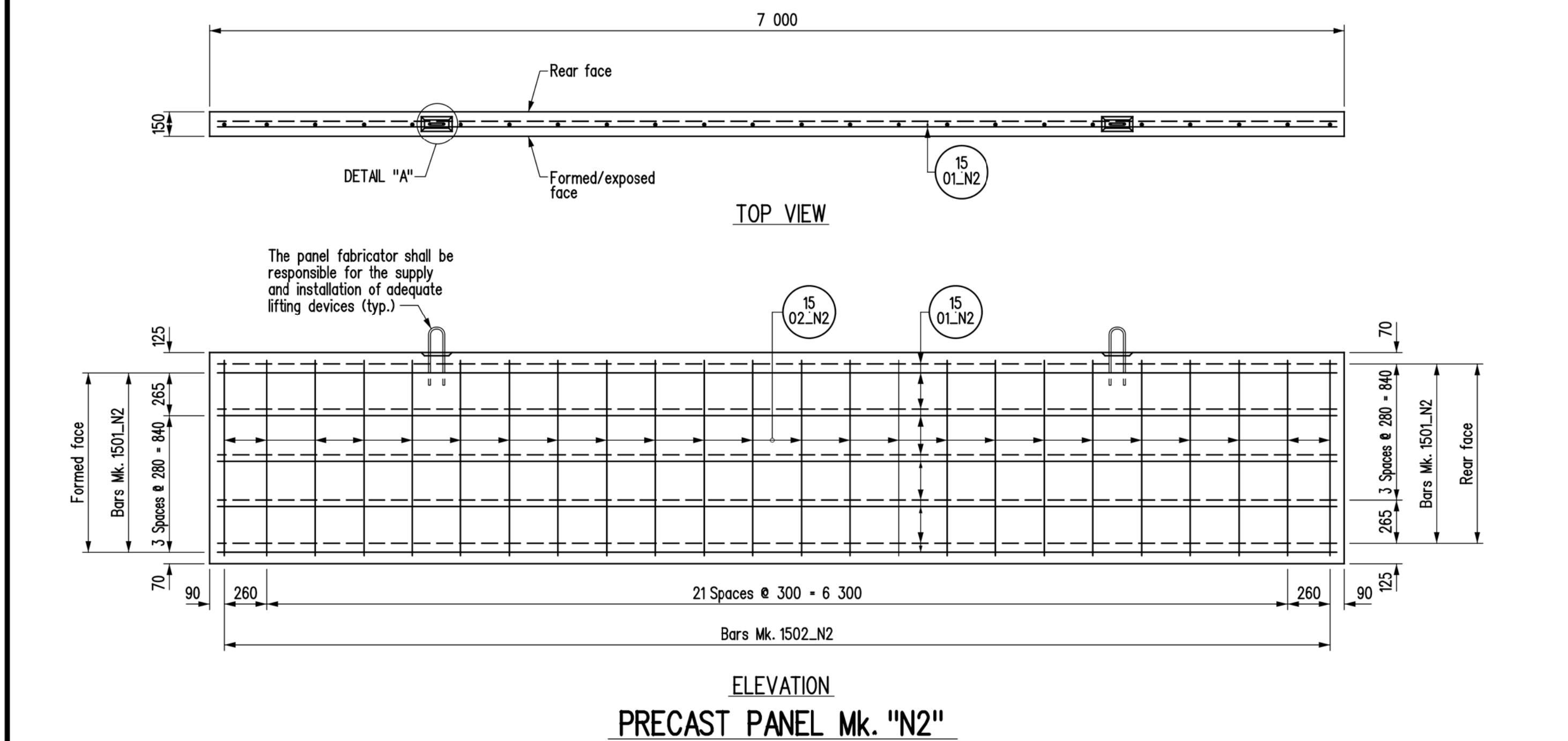
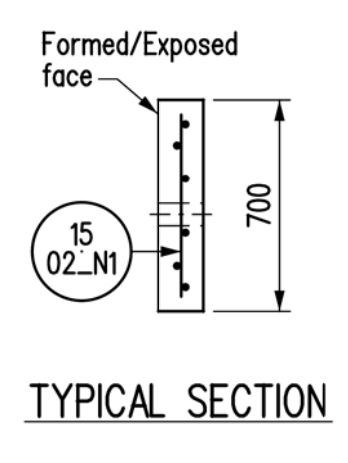
BILL OF REINFORCING STEEL - 12 M GIRDERS								SITE No.
MARK	TYPE	PIN DIAMETER	LENGTH	GIRDER TYPE	No. of GIRDERS	No. of BARS PER GIRDER	TOTAL No. of BARS PER GIRDER TYPE	BENDING DIAGRAM
1001_G1	BENT	45	4 080	G1	2	8	16	
1002_G1	BENT	45	3 660	G1	2	10	20	
1003_G1	BENT	45	2 950	G1	2	8	16	
1004_G1	STR		1 000	G1	2	4	16	
1501_G1	BENT	65	2 440	G1	2	41	82	
1502_G1	STR		10 300	G1	2	8	16	
1503_G1	STR		1 100	G1	2	12	24	
1001_G2	BENT	45	4 080	G2	8	8	64	
1002_G2	BENT	45	3 660	G2	8	10	80	
1003_G2	BENT	45	2 950	G2	8	8	64	
1004_G2	STR		1 000	G2	8	4	64	

BILL OF REINFORCING STEEL - 12 M GIRDERS								SITE No.
MARK	TYPE	PIN DIAMETER	LENGTH	GIRDER TYPE	No. of GIRDERS	No. of BARS PER GIRDER	TOTAL No. of BARS PER GIRDER TYPE	BENDING DIAGRAM
1501_G2	BENT	65	2 440	G2	8	41	328	
1502_G2	STR		10 300	G2	8	8	64	
1503_G2	STR		1 100	G2	8	12	96	
Total volume of structural concrete per exterior girder								4.94 m³
Total volume of structural concrete per interior girder								4.93 m³
NOTES:								
1. All dimensions given in bending diagram are out to out, except radii and extensions on 90°, 135° & 180° hooks. Extensions on 90°, 135° & 180° hooks are the "A" or "G" dimensions for standard 90°, 135° & 180° hooks referenced from the RSIC "Manual of Standard Practice". Radii are inside dimensions. All reinforcing steel bends and hooks shall conform to Clause 6.6.2 of C.S.A. A23.1-04, unless noted otherwise in the BILL OF REINFORCING STEEL.								
2. All reinforcing steel shall be deformed steel, unless noted otherwise in the BILL OF REINFORCING STEEL.								
3. All reinforcing steel shall conform to CSA G30.18-M92 "Billet Steel Bars for Concrete Reinforcement" Grade 400W, unless noted otherwise in the BILL OF REINFORCING STEEL.								
4. Like bars shall be bundled, securely tied and identified as to Mark and Site No. by appropriate means. All other items to be identified in a similar fashion.								
5. All bars shall be bent in accordance with the following detail:								

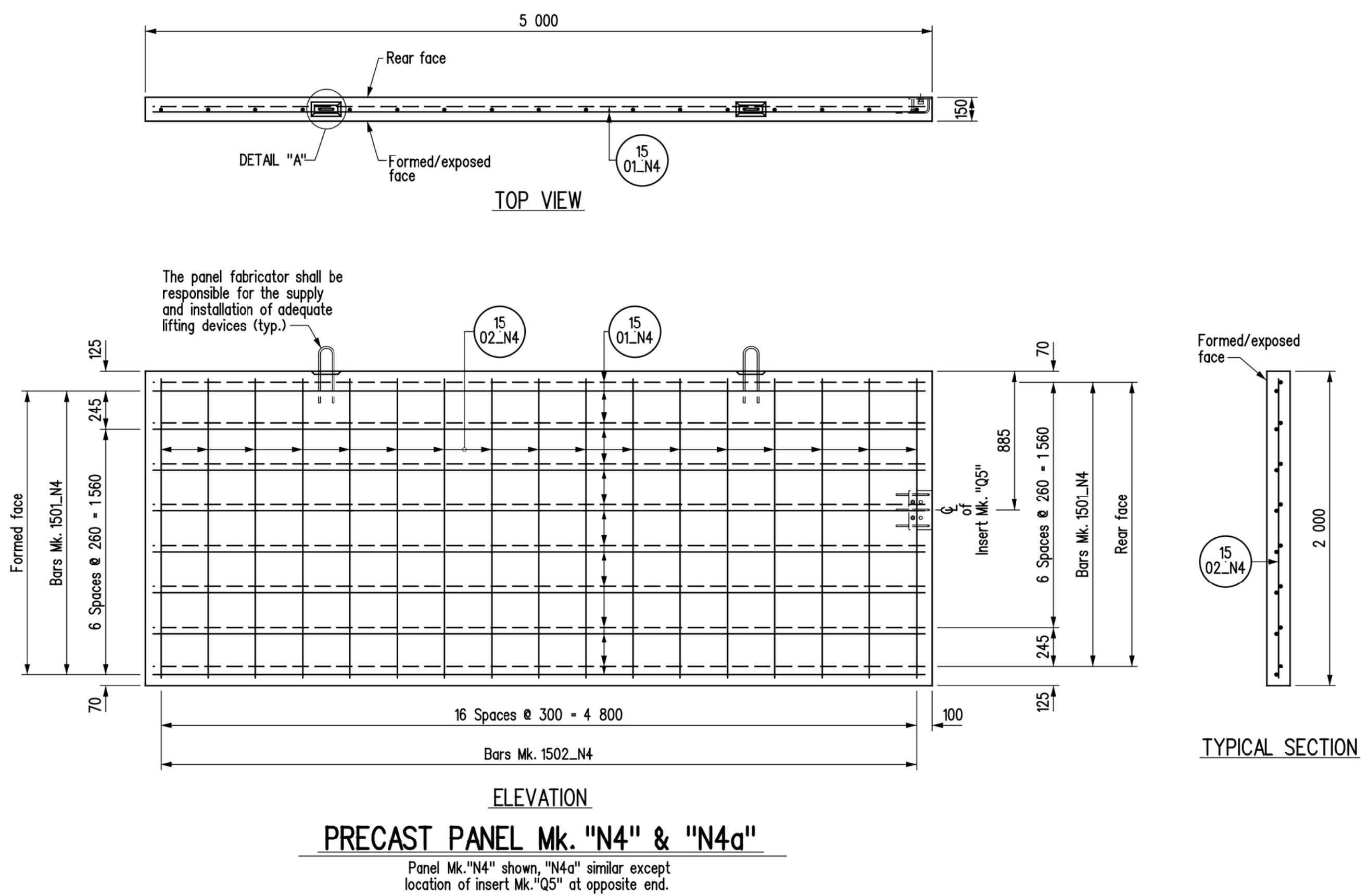
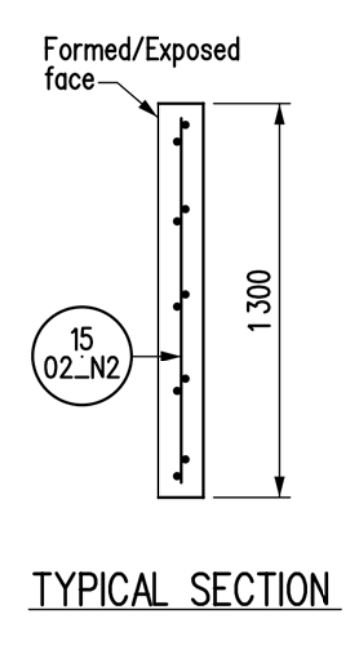
REVISIONS		PRECAST PRESTRESSED CHANNEL GIRDER DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:
DESIGN SEAL	RECORD SEAL		
PLACE ENGINEERS ELECTRONIC SEAL HERE		 Infrastructure Water Management and Structures	EXECUTIVE DIRECTOR OF STRUCTURES DATE _____
		DESIGN BY: B.A.N.	SCALE: _____ SHEET No. 65
		CHECKED: _____	_____
		DETAILS BY: K.P.	_____ SITE No. _____



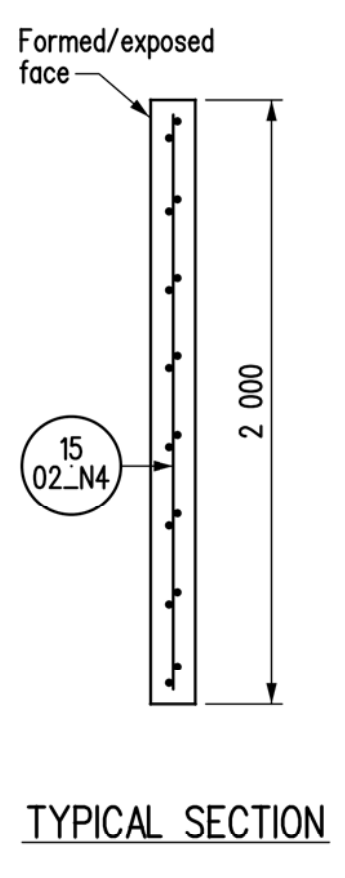
PRECAST PANEL Mk. "N1" & "N1a"
Panel Mk. "N1" shown, Panel Mk. "N1a" opposite hand.



PRECAST PANEL Mk. "N2"



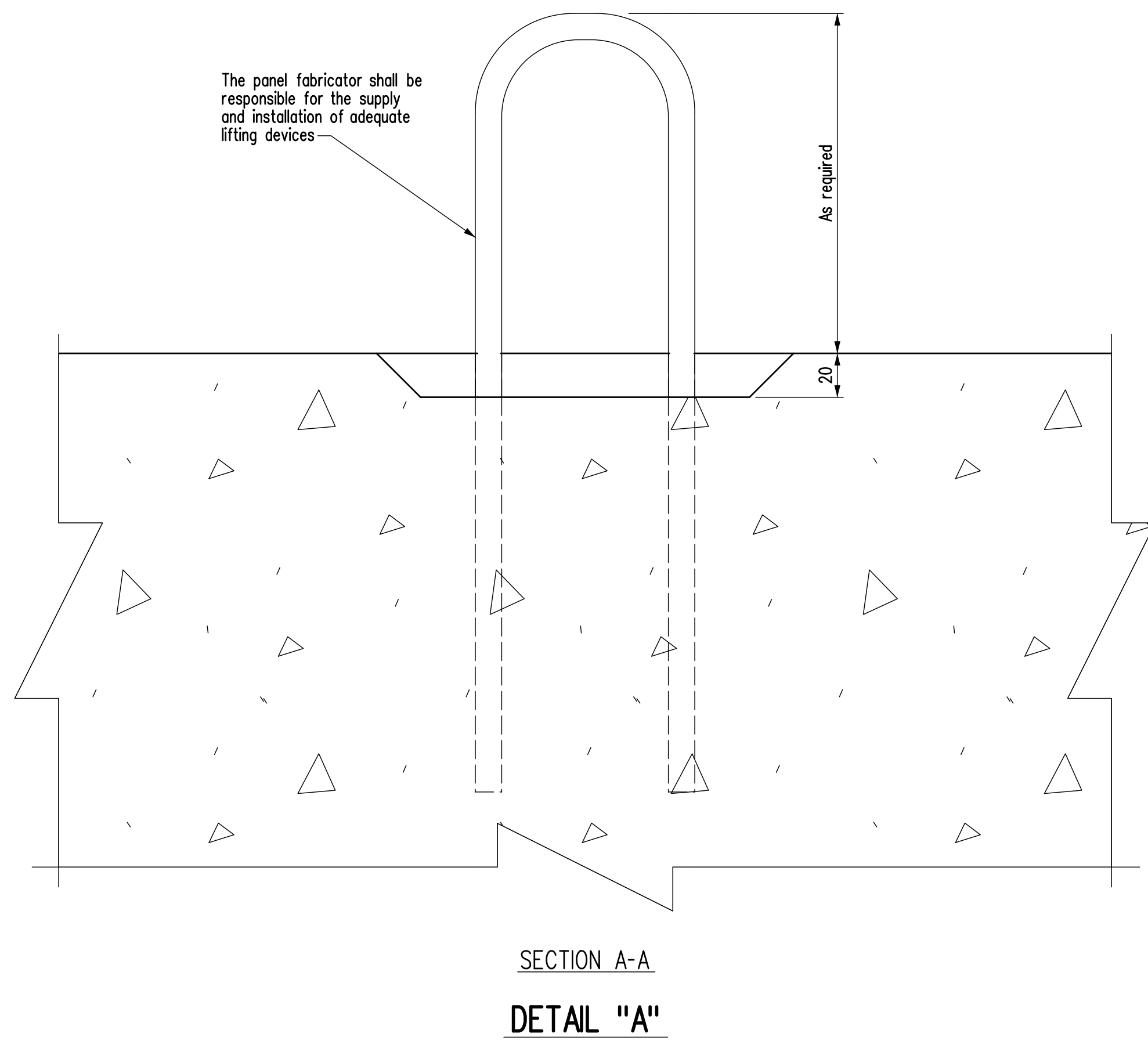
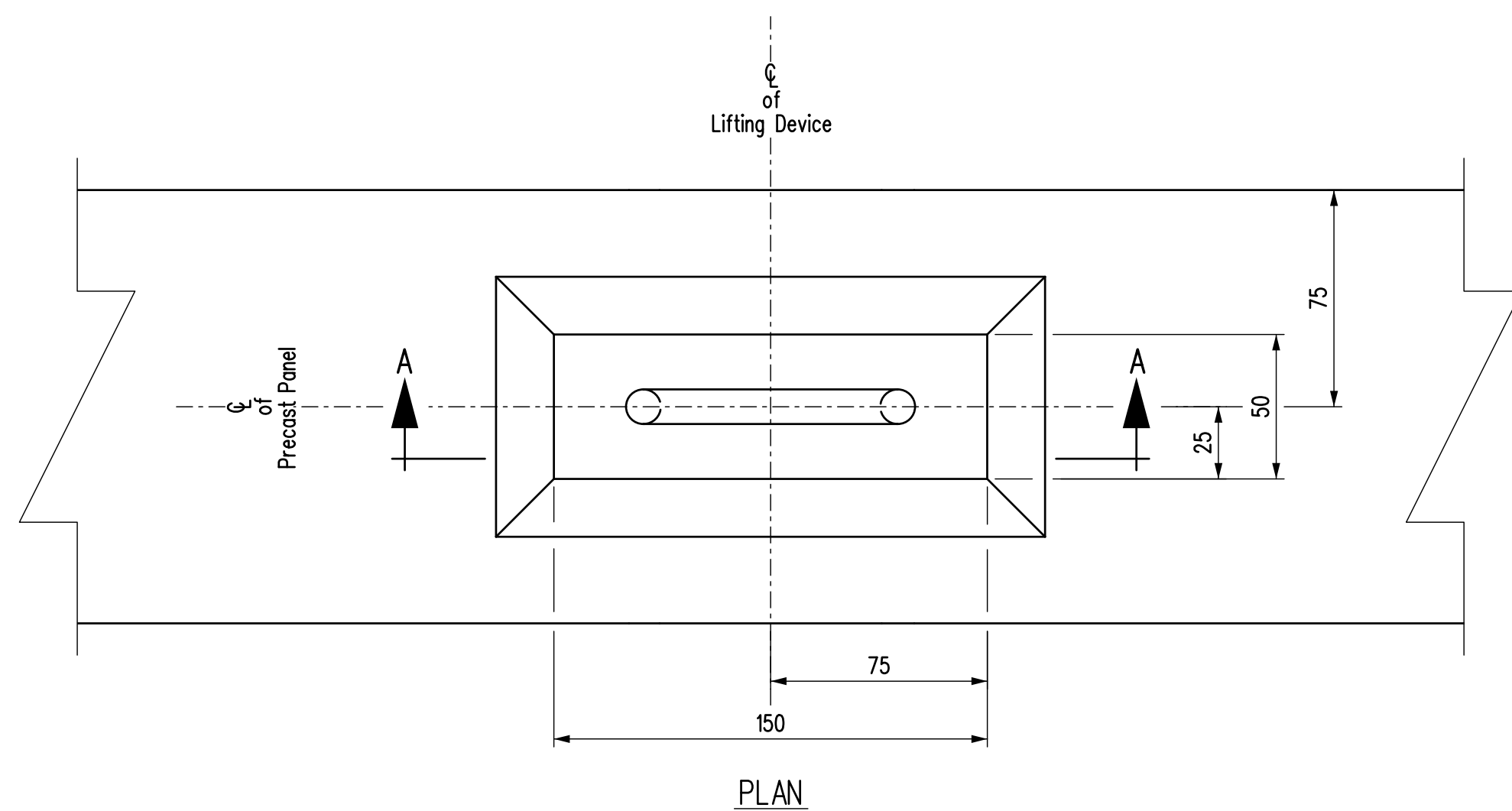
PRECAST PANEL Mk. "N4" & "N4a"
Panel Mk. "N4" shown, "N4a" similar except location of insert Mk. "Q5" at opposite end.



NOTES:

1. All panel exposed edges to be chamfered 20 mm except no chamfer on panels Mk. "N1" & "N1a", or if shown.
2. Mark reinforcing steel location on the edges of the back face of panels after casting.
3. Concrete cover shall be 50 mm unless noted otherwise.
4. Formed face to be placed as exposed face during construction.
5. After precast panel installation, all lifting devices to be cut-off flush and grouted as directed by Engineer.
6. For DETAIL "A" see sheet No. P2.
7. For BILL OF REINFORCING STEEL see Sheet No. P2.

REVISIONS		PRECAST PANEL DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:
20-- --		ISSUED FOR CONSTRUCTION	
		DESIGN SEAL	EXECUTIVE DIRECTOR OF STRUCTURES DATE
		RECORD SEAL	
PLACE ENGINEERS ELECTRONIC SEAL HERE		<p>Manitoba Infrastructure Water Management and Structures</p>	EXECUTIVE DIRECTOR OF STRUCTURES DATE
			SCALE: 1:25 SHEET No. P1
			or as shown SITE No. []



BILL OF REINFORCING FOR PRECAST PANELS

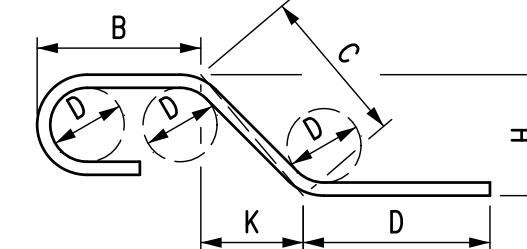
SITE No. _____

MARK	TYPE	PIN DIAMETER	LENGTH	PANEL TYPE	No. of PANELS	No. of BARS PER PANEL	TOTAL No. of BARS PER PANEL TYPE	BENDING DIAGRAM
1501_N1	STR		6 900	N1	2	6	12	
1502_N1	STR		600	N1	2	24	48	
1501_N1a	STR		6 900	N1a	2	6	12	
1502_N1a	STR		600	N1a	2	24	48	
1501_N2	STR		6 900	N2	4	10	40	
1502_N2	STR		1 200	N2	4	24	96	
1501_N4	STR		4 900	N4	2	16	32	
1502_N4	STR		1 900	N4	2	17	34	
1501_N4a	STR		4 900	N4a	2	16	32	
1502_N4a	STR		1 900	N4a	2	17	34	

Total mass of reinforcing steel							1659.80	kg
Panel Type	N1	N1a	N2	N3	N4	N4a		
Area m ² /panel	4.90	4.90	9.10	-	10.00	10.00		
Total area of precast Panels							96.00	m ²

NOTES:

- All dimensions given in bending diagram are out to out, except radii and extensions on 90°, 135° & 180° hooks. Extensions on 90°, 135° & 180° hooks are the "A" or "G" dimensions for standard 90°, 135° & 180° hooks referenced from the RSIC "Manual of Standard Practice". Radii are inside dimensions. All reinforcing steel bends and hooks shall conform to Clause 6.6.2 of C.S.A. A23.1-04, unless noted otherwise in the BILL OF REINFORCING STEEL.
- All reinforcing steel shall be deformed steel, unless noted otherwise in the BILL OF REINFORCING STEEL.
- All reinforcing steel shall conform to CSA G30.18-M92 "Billet Steel Bars for Concrete Reinforcement" Grade 400W, unless noted otherwise in the BILL OF REINFORCING STEEL.
- Like bars shall be bundled, securely tied and identified as to Mark and Site No. by appropriate means. All other items to be identified in a similar fashion.
- All bars shall be bent in accordance with the following detail:



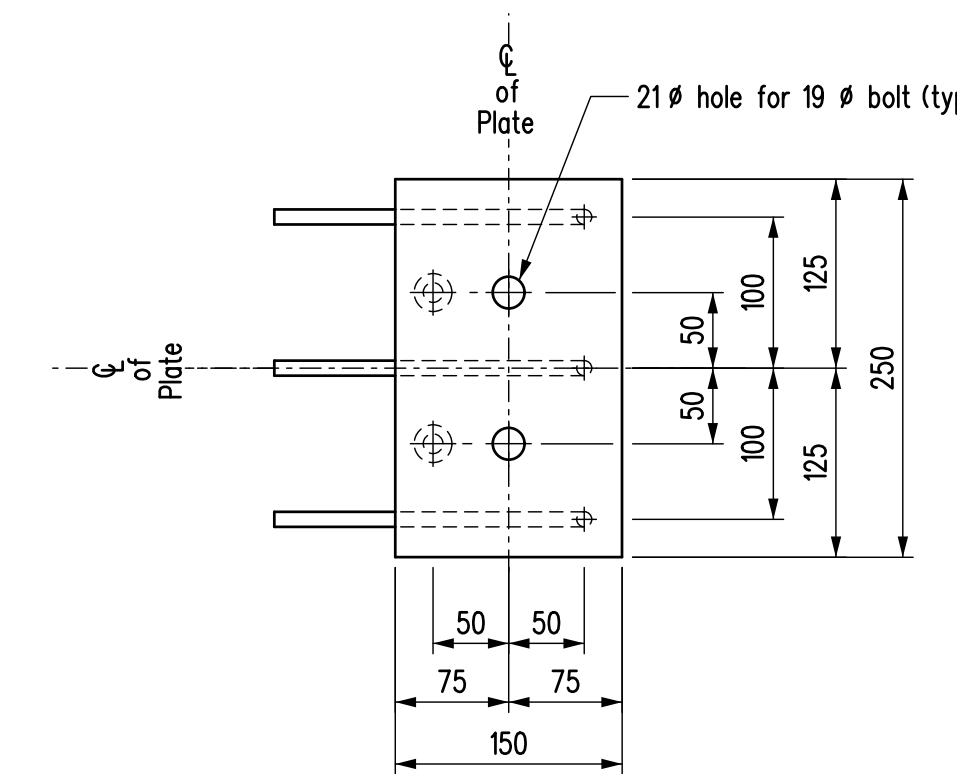
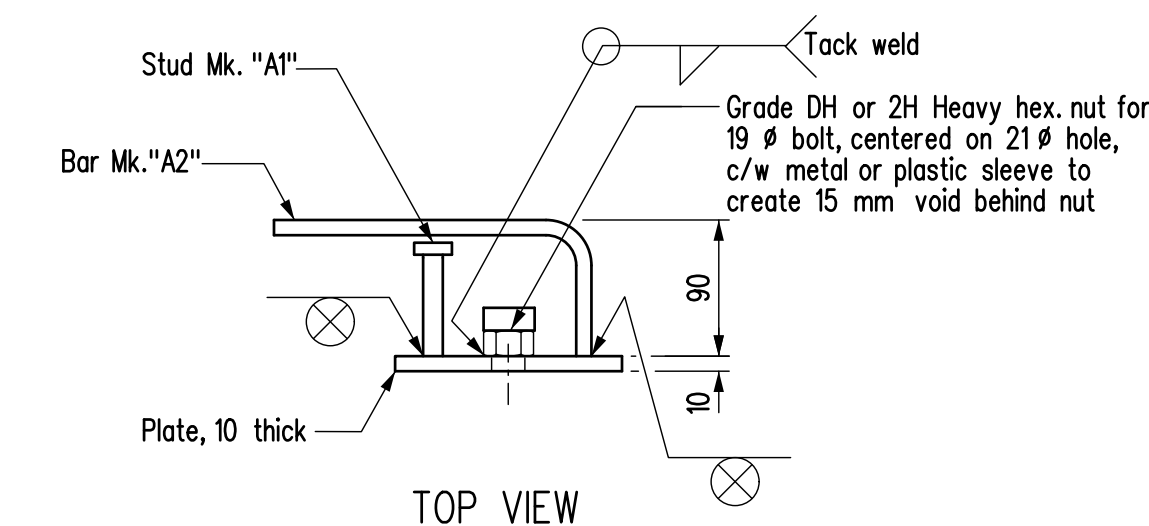
BILL OF MISCELLANEOUS METAL for PRECAST PANELS

Site No. _____

MARK No.	No.	DESCRIPTION	CORROSION PROTECTION	SIZE	LENGTH	REMARKS
Q5	4	Insert units	Hot dip galvanized			
		Each unit is fabricated from:				
		Steel plate		PL 10 x 150	250	As detailed
		2 - Studs Mk. "A1"		13 dia.	75	Nelson headed concrete anchors, Type H4L, Part No. 101-053-002 - As detailed
		3 - Bars Mk. "A2"		10 dia.	300	Nelson deformed bar anchors, Type D2L, Part No. 101-064-537 - As detailed
		2 - Heavy hex. nuts		for 19 dia. bolt		Grade DH or 2H heavy hex. nut, c/w metal or plastic sleeve
R34	8	A325 bolt c/w F436 hardened w washer		19 dia.	60	

NOTES:

- All material noted in the above Bill shall be hot dip galvanized after fabrication in accordance with CSA G164 for a minimum net retention of 610 g/m² unless otherwise stated in the specified material ASTM standards. The fabricator and galvanizer shall safeguard against embrittlement using recommended practices from applicable standards.
- Seal all welds prior to galvanizing.
- All structural steel to be CSA G40.21 Grade 300W.
- All bolts and inserts in the above Bill shall be Imperial thread.



INSERT Mk. "Q5"
Scale 1:5

NOTES:

- For location of DETAIL "A" see sheet No. P1.
- Precast panel concrete strength: f_c = 35 MPa.

REVISIONS		PRECAST PANEL DETAILS	
20___/___/___ ISSUED FOR CONSTRUCTION		RELEASD FOR CONSTRUCTION BY:	
DATE	BY	DESCRIPTION	
		DESIGN SEAL	RECORD SEAL
PLACE ENGINEERS ELECTRONIC SEAL HERE		 Infrastructure Water Management and Structures	
DESIGN		BY: B.A.N./_____	SCALE: 1:2
CHECKED: _____		SHEET No. P2	
DETAILS		BY: _____	or as shown
CHECKED: _____		SITE No. _____	